

# **USE ON REVIEW REPORT**

►	FILE #: 3-C-21-UR		AGENDA ITEM #: 2
			AGENDA DATE: 3/11/202
►	APPLICANT:	875 CORNERSTO	ONE MULTIFAMILY DEVELOPMENT
	OWNER(S):	USCC Real Estate	e Corp.
	TAX ID NUMBER:	131 069 (PART (	OF) View map on KGI
	JURISDICTION:	County Commissi	on District 6
	STREET ADDRESS:	875 Cornerstone I	Dr.
►	LOCATION:	East side of Corr Lovell Rd.	nerstone Dr., north side of Murdock Dr., southeast of
۲	APPX. SIZE OF TRACT:	12.35 acres	
	SECTOR PLAN:	Northwest County	
	GROWTH POLICY PLAN:	Planned Growth A	rea
	ACCESSIBILITY:	43.5 feet within a	of Murdock Dr., a minor arterial with a pavement width of right-of-way width of 88 feet, and off of Cornerstone Dr., a pavement width of 39.2 feet within a right-of-way width of
	UTILITIES:	Water Source:	First Knox Utility District
		Sewer Source:	First Knox Utility District
	WATERSHED:	Turkey Creek	
►	ZONING:	OB (Office, Medi	cal, and Related Services) / TO (Technology Overlay)
۲	EXISTING LAND USE:	PP (Public Parks	)
۲	PROPOSED USE:	Multi-dwelling de	evelopment
	HISTORY OF ZONING:	Rezoning request from CB/TO to I/TO in 1996 (11-M-96-RZ) was withdrawn before Planning Commission took action; rezoned from CB/TO to OB/TO in November 2020 (11-E-20-RZ).	
	SURROUNDING LAND USE AND ZONING:		ic Parks) - OB (Office, Medical, and Related Services) / TC ogy Overlay)
			re/forestry/vacant - CB (Business and Manufacturing) / TO ogy Overlay)
		East: Office - F	PC (Planned Commercial) / TO (Technology Overlay)
		West: Office - 0 Overlay)	CB (Business and Manufacturing) / TO (Technology
	NEIGHBORHOOD CONTEXT:		y a mix of office, industrial, and commercial uses, though gle family residential in the area as well.

#### **STAFF RECOMMENDATION:**

• APPROVE the request for a multifamily development with 216 dwelling units as shown on the site

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#### development plan, subject to 9 conditions.

1) Obtaining approval from the Tennessee Technology Corridor Development Authority (TTCDA) for the proposed development.

2) Connecting the development to sanitary sewer, as well as meeting other applicable requirements of the Knox County Health Department.

3) Provision of street names which are consistent with the Uniform Street Naming and Addressing system Ordinance (Ord. 91-1-102).

4) Installing all landscaping, as shown on the landscape plan, within six months of issuance of occupancy permits for the project, or posting a bond with the Knox County Department of Engineering and Public Works, to guarantee such installation.

5) Implementation of the street and intersection improvements and recommendations outlined in the Traffic Impact Study prepared by Canon & Canon and approved by the Knox County Department of Engineering and Public Works and Planning staff. The design details and timing of the installation of the improvements shall be worked out with the Knox County Department of Engineering and Public Works during the design plan stage for the development.

6) Installation of all sidewalks as identified on the concept plan. Sidewalks shall meet all applicable requirements of the Americans with Disabilities Act (ADA) and the Knox County Department of Engineering and Public Works, or posting a bond with the Knox County Department of Engineering and Public Works in an amount sufficient to guarantee the installation of the sidewalks.

7) Meeting all applicable requirements of the Knox County Department of Engineering and Public Works.

8) Meeting all applicable requirements of the Knox County Zoning Ordinance.

9) Review and approval by the Knox County Fire Marshal's Office.

#### COMMENTS:

The applicant is requesting approval of a multi-family development on 12.35 acres of an approximately 23acre tract located on the north side of Cornerstone Drive between Lovell Road and Murdock Drive, generally west of Pellissippi Parkway. The site is currently home to US Cellular soccer fields, but the apartment complex is proposed for the southeast half of the site.

The development consists of 216 dwelling units distributed between 9 buildings. 216 dwelling units over 12.35 acres yields a density of 17.49 du/ac.

Apartment buildings are to be three stories and will contain 72 one-bedroom units and 144 two- or threebedroom units. An amenity area including a clubhouse and pool area is centrally located within the complex. A dog park is proposed for the northwest corner of the site next to the soccer fields. A dumpster enclosure is proposed on the west side of the site.

The development will also require review and approval by the TN Technology Corridor Development Authority (TTCDA), and is scheduled to be heard on the March 8, 2021 TTCDA agenda.

The property was rezoned to OB (Office, Medical, and Related Services) / TO (Technology Overlay) by the Knox County Commission on December 21, 2020. The OB zoning district allows multi-dwelling development as a use on review with a density less than 24 du/ac.

The property has frontage on Cornerstone Drive and Murdock Drive, and has access from both roads. Murdock Drive is a minor arterial that becomes Dutchtown Road and has direct access to Pellissippi Parkway approximately  $\frac{3}{4}$  mile to the northeast.

Canon & Canon prepared a Traffic Impact Study (TIS) for Cornerstone Apartments, the last revision of which was on February 24, 2021. The following improvements are recommended to be implemented with the construction of this project:

- 1. Install stop signs on site at the access approaches to Cornerstone Drive and Murdock Drive.
- 2. At the intersection of Cornerstone Drive at Murdock Drive, shorten the existing westbound right-turn lane storage from 250' to 150' and shorten the existing right-turn lane taper from 200' to 150' to allow for the proposed site access along Murdock Drive to be installed outside of the existing right-turn lane taper.
- 3. Maintain intersection corner sight distances on the site driveways by ensuring that new site signage and landscaping is appropriately located.

The proposed parking for the development falls within the minimum and maximum number of spaces allowed by the Design Guidelines and those required by Knox County. The parking contains 329 surface parking

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spaces including 21 garage spaces.

Sidewalks are provided throughout the site and will connect with the existing sidewalk along Cornerstone Drive. The existing sidewalk along Murdock Drive will be replaced with a new sidewalk that parallels the street and allows the curb cut at the entry.

EFFECT OF THE PROPOSAL ON THE SUBJECT PROPERTY, SURROUNDING PROPERTIES AND THE COMMUNITY AS A WHOLE

1. The development will be served by First Knox Utility District.

2. The proposed apartment complex will have a minimal impact on the existing street system.

3. The proposed apartment complex should have a minimal impact on traffic since it can be accessed from Murdock Drive and Lovell Road, both of which are minor arterials.

4. The proposed facility is compatible with the scale and intensity of the surrounding development and zoning pattern.

#### CONFORMITY OF THE PROPOSAL TO CRITERIA ESTABLISHED BY THE KNOX COUNTY ZONING ORDINANCE

1) With the recommended conditions, the proposed facility meets all requirements of the OB zoning district and the criteria for approval of a use on review.

2) The proposed facility is consistent with the general standards for uses permitted on review:

- a) The proposal is consistent with the adopted plans and policies of the General Plan and Sector Plan.
- b) The use is in harmony with the general purpose and intent of the Zoning Ordinance.
- c) The use is compatible with the character of the neighborhood where it is proposed.
- d) The use will not significantly injure the value of adjacent property.
- e) The use will not draw additional traffic through residential areas since the site is located on a minor arterial street.
- f) No surrounding land uses will pose a hazard or create an unsuitable environment for the proposed use.

#### CONFORMITY OF THE PROPOSAL TO ADOPTED PLANS

1. The Northwest County Sector Plan designates the property as Office land use. The Office land use allows the OB zone in the county, which allows multifamily use as a use on review.

2. The site is identified as being within the Planned Growth Area on the Knoxville-Knox County-Farragut Growth Policy Plan.

ESTIMATED TRAFFIC IMPACT: 1907 (average daily vehicle trips)

Average Daily Vehicle Trips are computed using national average trip rates reported in the latest edition of "Trip Generation," published by the Institute of Transportation Engineers. Average Daily Vehicle Trips represent the total number of trips that a particular land use can be expected to generate during a 24-hour day (Monday through Friday), with a "trip" counted each time a vehicle enters or exits a proposed development.

ESTIMATED STUDENT YIELD: 16 (public school children, grades K-12)

Schools affected by this proposal: Farragut Primary/Intermediate, Hardin Valley Middle, and Hardin Valley Academy.

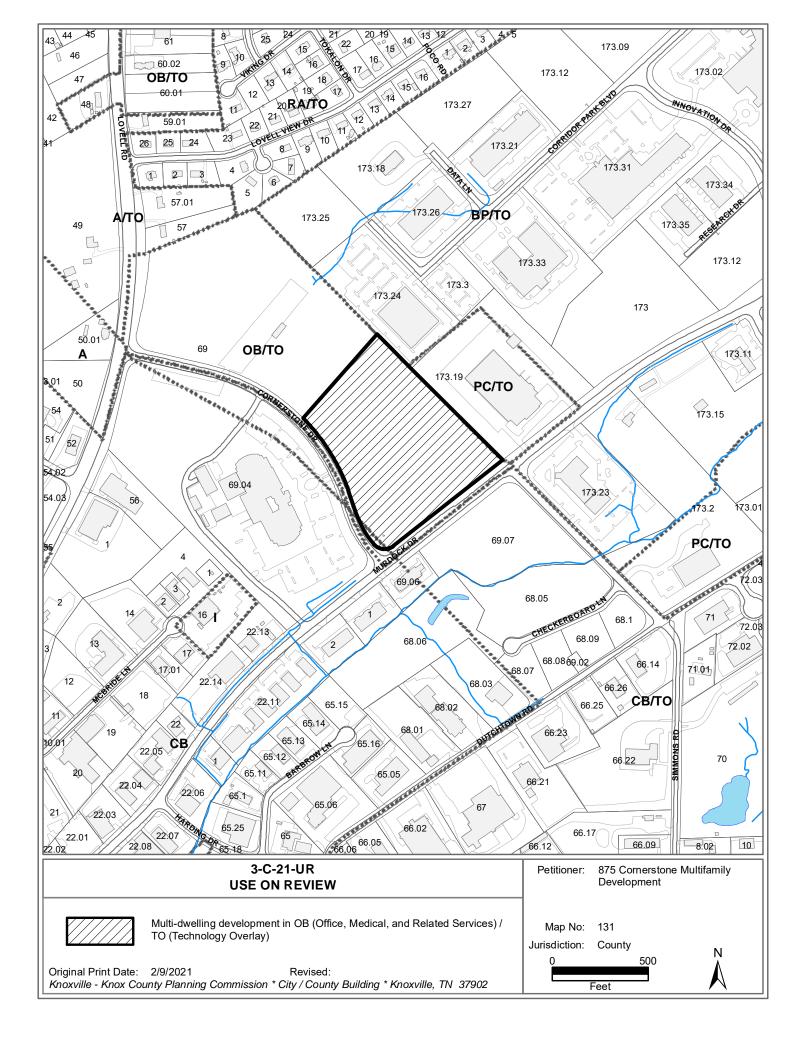
• Potential new school population is estimated using locally-derived data on public school student yield generated by new housing.

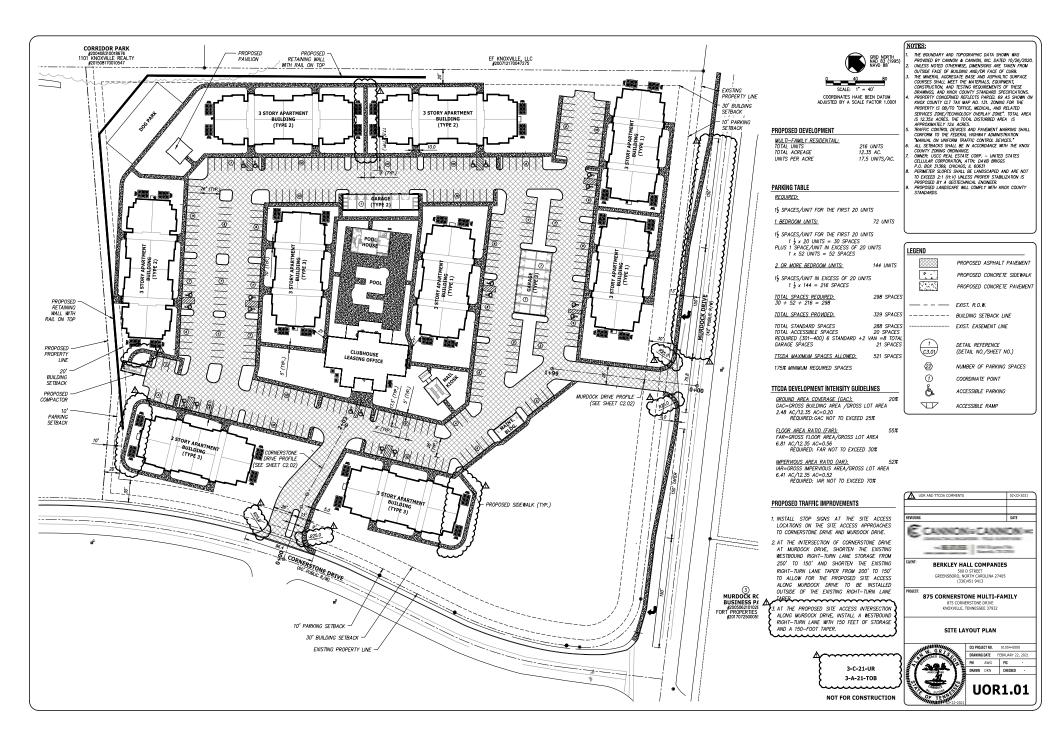
• Students are assigned to schools based on current attendance zones as determined by Knox County Schools. Students may request transfers to different zones, and zone boundaries are subject to change.

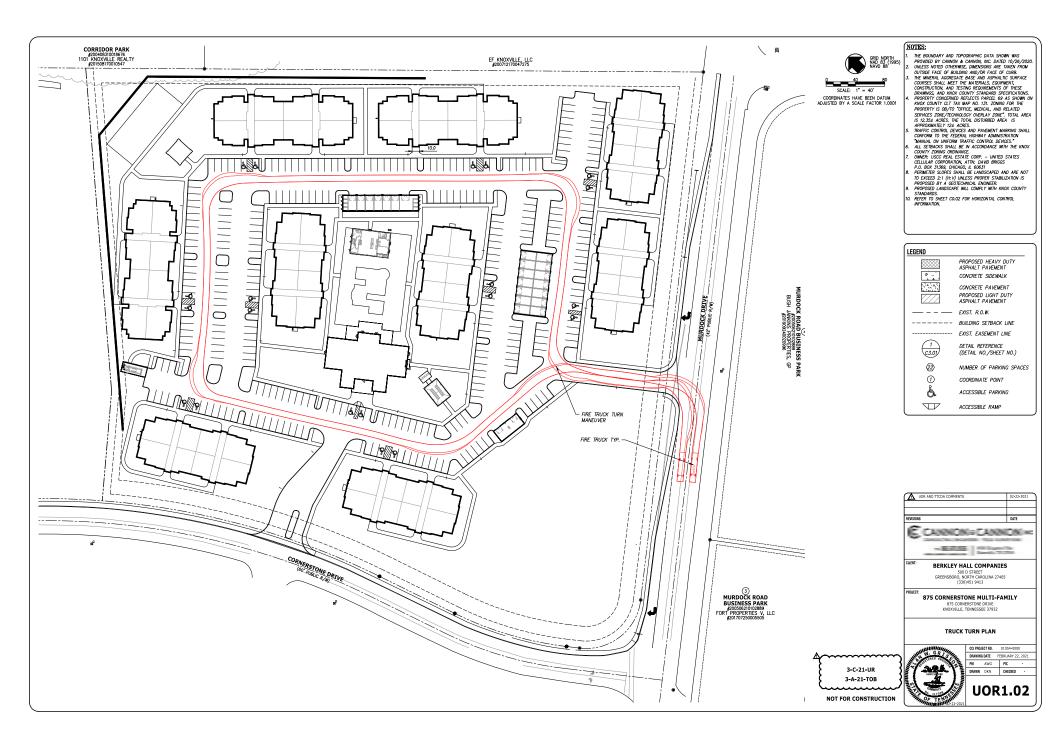
• Estimates presume full build-out of the proposed development. Build-out is subject to market forces, and timing varies widely from proposal to proposal.

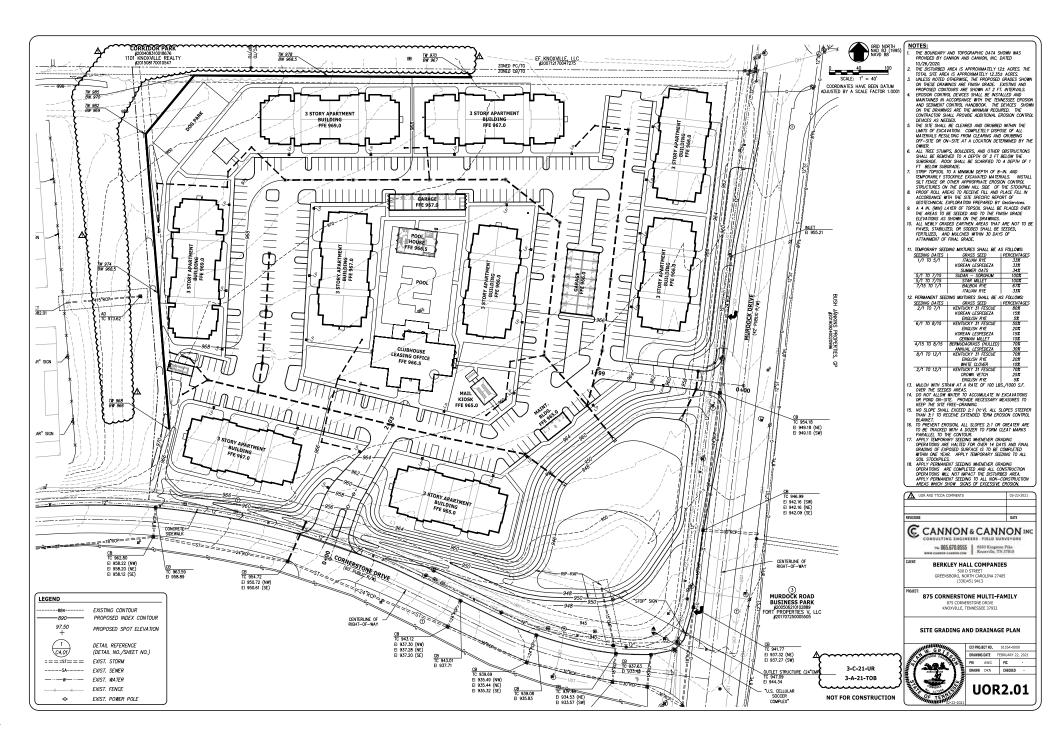
• Student yields from new development do not reflect a net addition of children in schools. Additions occur incrementally over the build-out period. New students may replace current population that ages through the system or moves from the attendance zone.

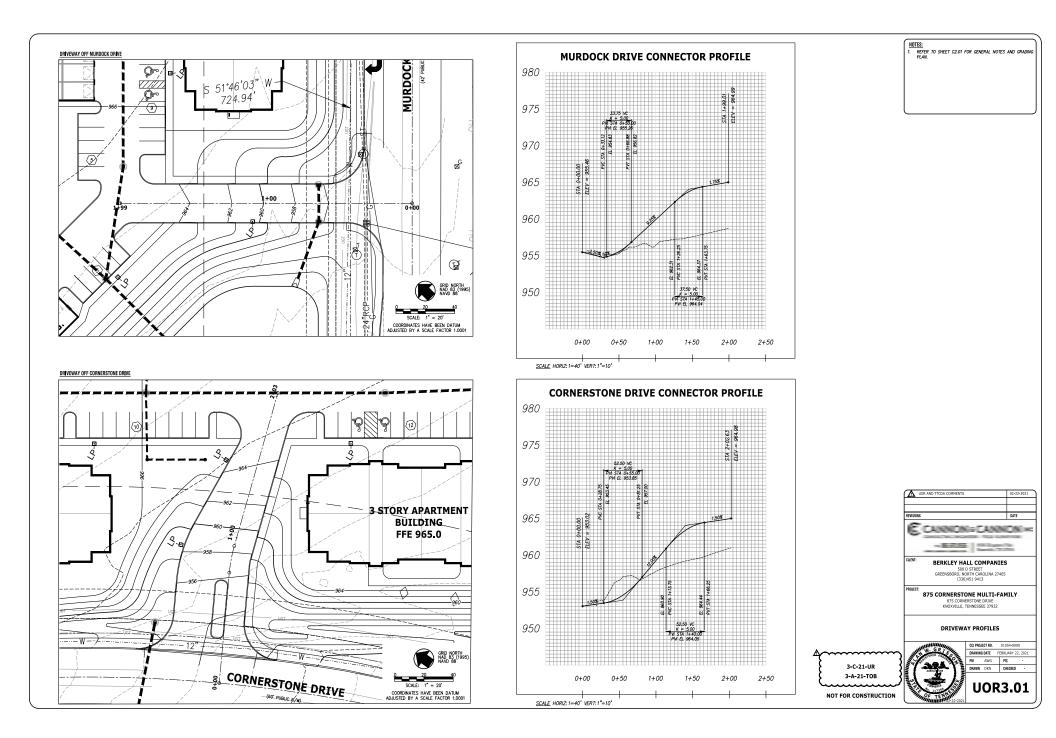
The Planning Commission's approval or denial of this request is final, unless the action is appealed to the Knox County Board of Zoning Appeals. The date of the Knox County Board of Zoning Appeals hearing will depend on when the appeal application is filed. Appellants have 30 days to appeal a Planning Commission decision in the County.



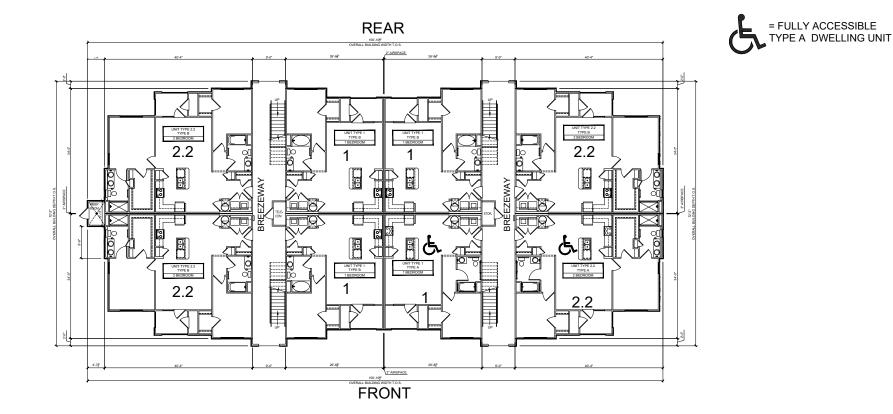






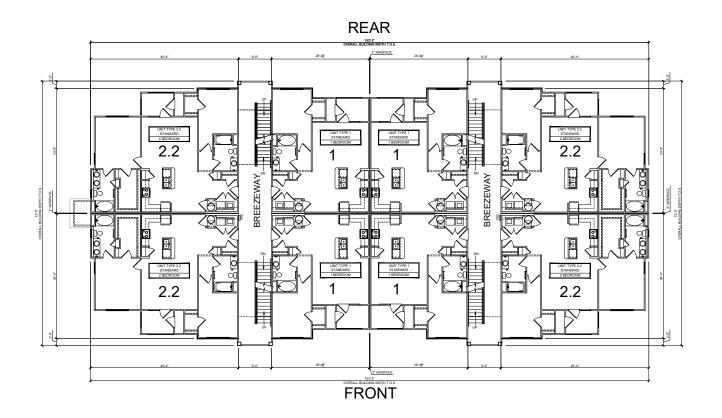






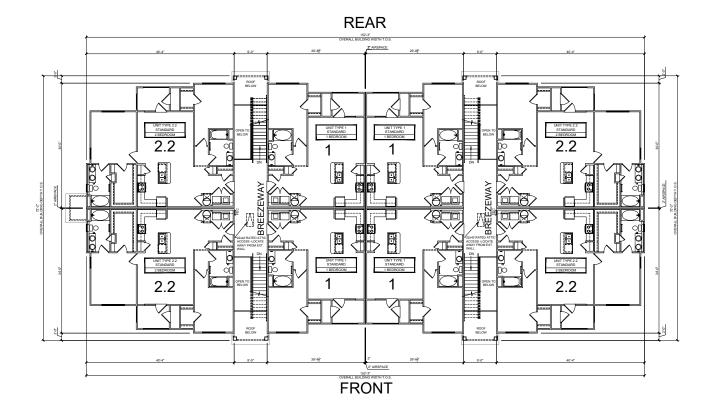
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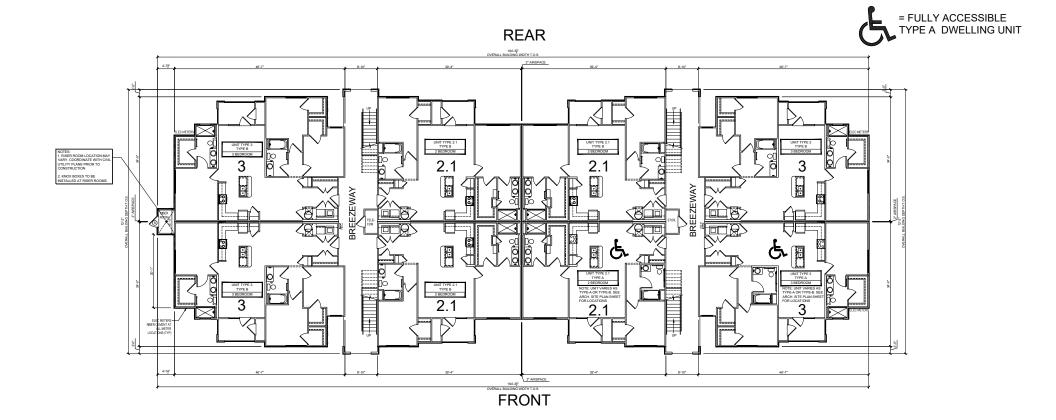


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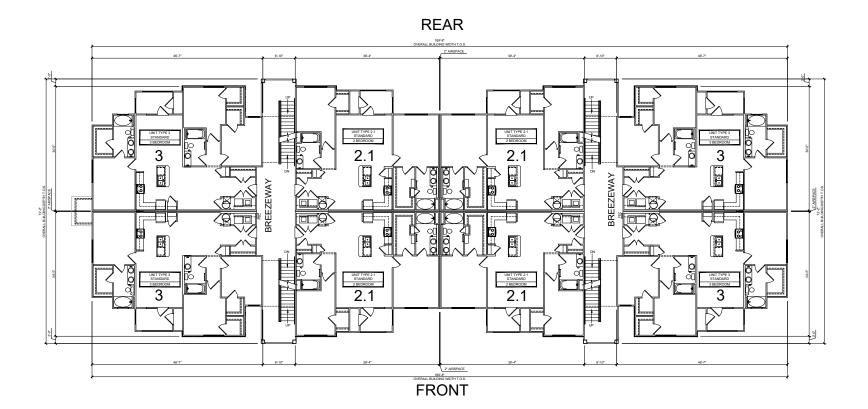






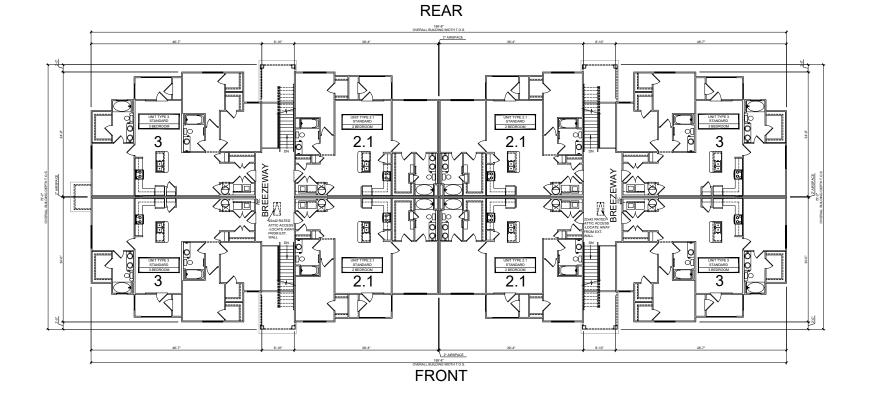
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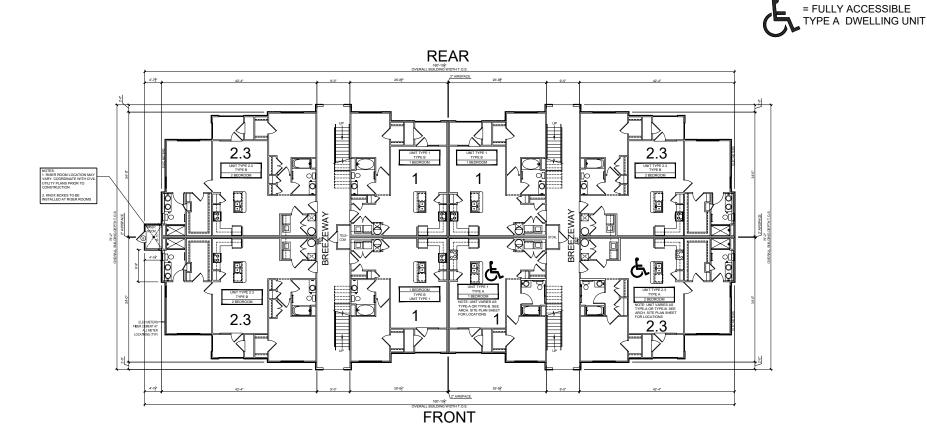
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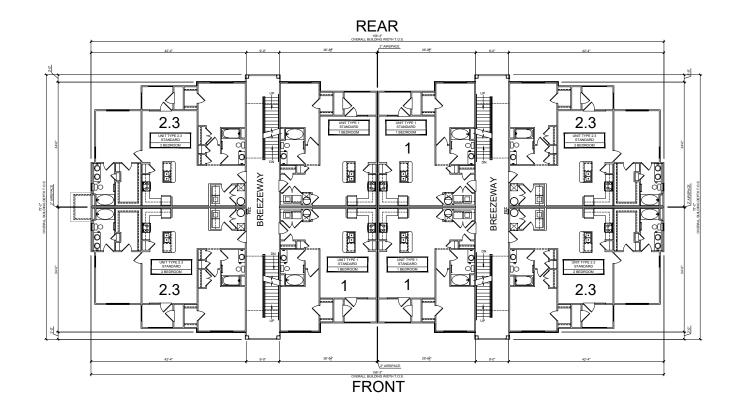






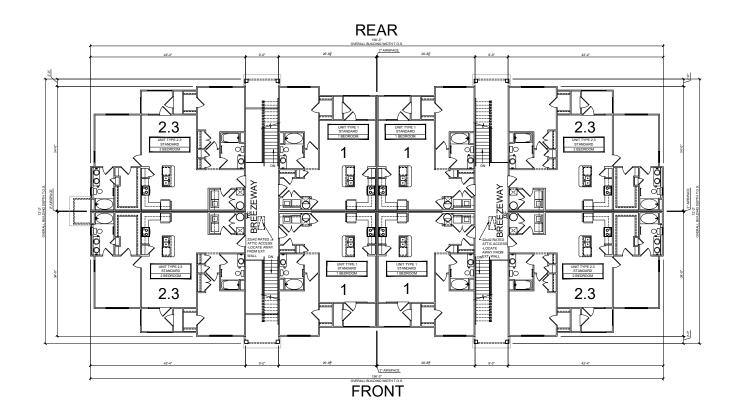
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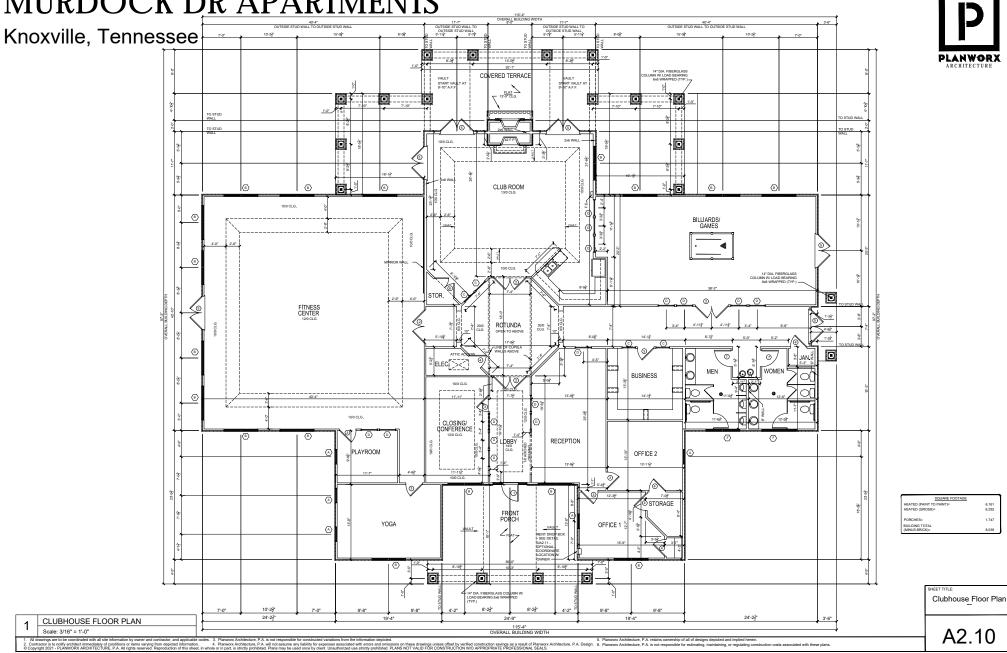


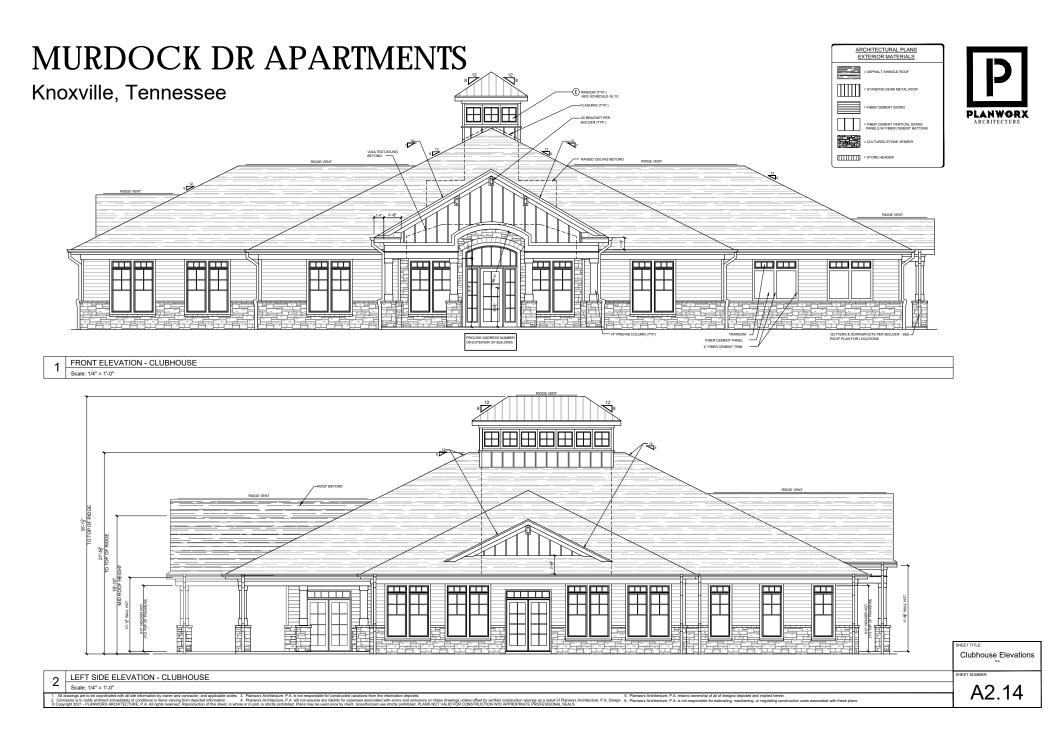


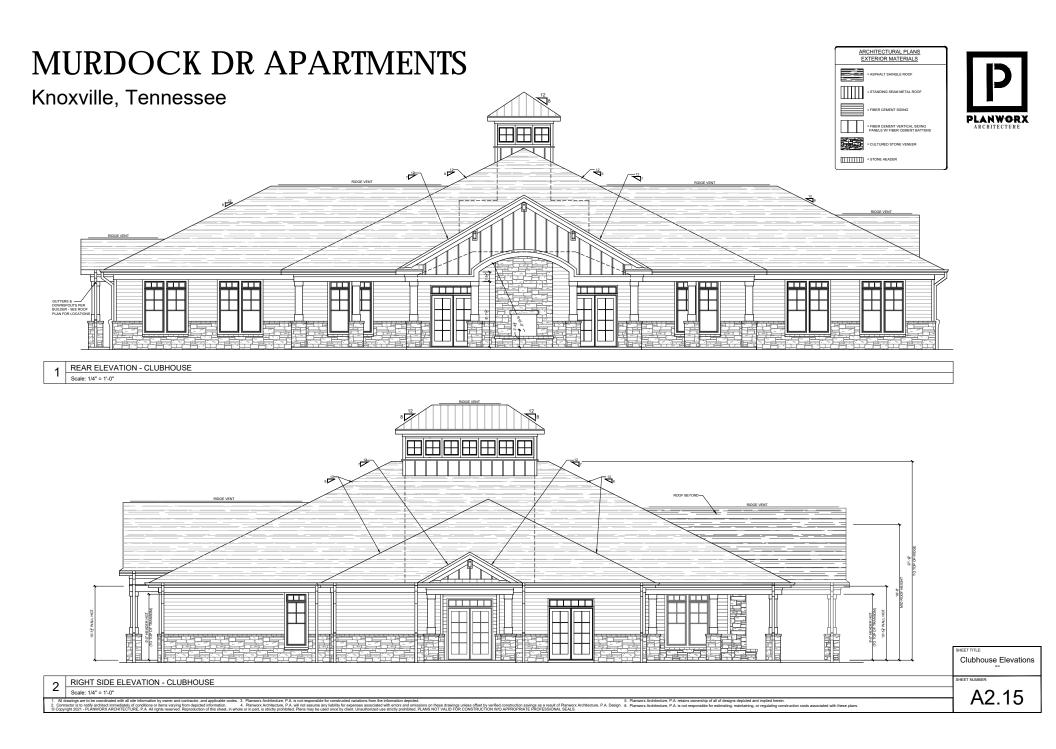
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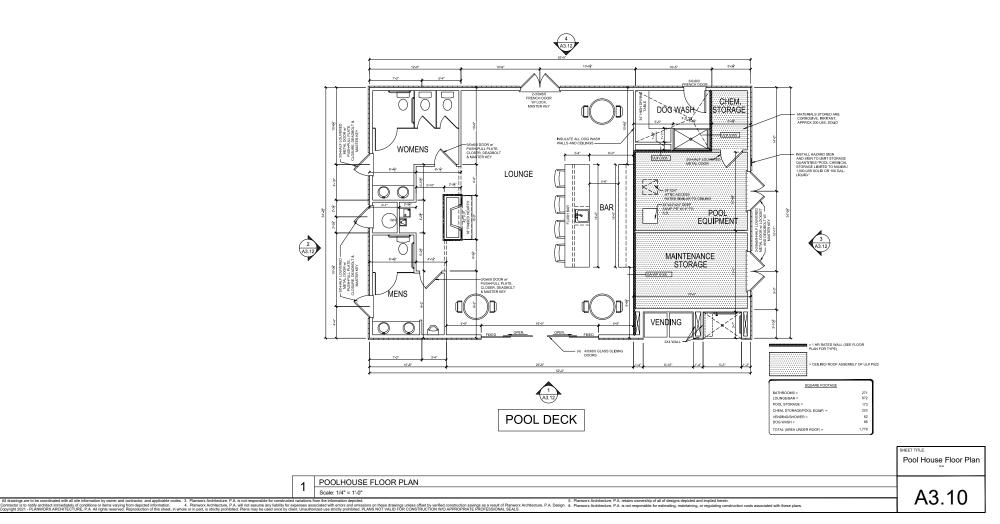


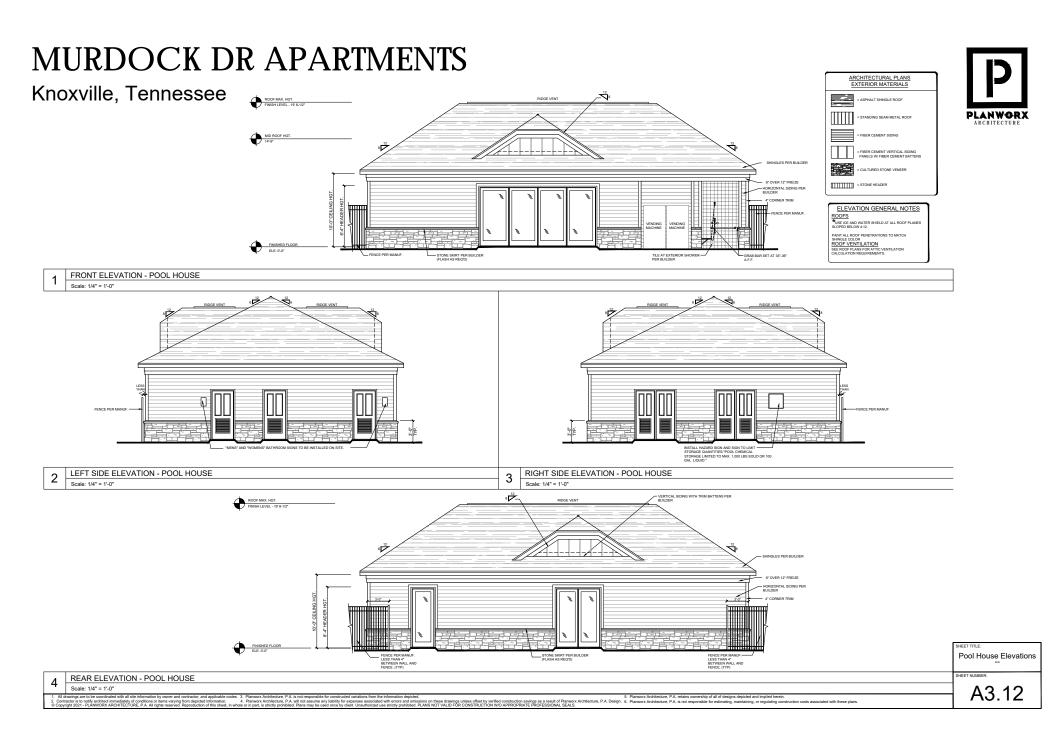






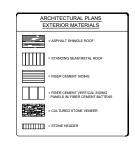






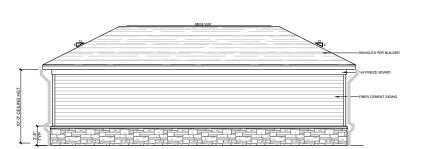
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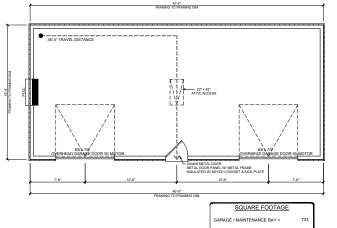
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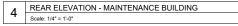




5 FRONT ELEVATION - MAINTENANCE BUILDING Scale: 1/4" = 1'-0"







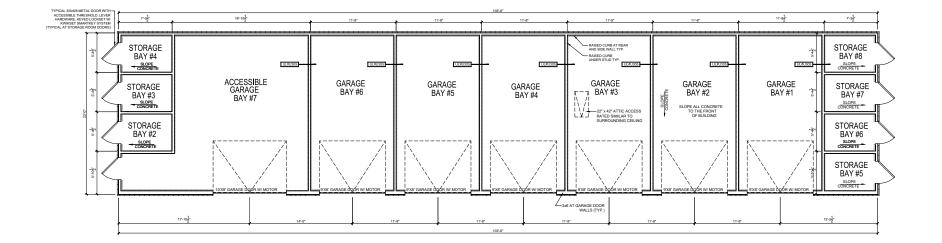




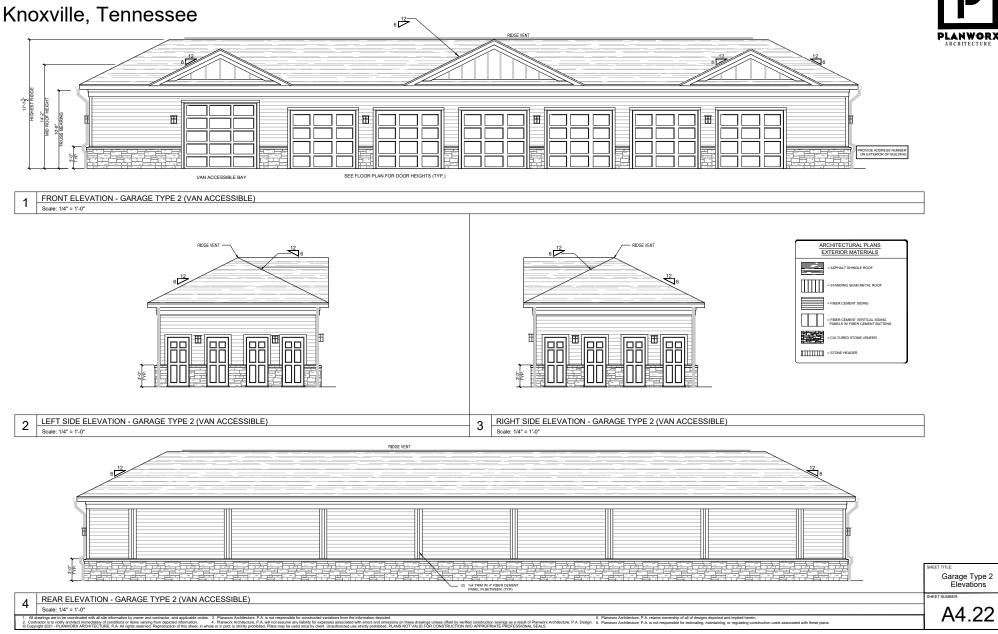
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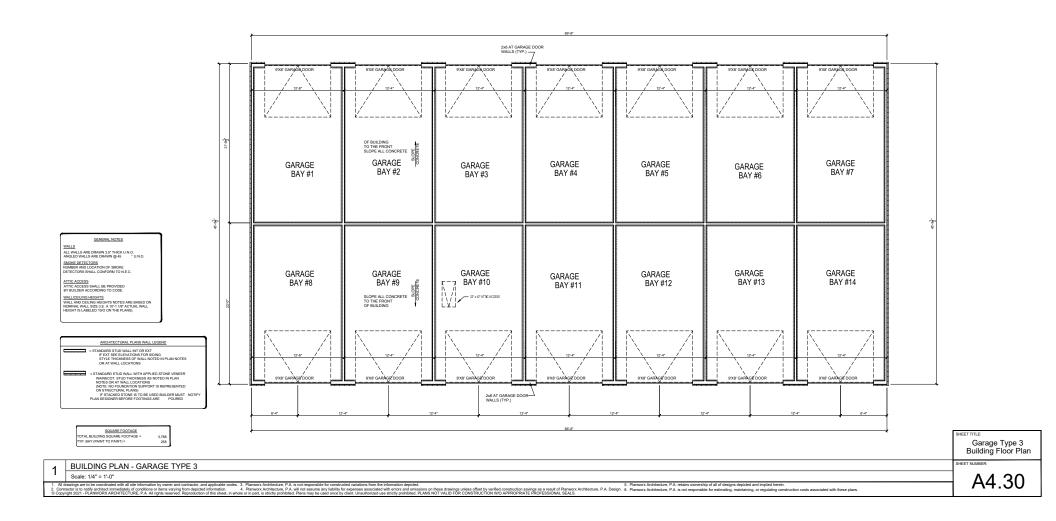




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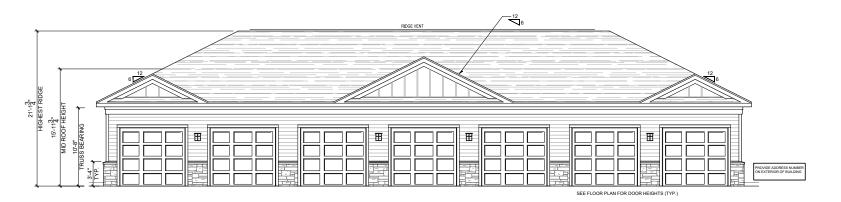






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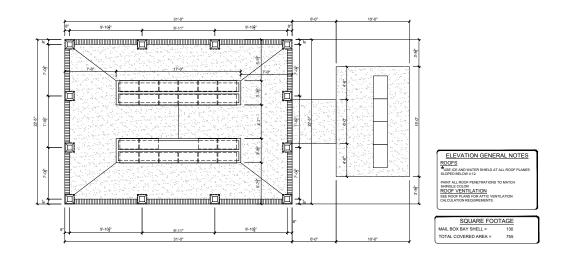


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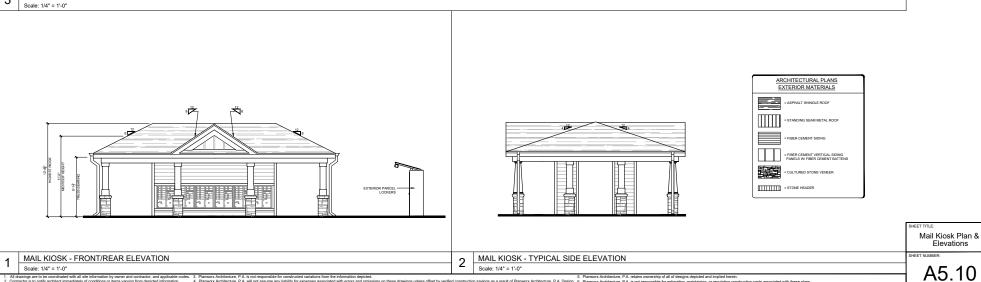
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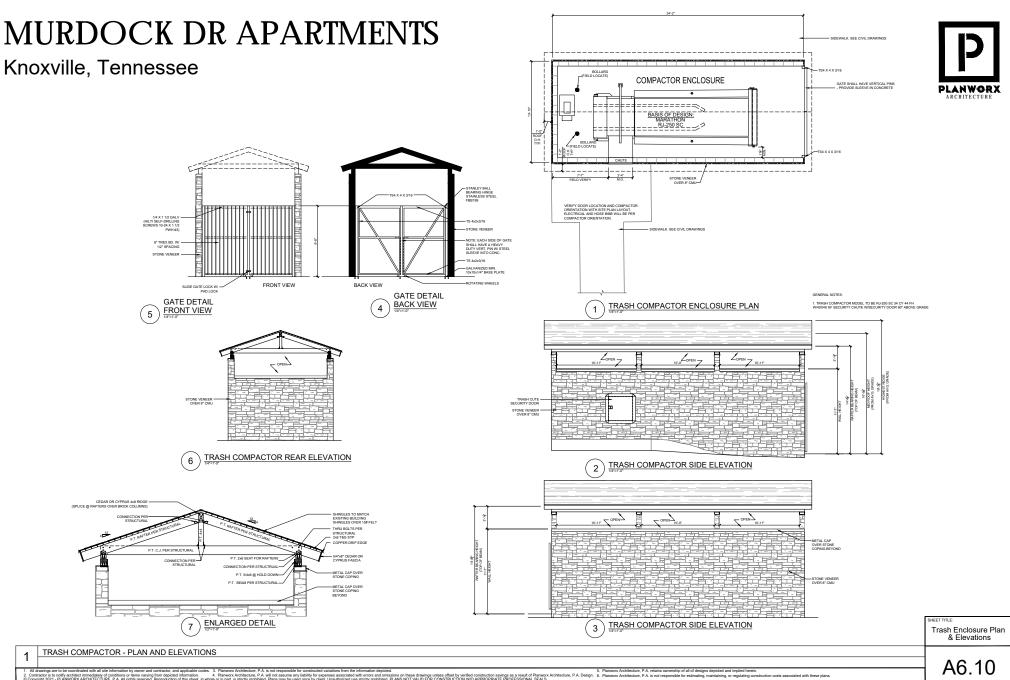




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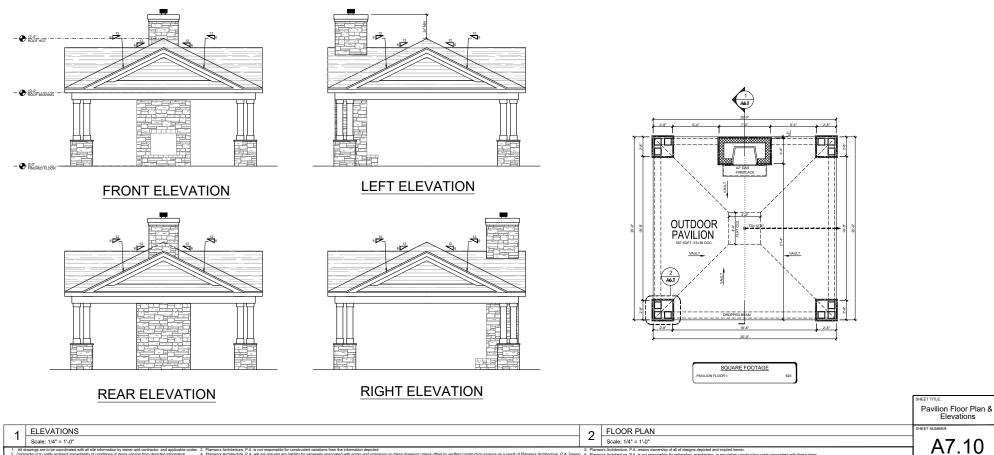
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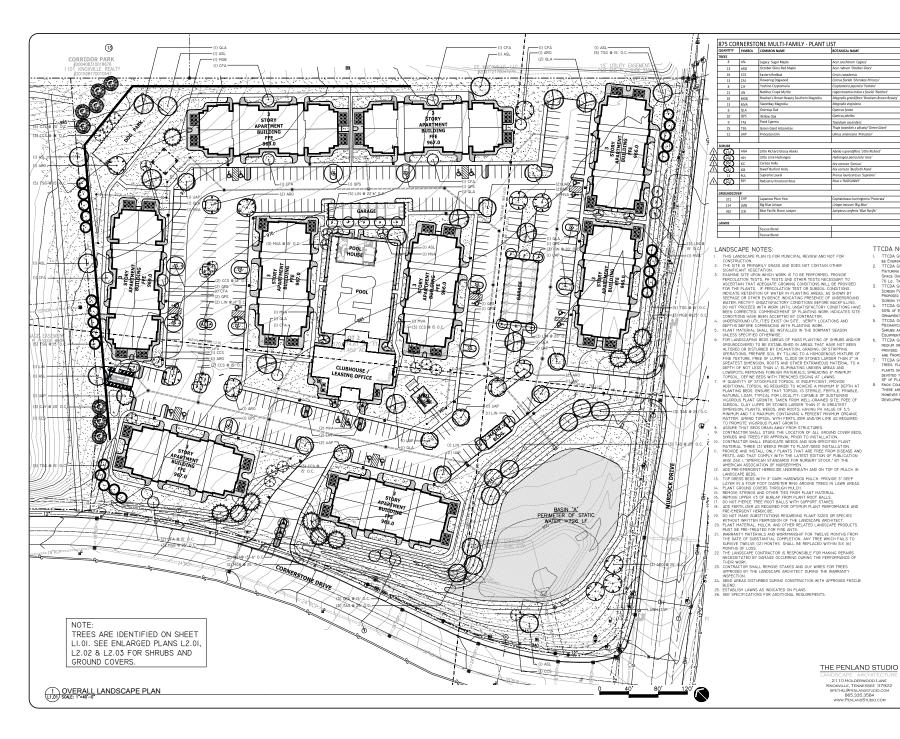
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REMARKS

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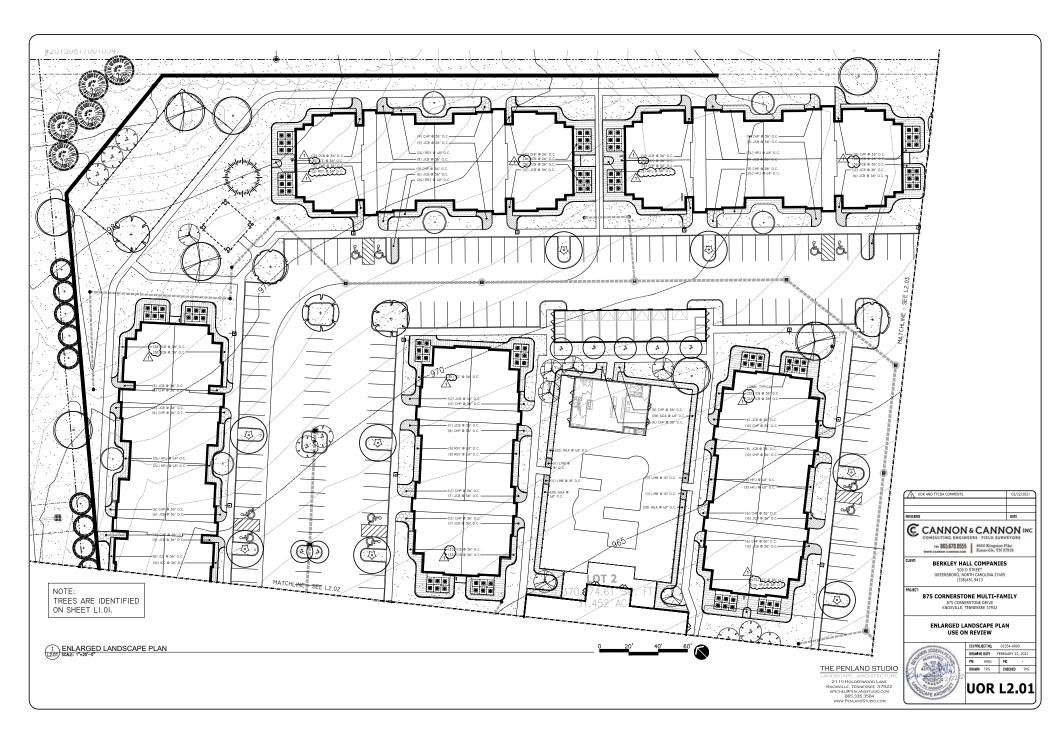
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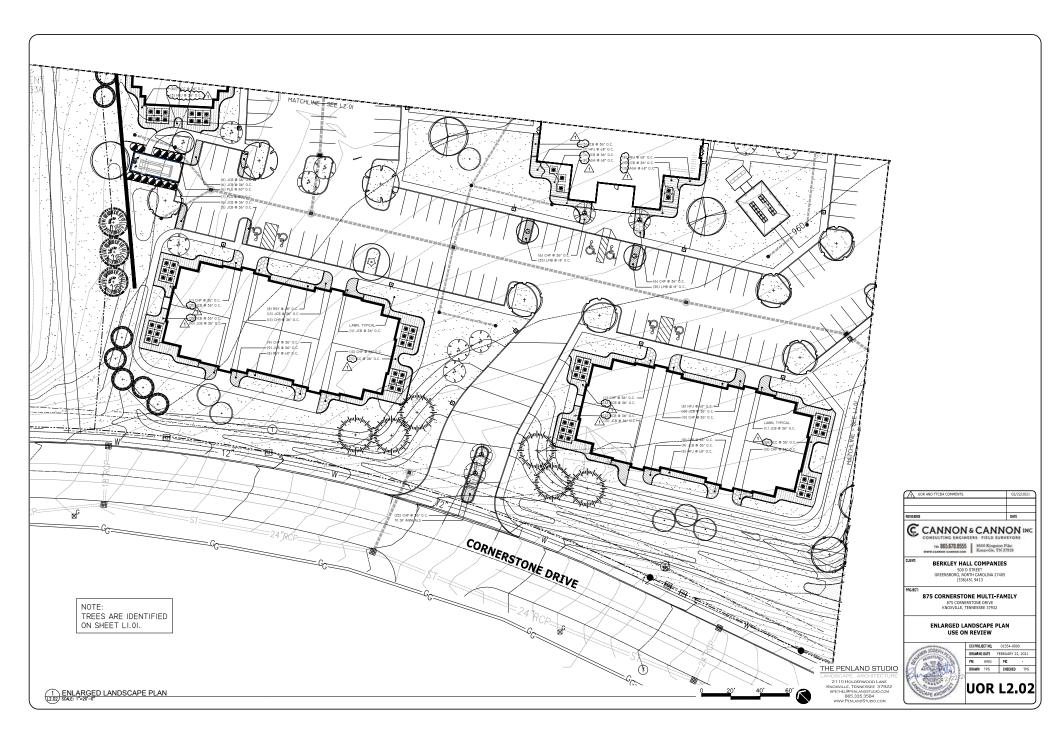
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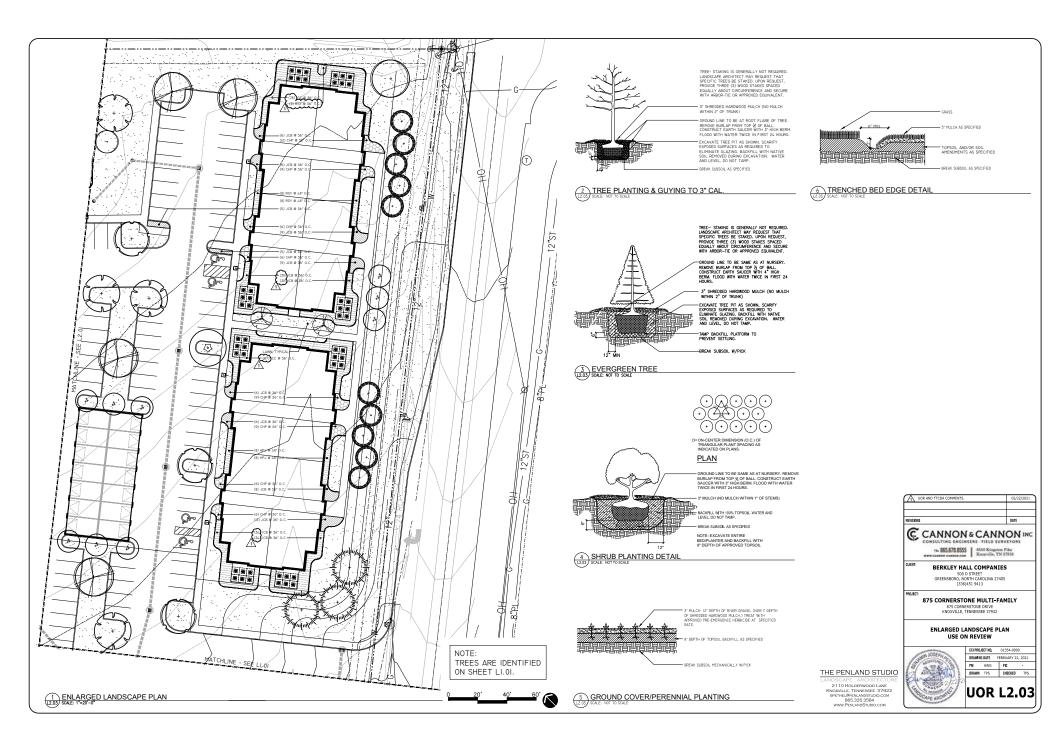
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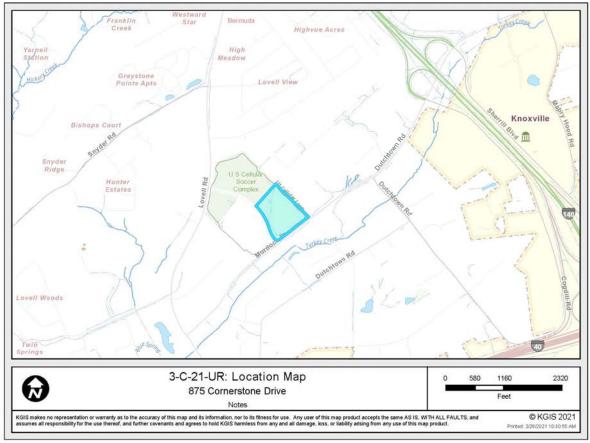
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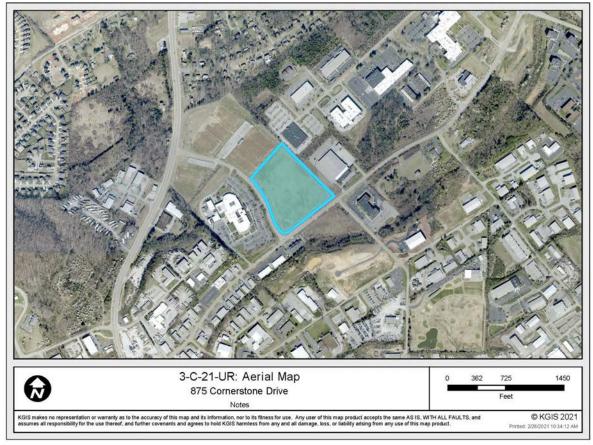


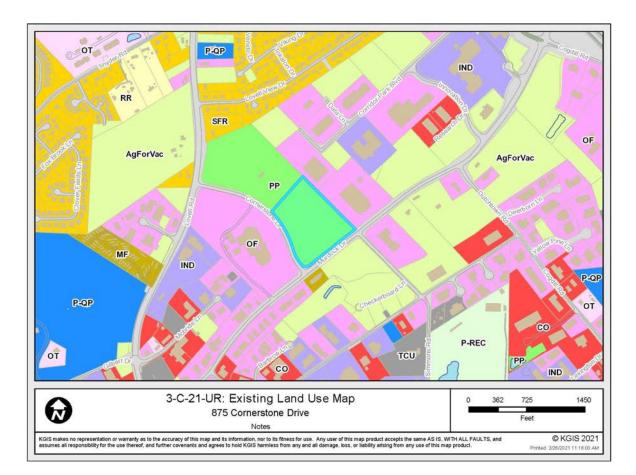
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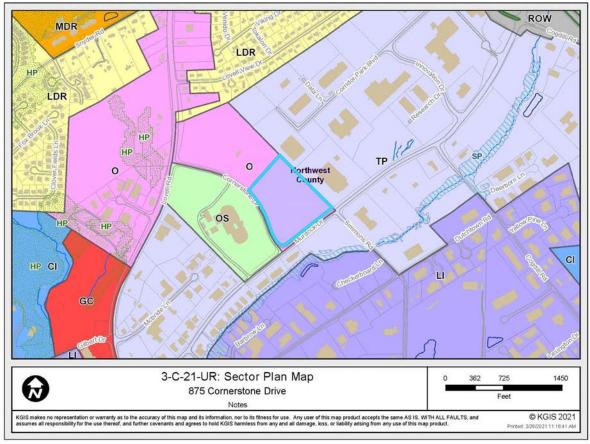


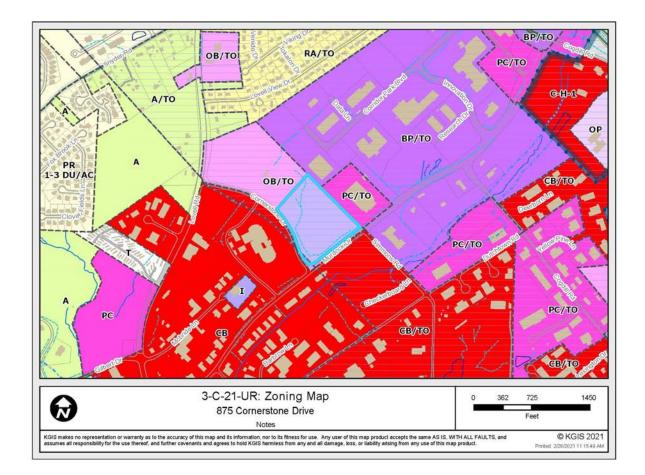
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## 3-C-21-UR EXHIBIT A. Contextual Images





## 875 CORNERSTONE DRIVE APARTMENTS KNOX COUNTY, TENNESSEE

TRAFFIC IMPACT STUDY

CORNERSTONE DRIVE AT MURDOCK DRIVE KNOX COUNTY, TENNESSEE

CCI PROJECT NO. 01554-0000



#### PREPARED FOR:

Berkley Hall Companies 500-D State Street Greensboro, NC 27405

#### SUBMITTED BY:

Cannon & Cannon, Inc. 8550 Kingston Pike Knoxville, TN 37919 865.670.8555

> REVISED February 23 **2021**

## **875 CORNERSTONE DRIVE APARTMENTS KNOX COUNTY. TENNESSEE**

#### TRAFFIC IMPACT STUDY

#### CORNERSTONE DRIVE AT MURDOCK DRIVE KNOX COUNTY, TENNESSEE

#### CCI PROJECT NO. 01554-0000



This report replaces the previous version of the traffic impact study dated 01/18/2020 prepared for this project in its entirety. The associated changes are related to comments received from the Knoxville-Knox County Planning, which are located in Appendix F.

#### PREPARED FOR:

#### SUBMITTED BY:

Berkley Hall Companies 500-D State Street Greensboro, NC 27405

Cannon & Cannon, Inc. 8550 Kingston Pike Knoxville, TN 37919 865.670.8555

REVISED February 23

2021

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#### **EXECUTIVE SUMMARY**

This report provides a summary of a traffic impact study that was performed for a proposed multifamily residential development to be located on Cornerstone Drive in Knox County, Tennessee. The project site is located in the northeast quadrant of the intersection of Cornerstone Drive and Murdock Drive. The development plan for this project proposes a multi-family residential development with 216 units. The proposed development will have two access driveways, one access onto Cornerstone Drive and one access onto Murdock Drive.

The purpose of this study was the evaluation of the traffic operational and safety impacts of the proposed development upon roadways in the vicinity of the project site. Discussion with Knox County officials resulted in the existing intersection of Cornerstone Drive at Murdock Drive being identified for detailed study. Additionally, the proposed site access locations along Cornerstone Drive and Murdock Drive were included in the study. Appropriate intersection evaluations such as capacity analyses and signal warrant analyses were conducted at the study intersections for existing and future conditions, both with and without site generated traffic, in order to determine the anticipated impacts and to establish recommended measures to mitigate these impacts.

The primary conclusion of this study is that the traffic generated from the proposed development will not have significant impacts at the studied intersections. The capacity analysis indicates a minimal increase in delay is expected at each intersection once the proposed development is built-out. The intersection of Cornerstone Drive at Murdock Drive does not warrant a signal installation under build-out conditions and the existing intersection configuration / control is expected to adequately accommodate traffic generated by the proposed development.

Additionally, anticipated traffic volumes at the site access intersection along Murdock Drive indicate a westbound right-turn lane is not recommended to be installed. Currently, the site access on Murdock Drive is proposed to be installed in the existing right-turn lane taper for the intersection of Cornerstone Drive and Murdock Drive. It is recommended to shorten the existing right-turn lane / taper to accommodate the installation of the proposed site access so the site access is not installed within a turn lane taper.

The following listing is a summary of the improvements that are recommended to be implemented with the construction of this project:

- 1. Install STOP signs at the site access locations on the site access approaches to Cornerstone Drive and Murdock Drive.
- 2. At the intersection of Cornerstone Drive at Murdock Drive, shorten the existing westbound right-turn lane storage from 250' to 150' and shorten the existing right-turn lane taper from 200' to 150' to allow for the proposed site access along Murdock Drive to be installed outside of the existing right-turn lane taper.
- 3. Maintain intersection corner sight distances on the site driveways by ensuring that new site signage and landscaping is appropriately located.



#### **INTRODUCTION & PURPOSE OF STUDY**

This report provides a summary of a traffic impact study that was performed for a proposed multifamily residential development to be located on Cornerstone Drive in Knox County, Tennessee. The project site is located in the northeast quadrant of the intersection of Cornerstone Drive and Murdock Drive. FIGURE 1 is a location map showing the major roadways in the project site vicinity.



LOCATION MAP

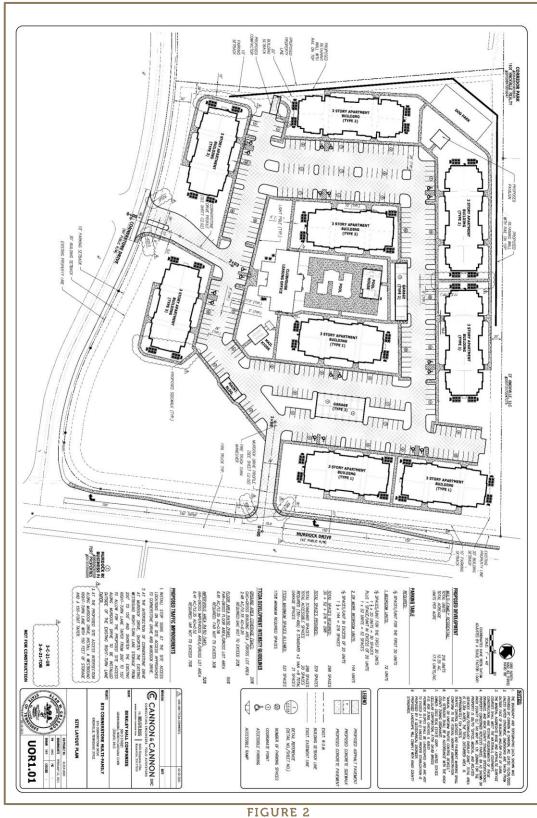
The development plan for this project proposes a multi-family residential development with 216 units. The proposed development will have two access driveways, one access onto Cornerstone Drive and one access onto Murdock Drive. FIGURE 2 is a Conceptual Site Plan detailing the proposed site.

The purpose of this study was the evaluation of the traffic operational and safety impacts of the proposed development upon roadways in the vicinity of the project site. Discussion with Knox County officials resulted in the existing intersection of Cornerstone Drive at Murdock Drive being identified for detailed study. Additionally, the proposed site access locations along Cornerstone Drive and Murdock Drive were included in the study. Appropriate intersection evaluations such as capacity analyses and signal warrant analyses were conducted at the study intersections for existing and future conditions, both with and without site generated traffic, in order to determine the anticipated impacts and to establish recommended measures to mitigate these impacts.



#### SECTION 2 INTRODUCTION & PURPOSE OF STUDY

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CONCEPTUAL SITE PLAN



#### **EXISTING CONDITIONS**

#### EXISTING ROADWAY CONDITIONS

Roadway conditions for the study roadways are summarized as follows:

- Cornerstone Drive is a three-lane local road with one lane in each direction and a center twoway left-turn lane. Lane widths are 12 feet and curb, gutter, and sidewalk are on both sides. There is no posted speed limit along Cornerstone Drive.
- Murdock Drive is a three-lane road with one lane in each direction and a center two-way leftturn lane. It is classified as a Minor Arterial per Knoxville-Knox County Planning Major Road Plan. Lane widths are 12 feet and the posted speed limit is 40 mph within the vicinity of the proposed site.

Traffic control for the study intersections is as follows:

• Cornerstone Drive at Murdock Drive is currently side-street STOP controlled.

#### **EXISTING SITE CONDITIONS**

The project site is located in the northeast quadrant of the intersection of Cornerstone Drive at Murdock Drive. It is bordered by the US Cellular Soccer Complex to the north and the US Cellular Business Office to the west. The site is relatively flat and does slope upward from Murdock Drive to the soccer fields north of the proposed site. The site access point on Cornerstone Drive is proposed to tie into the road across from the existing US Cellular Business Office access, creating a four-way intersection along Cornerstone Drive. FIGURE 3 provides an aerial view of the project site and the surrounding area.

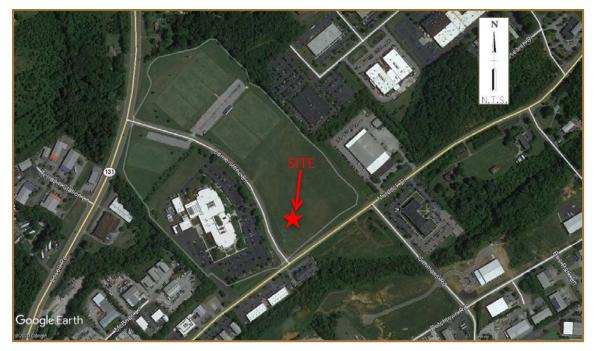


FIGURE 3 EXISTING SITE CONDITIONS



#### EXISTING TRAFFIC DATA

Two types of existing traffic data were gathered for this study. The Tennessee Department of Transportation (TDOT) collects annual average daily traffic (AADT) data on roadways in the study area. A count station was found near the project site that was felt to have particular relevance for this study. The most currently available data from this station is contained in Table 1.

COUNT YEAR	TDOT COUNT STATION 47000464 MURDOCK DRIVE EAST OF CORNERSTONE DRIVE
2014	6,119
2015	6,775
2016	7,147
2017	6,821
2018	6,476
2019	6,555

#### TABLE 1: ANNUAL AVERAGE DAILY TRAFFIC COUNT SUMMARY

In addition to the available AADT data, intersection turning movement traffic counts were conducted at the existing study intersections to determine the current peak hour operating volumes. The traffic counts were conducted during the first week of November 2020. During this time, regional traffic volumes and patterns were recovering from COVID-19 pandemic restrictions, including business and school closures and widespread telecommuting or working from home practices. At the time of the counts, schools were conducting in-school instruction at a reduced student capacity. In consultation with the Knoxville-Knox County Planning, the November 2020 count data was increased by 20% to address reductions in typical travel volumes due to the ongoing pandemic.

The 2020 raw traffic data is summarized in FIGURE 4 and the factored traffic data is summarized in FIGURE 5. The raw data traffic count summary sheets are contained in APPENDIX A.

#### EXISTING CAPACITY ANALYSES / LEVELS-OF-SERVICE

Capacity analyses employing the methods of the *Highway Capacity Manual* were conducted for the existing conditions at the study intersections. These analyses were performed with the 2020 existing factored traffic volumes, shown in FIGURE 5, and existing intersection traffic control and lane configurations. The EVALUATIONS section of this report may be referenced for tabular summaries of these analyses, while more detailed summaries are presented on the computer printouts contained in APPENDIX C. Also contained in APPENDIX C is a section entitled "Capacity and Level of Service Concepts", which provides a description of the utilized procedures.



#### SECTION 3 EXISTING CONDITIONS

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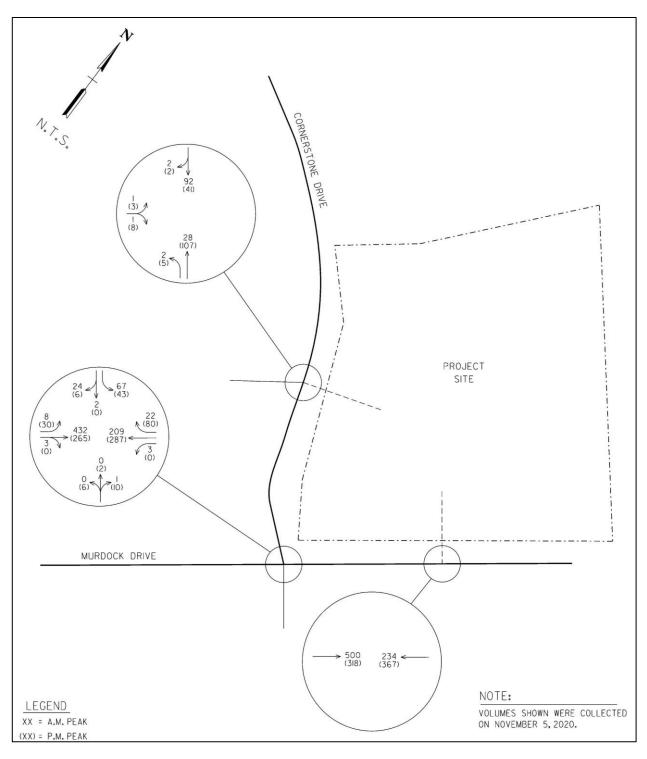


FIGURE 4 2020 EXISTING RAW TRAFFIC VOLUMES



#### SECTION 3 EXISTING CONDITIONS

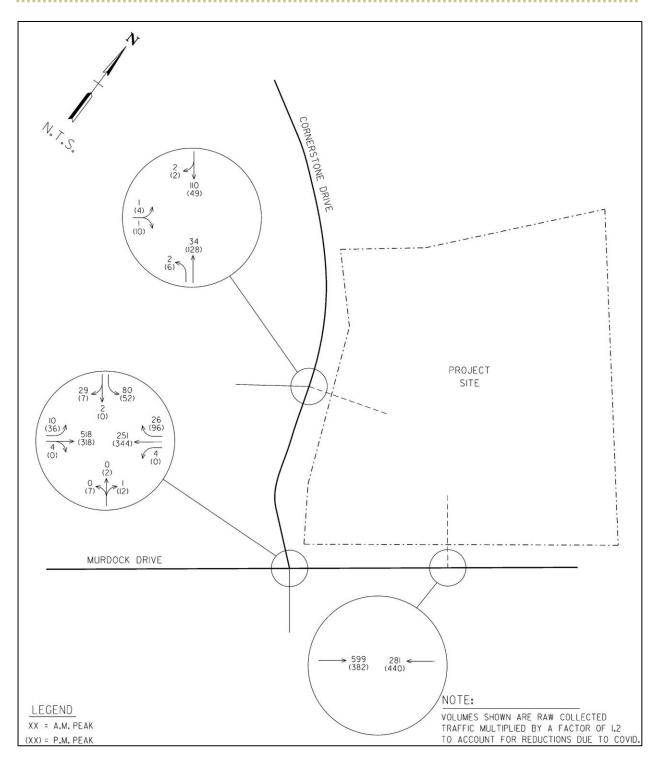


FIGURE 5 2020 EXISTING FACTORED TRAFFIC VOLUMES



#### **BACKGROUND CONDITIONS**

#### BACKGROUND TRAFFIC GROWTH

The proposed development is anticipated to be constructed in one general phase with completion anticipated by 2022. Therefore, year 2022 was established as the appropriate design / analysis year for the study. In order to determine traffic volumes resulting solely from background traffic growth to year 2022, it was necessary to establish an annual growth rate for existing traffic. The TDOT ADT values previously discussed, as well as knowledge of the area, were used to determine an approximate annual growth rate. Based on the available data, a background annual growth rate of two percent was assumed. FIGURE 6 contains the background traffic volumes that would result from this annual growth rate from year 2020, when the counts were conducted, to year 2022.

#### BACKGROUND CAPACITY ANALYSES / LEVELS-OF-SERVICE

Capacity analyses as described in the Existing Conditions section of this report were conducted utilizing the Year 2022 background volumes shown in FIGURE 6 and existing intersection traffic control and lane configurations. The EVALUATIONS section of this report may be referenced for tabular summaries of these analyses, while more detailed summaries are presented on the computer printouts contained in APPENDIX C.



BACKGROUND CONDITIONS

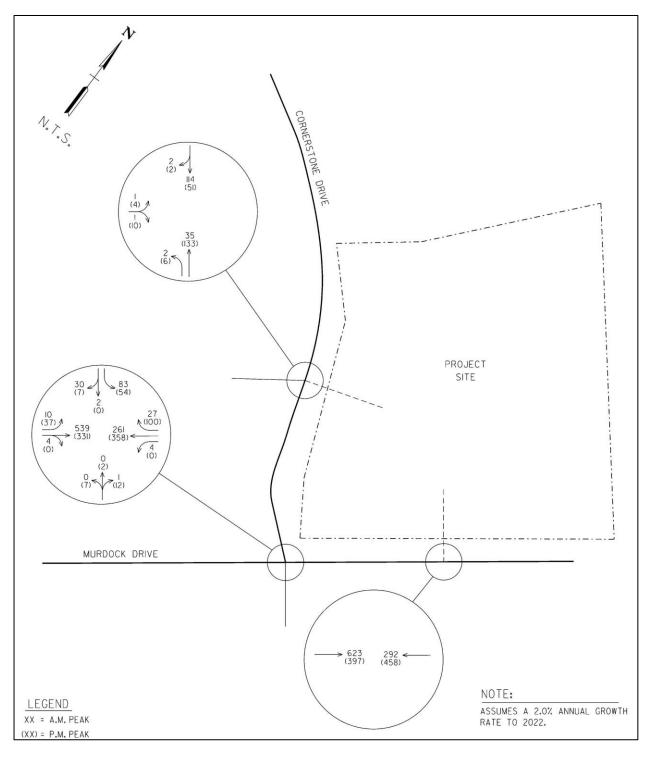


FIGURE 6 2022 BACKGROUND TRAFFIC VOLUMES



#### **FUTURE CONDITIONS**

#### TRIP GENERATION

In order to estimate the expected traffic volumes to be generated by the proposed development, the procedures recommended by the Institute of Transportation Engineers were utilized. The proposed development will include 216 multi-family residential apartment units. Local trip generation rates developed by the Knoxville-Knox County Metropolitan Planning Commission for multi-family apartment type developments within the region were utilized to generate the estimated trips. The generated traffic volumes were determined based on the data for the peak hours of adjacent street traffic. See TABLE 2 for a summary of the traffic generated for this project. More detailed information is contained in APPENDIX B.

LAND USE	ITE Code	SIZE	WEEKDAY (TRIPS/DAY)	AM PEAK HOUR (TRIPS/HOUR)	PM PEAK HOUR (TRIPS/HOUR)
Multi-Family Residential	n/a	216 Dwelling Units	1,906	109	155
Entering Trips Exiting Trips			953 (50%) 953 (50%)	24 (22%) 85 (78%)	85 (55%) 70 (45%)

#### TABLE 2: TRIP GENERATION SUMMARY

A.M. Peak Hour trip generation is based on Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m. P.M. Peak Hour trip generation is based on Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

#### TRIP DISTRIBUTION AND ASSIGNMENT

The proposed trip distribution for this development was determined through a review of existing travel patterns, local knowledge of the study area, proposed site location in relation to surrounding roadway network, and engineering judgement. FIGURE 7 provides a summary of how the above site generated trips would be assigned to the study intersection. FIGURE 8 provides the proposed trip assignment volumes to the studied intersections.

#### FUTURE TRAFFIC VOLUMES

Future projected traffic volumes for the study intersection were developed by adding the generated and assigned trips shown in FIGURE 8 to the 2022 background traffic volumes developed in the previous section and shown in FIGURE 6. These combined 2022 volumes reflect the existing traffic, the background traffic growth, and the generated traffic from the proposed development. These future volumes are shown on FIGURE 9 and are the combined volumes used in the analyses of future conditions with the proposed development.



#### FUTURE CAPACITY ANALYSES / LEVELS-OF-SERVICE

Capacity analyses, as described in the Existing Conditions section of this report, were conducted for future conditions utilizing the traffic volumes shown in the build-out scenario. These analyses employed appropriate modifications to the existing lane configurations and traffic control in order to serve the development, as discussed in the EVALUATIONS section of this report. Tabular summaries of the analysis results and associated discussion are also contained in the EVALUATIONS section. In addition, detailed computer printout summaries of the analyses are contained in APPENDIX C.



**FUTURE CONDITIONS** 

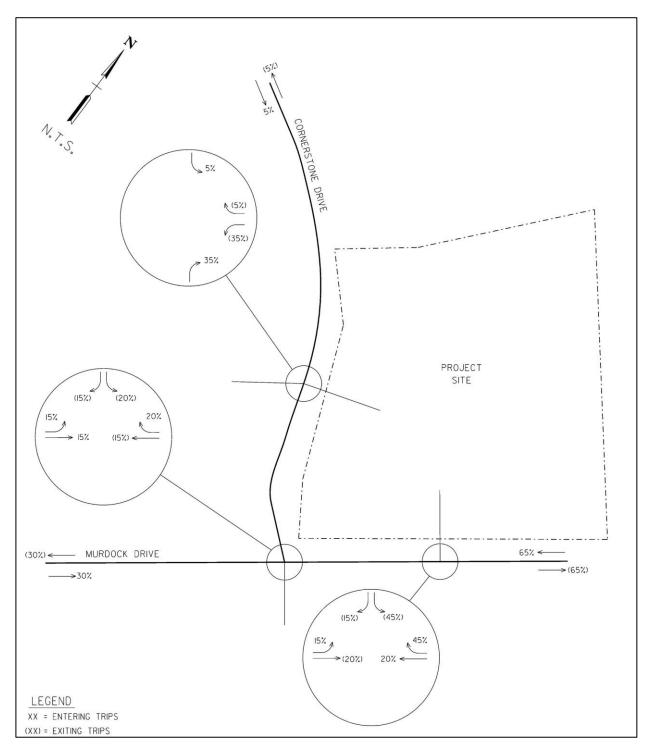


FIGURE 7 TRIP DISTRIBUTION



FUTURE CONDITIONS

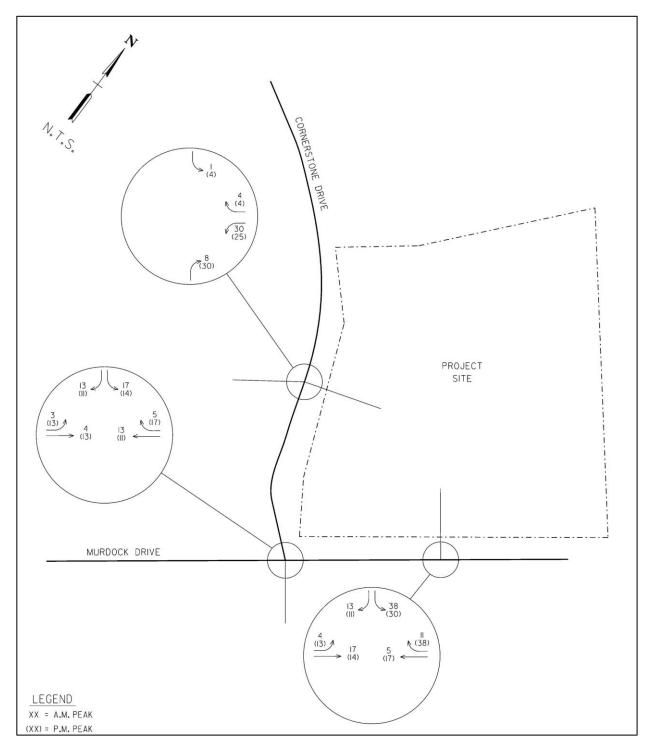


FIGURE 8 TRIP ASSIGNMENT



FUTURE CONDITIONS

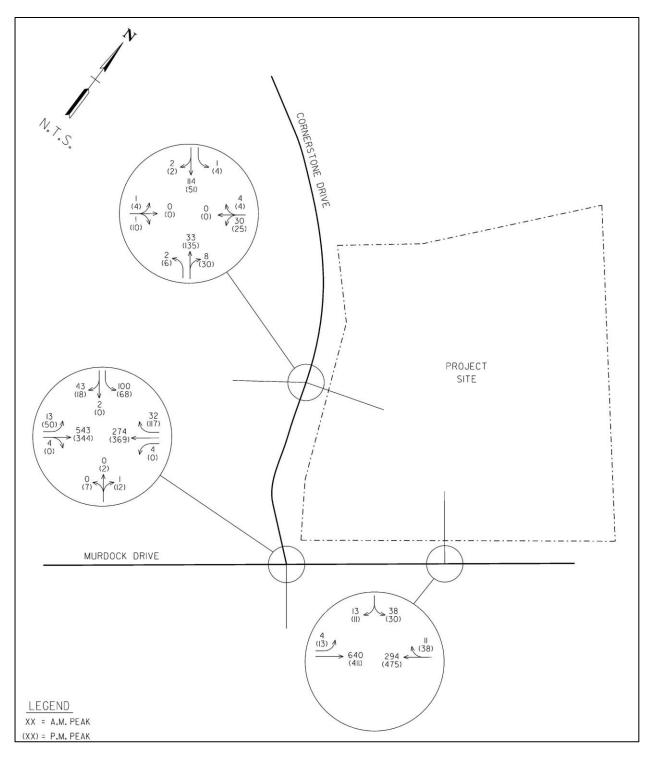


FIGURE 9 2022 COMBINED TRAFFIC VOLUMES



#### **EVALUATIONS**

#### INTERSECTION CAPACITY ANALYSES

As discussed in the preceding sections of this report, capacity analyses employing the methods of the Highway Capacity Manual (HCM 6<sup>th</sup> Edition) were conducted for the study intersections. These analyses were performed for the previously discussed development scenarios. A summary of the capacity analyses results is shown in TABLE 3, while the resulting conclusions and recommendations are covered in the CONCLUSIONS and RECOMMENDATIONS section of this report.

INTERSECTION	TIME PERIOD	YEAR 2020 EXISTING (LOS/DELAY)	YEAR 2022 BACKGROUND (LOS/DELAY)	YEAR 2022 COMBINED (LOS/DELAY)
Cornerstone Dr. at Murdock Dr. <sup>1</sup> SIDE STREET STOP CONTROL	A.M. P.M.	C 17.1 C 16.7	C 17.8 C 17.4	C 19.0 C 18.6
Site Access at Cornerstone Dr. <sup>1</sup> SIDE STREET STOP CONTROL	EB A.M. EB P.M. WB A.M. WB P.M.	A 9.3 A 9.1 -	A 9.4 A 9.1 -	A 9.5 A 9.3 B 10.0 B 10.7
Site Access at Murdock Dr. <sup>1</sup> SIDE STREET STOP CONTROL	A.M. P.M.	-	-	C 15.8 C 15.9

#### TABLE 3: CAPACITY ANALYSES SUMMARY

<sup>1</sup>SIDE STREET STOP CONTROL – Data shown are Level-of-Service and Average Vehicular Delay (seconds) for the critical side street approach utilizing HCM methodology.

#### TRAFFIC SIGNAL WARRANT ASSESSMENT

The traffic signal volume warrants from the Manual on Uniform Traffic Control Devices were evaluated for the study intersection of Cornerstone Drive at Murdock Drive. Traffic signal warrant analyses were performed for three different scenarios; existing, background, and combined. These are summarized below, along with the associated results. Spreadsheets summarizing these analyses are contained in APPENDIX D.

- Scenario 1 2020 Existing Factored Traffic Volumes No signal warrants satisfied
  - o Raw traffic data factored by 1.2 to account for reductions due to COVID-19 impacts
- Scenario 2 Year 2022 Background Traffic Volumes No signal warrants satisfied
  - Existing factored data with 2.0% annual growth applied from Year 2020 to Year 2022
- Scenario 3 Year 2022 Combined Traffic Volumes No signal warrants satisfied
  - AM Peak hour generated trips were added to volumes beginning at hours 7am, 8am, 11am, and 12pm
  - PM Peak hour generated trips were added to volumes beginning at hours 2pm, 3pm, 4pm, and 5pm



#### TURN LANE ASSESSMENTS

A turn lane evaluation was conducted for a potential right-turn lane to enter the project site at the proposed site access intersections along Cornerstone Drive and Murdock Drive. This evaluation, which utilized Knox County turn lane warrants, found that a right-turn lane is not warranted for the Murdock Drive site access intersection and is not warranted for the Cornerstone Drive site access intersection. The spreadsheets summarizing this evaluation are contained in APPENDIX E.

The existing two-way-left-turn lane along Murdock Drive was evaluated for potential queuing conflicts for eastbound left turns entering the site along Murdock Drive and existing westbound left turns entering the existing commercial development at the intersection of Cornerstone Drive and Murdock Drive. Proposed development plans indicate the site access along Murdock Drive will be constructed roughly 400 feet to the east of the intersection of Cornerstone Drive at Murdock Drive.

The interaction of left turns at these two intersections is typically not desirable and can often result in conflicting left-turn movements from the main road to the side streets. However, the capacity analysis for these two study intersections indicate minimal left turn queues of less than 25 feet at each intersection is expected during the studied peak hours. Since minimal offset left-turn traffic is expected at each of these intersections, the offset left-turn configuration is not anticipated to negatively impact intersection operation at either intersection of Cornerstone Drive at Murdock Drive and the proposed site access at Murdock Drive.

#### SIGHT DISTANCE ASSESSMENT

Intersection sight distance was assessed looking both directions from the proposed site driveway intersections. Excellent sight distance is available at all locations to satisfy requirements, as all roadway approaches are relatively flat, straight and without sight limiting vegetation or fixed objects. Care should be taken during the site development process to ensure that site features such as landscaping and signage do not restrict these existing sight distances.

#### PEDESTRIAN CONNECTION ASSESSMENT

The proposed development will provide sidewalk along the property frontage to Murdock Drive and tie into existing sidewalk along Cornerstone Drive. The existing property contains a walking trail that traverses the US Cellular soccer fields and office building properties. The proposed development will remove the portion of the walking trail on this site property.



#### **CONCLUSIONS & RECOMMENDATIONS**

The primary conclusion of this study is that the traffic generated from the proposed development will not have significant impacts at the studied intersections. The capacity analysis indicates a minimal increase in delay is expected at each intersection once the proposed development is built-out. The intersection of Cornerstone Drive at Murdock Drive does not warrant a signal installation under build-out conditions and the existing intersection configuration / control is expected to adequately accommodate traffic generated by the proposed development.

Additionally, anticipated traffic volumes at the site access intersection along Murdock Drive indicate a westbound right-turn lane is not recommended to be installed. Currently, the site access on Murdock Drive is proposed to be installed in the existing right-turn lane taper for the intersection of Cornerstone Drive and Murdock Drive. It is recommended to shorten the existing right-turn lane / taper to accommodate the installation of the proposed site access so the site access is not installed within a turn lane taper.

The following listing is a summary of the improvements that are recommended to be implemented with the construction of this project:

- 1. Install STOP signs at the site access locations on the site access approaches to Cornerstone Drive and Murdock Drive.
- 2. At the intersection of Cornerstone Drive at Murdock Drive, shorten the existing westbound right-turn lane storage from 250' to 150' and shorten the existing right-turn lane taper from 200' to 150' to allow for the proposed site access along Murdock Drive to be installed outside of the existing right-turn lane taper.
- 3. Maintain intersection corner sight distances on the site driveways by ensuring that new site signage and landscaping is appropriately located.



#### SECTION 8 APPENDIX

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APPENDIX

**APPENDIX ORDER:** 

- A. TRAFFIC DATA
- **B. TRIP GENERATION INFORMATION**
- C. CAPACITY ANALYSES
- D. SIGNAL WARRANT SPREADSHEETS
- **E. TURN LANE WARRANT SHEETS**
- F. MPC COMMENTS



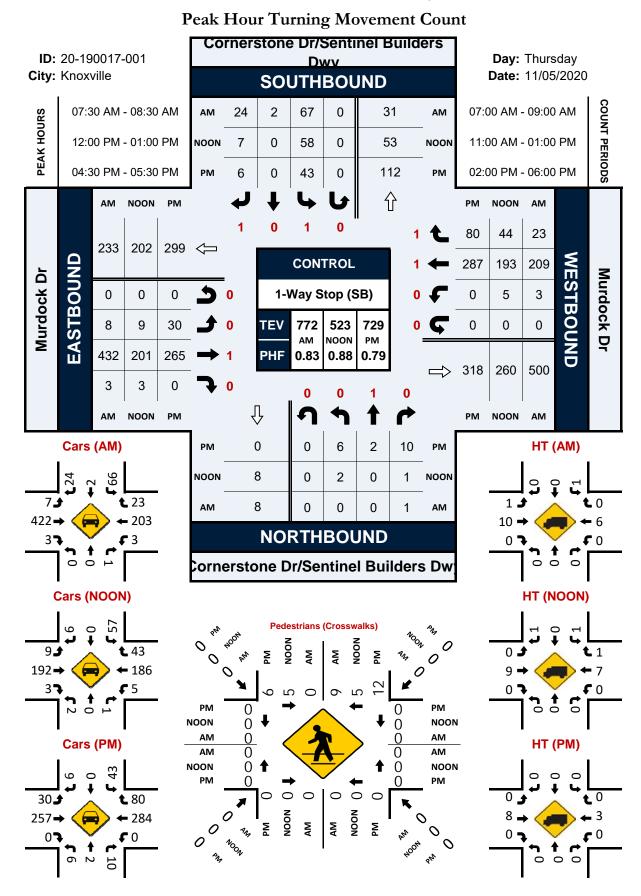
#### **APPENDIX A** TRAFFIC DATA

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#### APPENDIX A - TRAFFIC DATA



## Cornerstone Dr/Sentinel Builders Dwy & Murdock Dr



A-2

: 20-190017-001	ocation: Cornerstone Dr/Sentinel Builders Dwy & Murdock Dr	: Knoxville
Project ID: 20-190017-001	Location: Corner	City: Knoxville

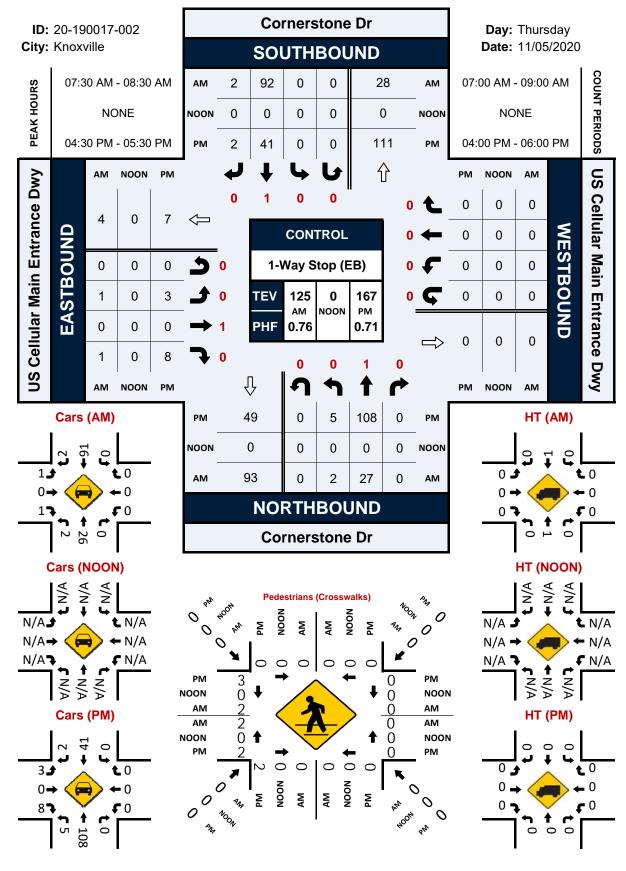
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## Cornerstone Dr & US Cellular Main Entrance Dwy

Peak Hour Turning Movement Count



# Project ID: 20-190017-002 Location: Connerstone Dr & US Cellular Main Entrance Dwy City: Knoxville

Day: Thursday Date: 11/05/2020

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Comerstone Dr ITme         Comerstone Dr Left         Comerstone Dr multiplicational         Commerstone Dr Southbound         Contenstone dr Eastbound         US Cellular Main Entrance Dwy Eastbound           K Hour Kaatysis from 07/30 AM to 093/0 AM K Hour Kaatysis from 07/30 AM to 093/0 AM T 753 AM         0         1         0         0         1         0         0         1         0         1         0         1         0         1         0         1         0         1         0         1         0         0         1         0         0         1         0         0         1         0         0         1         0         0         1         0 <t< th=""><th>City: Knoxville</th><th>City: Knoxville</th><th>e</th><th></th><th></th><th></th><th></th><th>2</th><th>ב</th><th>EAK</th><th>Ĕ</th><th>PEAK HOUKS</th><th>•</th><th></th><th></th><th></th><th></th><th></th><th>Date:</th><th>Date: 11/05/2020</th><th>20</th><th></th></t<>	City: Knoxville	City: Knoxville	e					2	ב	EAK	Ĕ	PEAK HOUKS	•						Date:	Date: 11/05/2020	20	
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PHF         0.733         0.733         0         0         0         200           1 vans         2         6         0         91         2         0         1         0         1         0         2         0         100         20         100         0         100         0         100         0         100         0         0         100         0         0         100         0	% App. Total		93.1	0.0	0.0	100	0.0	97.9	2.1	0.0	100	50.0	0.0	50.0	0.0	100	0.0	0.0	0.0	0.0	0	
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J. Vanal         100.0         96.3         0.0 <th< td=""><td>Cars, PU, Vans</td><td></td><td>26</td><td>0</td><td>0</td><td>28</td><td>0</td><td></td><td>2</td><td>0</td><td>93</td><td>-</td><td>0</td><td>-</td><td>0</td><td>2</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>123</td></th<>	Cars, PU, Vans		26	0	0	28	0		2	0	93	-	0	-	0	2	0	0	0	0	0	123
Trucks         0         1         0         0         1         0 <td>% Cars, PU, Vans</td> <td></td> <td>96.3</td> <td>0.0</td> <td>0.0</td> <td>96.6</td> <td>0.0</td> <td></td> <td>100.0</td> <td>0.0</td> <td>98.9</td> <td>100.0</td> <td>0.0</td> <td>100.0</td> <td>0.0</td> <td>100.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>98.4</td>	% Cars, PU, Vans		96.3	0.0	0.0	96.6	0.0		100.0	0.0	98.9	100.0	0.0	100.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	98.4
Trustal         0.0         3.7         0.0         0.3         3.4         0.0         1.1         0.0	Heavy Trucks		-	0	0	-	0	<del>.</del>	0	0	-	0	0	0	0	0	0	0	0	0	0	.,
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	% App. Total		95.6	0.0	0.0	100	0.0	95.3	4.7		100	27.3	0.0	72.7		100	0.0	0.0	0.0	0.0	0	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	PHF	1				0.706					0.768					0.550	,					0.708
100.0         100.0         0.0         0.0         100.0         0.0         100.0         0.0 <th< td=""><td>Cars, PU, Vans</td><td></td><td>108</td><td>0</td><td>0</td><td>113</td><td>0</td><td>4</td><td>5</td><td></td><td>43</td><td>ε</td><td>0</td><td>8</td><td>0</td><td>=</td><td>0</td><td>0</td><td>0</td><td>0</td><td>0</td><td>167</td></th<>	Cars, PU, Vans		108	0	0	113	0	4	5		43	ε	0	8	0	=	0	0	0	0	0	167
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	% Cars, PU, Vans		100.0	0.0	0.0	100.0	0.0	100.0	100.0		100.0	100.0	0.0	100.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	100.0
0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	Heavy Trucks		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	%Heavy Trucks		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

#### APPENDIX B TRIP GENERATION

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APPENDIX B - TRIP GENERATION INFORMATION



#### KNOX COUNTY LOCAL APARTMENT TRIP GENERATION STUDY

#### PURPOSE

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A Traffic Impact Study (TIS) is currently required in Knox County when a proposed development is projected to generate in excess of 750 trips per day. The determinations of when the threshold is met as well as all subsequent analyses in the TIS are performed using the rates and equations given in the Institute of Transportation Engineers (ITE) Trip Generation Manual. Local governmental agencies rely heavily on the accuracy of these trip generation rates in order to correctly predict the impacts of a proposed development on the transportation system. Therefore, in certain instances, it is logical to verify whether the "national" rates and equations given in the ITE Trip Generation Manual are appropriate for use in a specific local area or region.

The decision was made to study the local trip-making characteristics of apartments because of the discrepancy between the trip generation rates for apartments and single family residential land uses as given in the ITE Trip Generation Manual. While these two land uses are similar in nature, the Trip Generation Manual predicts about three less trips per dwelling unit generated by apartments for the average weekday. Additionally the Trip Generation Manual points out that due to the age of their database, which dates back to the 1960's, "the rates for apartments probably had changed over time". It is also assumed that some of the ITE data had come from larger metropolitan areas with denser development and greater transit use than Knox County, which would contribute to lower trip generation Manual or generate new ones that can be applied to locally proposed apartment developments.

#### PROCEDURE

The procedures recommended by ITE in conducting local trip generation studies were generally followed for this study, along with some important assumptions that have made. ITE has published a proposed recommended practice entitled "Trip Generation Handbook" which specifically outlines procedures for conducting local trip generation studies and establishing new rates and equations.

The first step in the study was to define the number and location of the sites to be studied, as well as the counting methodology. Initially 14 sites were selected, although one apartment complex – the College Park Apartments – was later omitted due to uncharacteristically high traffic generation numbers. The number of sites used in this study far exceeds the recommended minimum amount suggested by ITE, which is five sites. Traffic counts were taken for week-long periods at 15-minute intervals between July 22, 1996 and August 9, 1996 at the access points to the apartment complexes. A Technical Appendix to this report contains the traffic count data collected at each apartment complex.

#### RESULTS

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The traffic count data was analyzed using spreadsheets in order to determine the weighted average rates and regression equations. In order to be considered valid, the local rates and equations for each time period of analysis that were generated must meet certain statistical criteria. First, the standard deviation of the independent variable (dwelling units) should be no more than 110 percent of the weighted average rate; and secondly, the regression equations require a computed coefficient of determination ( $\mathbb{R}^2$ ) value of at least 0.75 before good data fit is indicated. This statistical criteria is met by the local data results, and in fact it often exceeds the level of data fit given by their counterparts in the ITE Trip Generation Manual. Finally, in order to simplify the use of the local data, plots were generated that appear identical to the actual ones in the ITE Trip Generation Manual.

The resulting rates and equations calculated from the local data indicate that the average weekday trip generation of apartments in this area is well above the national rates reported in the ITE manual. For example, the locally computed average rate for number of trips generated during a weekday is 35% higher than the rate given by ITE (increase from 6.63 trips per dwelling unit to 9.03 trips per dwelling unit). The trip generation rates do not increase as much for the AM and PM peak hours however. The local rate is roughly 8% higher for the AM peak, and 16% higher for the PM peak. The plots from the ITE Trip Generation Manual are included in the Technical Appendix for comparison purposes.

#### ASSUMPTIONS MADE

Some important assumptions have been made which may affect the results of the local data that was collected;

- It is important to note that the local trip generation rates were computed for the *total* number of dwelling units in the apartment complex, and <u>not</u> necessarily for the number of occupied dwelling units. There are several reasons why this was done, chiefly because of the need for comparability with the rates given in ITE Trip Generation Manual, as it does not specify whether the dwelling units are occupied. According to ITE procedures the selected sites must only be of "reasonably full occupancy (i.e. at least 85%)". The Apartment Association of Greater Knoxville (AAGK) publishes quarterly reports on occupancy levels of apartment complexes, and the report covering the period of the data collection was reviewed to determine occupancy levels. According to the AAGK report from July 1, 1996 September 30, 1996 all of the apartment complexes surveyed in this study met the minimum 85% occupancy level, with an average occupancy rate for all sites studied of 94%.
- The count data that was collected at each apartment complex was used "raw" meaning that it was not factored for possible daily or seasonal variations. Once again, according to an ITE representative it is not known whether the data used in the Trip Generation Manual was factored or not, so therefore in order to be able to compare

local rates to those in the manual you must assume that count data should not be factored. Additionally, it was felt that apartment complexes would generally not be as susceptible to major seasonal fluctuations as other land uses might be. The local rates were also developed using count data that was collected and averaged over an entire week, which should limit some of the daily variations. Finally, reliable local daily and seasonal variation factors do not truly exist.

#### CONCLUSION

The local apartment study methodology and results were distributed for comment to a group of local transportation professionals who are directly responsible for either preparing or reviewing traffic impact studies. A meeting was held between this group on February 16, 2000 in order to gather comments and discuss the study in greater detail. The following conclusions are based on the discussion and consensus reached at this meeting:

- 1. The trip generation rates and equations meet statistical requirements and resulted from a study that followed accepted procedures; therefore they should be adopted for future use. Furthermore, the rates and equations are recommended for use in reviewing the traffic impact of any development termed as "multi-family", such as townhouse and condominium developments due to their similarity to apartment complexes.
- 2. The Traffic Access and Impact Study Guidelines and Procedures adopted by MPC should be amended with the language that local data should be used when available, which will allow the implementation of these new multi-family trip generation rates.
- 3. The following suggestions were made for future consideration:
  - This study should be updated with data collected from local townhouse and condominium developments in order to further justify the use of the new trip generation rates.
  - A statistical comparison should be made between any newly developed rates and the ITE single family trip generation rates to determine if there is a significant difference. If there is no difference then perhaps ITE single-family rates could be used for any residential development proposed in Knox County.

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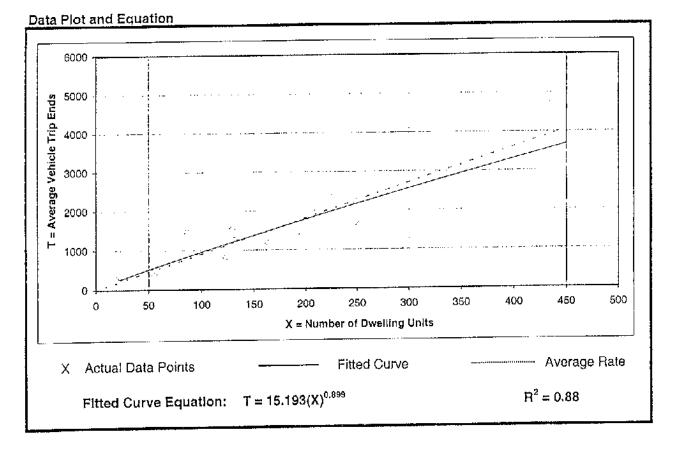
# Local Apartment Trip Generation Study

Average Vehicle Trip Ends vs: Dwelling Units On a: Weekday

Number of Studies:	13
Average Number of Dwelling Units:	193
Directional Distribution:	50% entering, 50% exiting

Trip Generation Per Dwelling Unit

Average Rate	Ranges of Rates	Standard Deviation
9.03	6.59 - 17.41	2.47

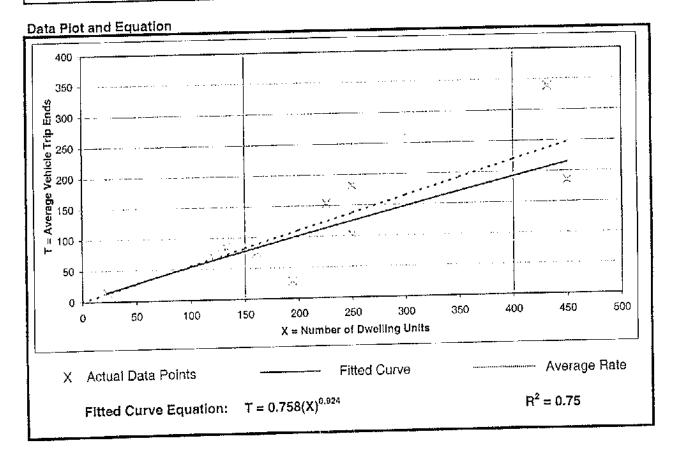


# Local Apartment **Trip Generation Study**

Average Vehicle Trip Ends vs: On a:	Dwelling Units Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.
Number of Studies:	13
Average Number of Dwelling Units:	193
Directional Distribution:	22% entering, 78% exiting

#### eration Per Dwelling Unit

,	The Generation Fer Divening enter	Ranges of Rates	Standard Deviation
	Average Rate		0.18
	0.55	0.14 - 0.78	0,10



B-6 December 1999

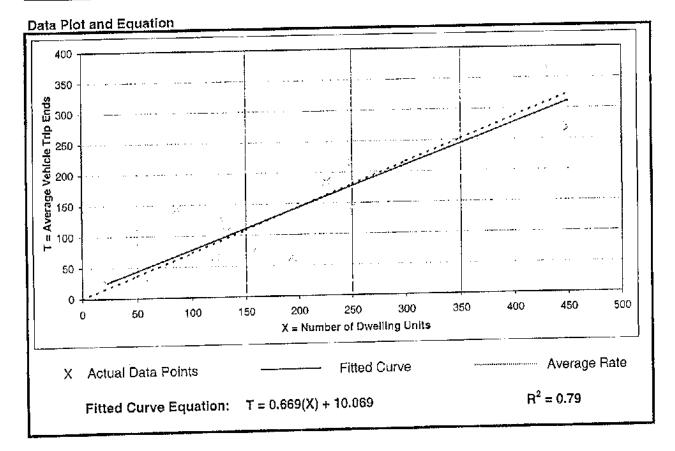
## Knoxville/Knox Co. MPC

# Local Apartment Trip Generation Study

Average Vehicle Trip Ends vs:<br/>On a:Dwelling Units<br/>Weekday,<br/>Peak Hour of Adjacent Street Traffic,<br/>One Hour Between 4 and 6 p.m.Number of Studies:13<br/>193<br/>Directional Distribution:55% entering, 45% exiting

#### Trip Generation Per Dwelling Unit

Augregia Bato	Ranges of Rates	Standard Deviation
Average Rate	0.32 - 1.66	0.25
0.72	0.82 - 1.00	



### **APPENDIX C** CAPACITY ANALYSES

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#### APPENDIX C - CAPACITY ANALYSES



#### **CAPACITY AND LEVEL-OF-SERVICE CONCEPTS**

In a general sense, a roadway is similar to a pipeline or other material carrying conduit in that it has a certain capacity for the amount of material (vehicles) that it can efficiently carry. As the number of vehicles in a given time period gradually increases, the quality of traffic flow gradually decreases. On roadway sections this results in increasing turbulence in the traffic stream, and at intersections it results in increasing stops and delay. As the volumes begin to approach the capacity of the facility, these problems rapidly magnify, with resulting serious levels of congestion, stops, delay, excess fuel consumption, pollutant emissions, etc.

The Transportation Research Board has published the <u>Year 2010 Highway Capacity Manual</u> (HCM2010), which establishes theoretical techniques to quantify the capacity conditions on all types of roadways, intersections, ramps, pedestrian facilities, etc. A basic concept that is applicable to most of these techniques is the idea of level of service (LOS). This concept establishes a rating system that quantifies the quality of traffic flow, as perceived by motorists and/or passengers. The general system is similar to a school grade scale, and is outlined as follows:

Level of Service (LOS)	General Quality of Traffic Flow	Description of Corresponding Conditions
A	Excellent	Roadways – Free flow, high maneuverability Intersections – Very few stops, very low delay
В	Very Good	Roadways – Free flow, slightly lower maneuverability Intersections – Minor stops, low delay
С	Good	Roadways – Stable flow, restricted maneuverability Intersections – Significant stops, significant delay
D	Fair	Roadways – Marginally stable flow, congestion seriously restricts maneuverability Intersections – High stops, long but tolerable delay
Е	Poor	Roadways – Unstable flow*, lower operating speeds, congestion severely restricts maneuverability Intersections – All vehicles stop, very long queues and very long intolerable delay
F	Very Poor	Roadways – Forced flow, stoppages may be lengthy, congestion severely restricts maneuverability Intersections – All vehicles stop, extensive queues and extremely long intolerable delay

\*Unstable flow is such that minor fluctuations or disruptions can result in rapid degradation to LOS F.

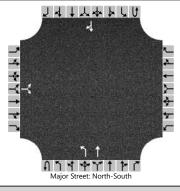
LOS	CONTROL DELAY (S/VEH)							
203	SIGNALIZED	UNSIGNALIZED	ROUNDABOUT					
A	≤10	≤10	≤10					
В	>10-20	>10-15	>10-15					
С	>20-35	>15-25	>15-25					
D	>35-55	>25-35	>25-35					
E	>55-80	>35-50	>35-50					
F	>80	>50	>50					

#### LOS CRITERIA: SIGNALIZED & UNSIGNALIZED INTERSECTIONS

Another measure of intersection capacity that is often used in the evaluation of intersection operations is the volume to capacity (V/C) ratio. This ratio is defined as "the ratio of flow rate to capacity", and is a good measure of how much of an intersection's available capacity has been used up by the analysis volumes. Conversely, it also provides an indication of the reserve capacity available for future growth in traffic volumes.

The Intersection Capacity Utilization (ICU) is another measure that expresses a value similar to the V/C ratio. Specifically, the ICU method "sums the amount of the time required to serve all movements at saturation for a given cycle length and divides by that reference cycle length." The ICU is considered a more accurate measure of volume to capacity conditions for a signalized intersection, primarily because it accounts for the effects of the signal timing on intersection capacity.

General Information		Site Information	
Analyst	ВЈН	Intersection	Cornerstone at Access
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County
Date Performed	11/24/2020	East/West Street	Site Access
Analysis Year	2020	North/South Street	Cornerstone Drive
Time Analyzed	AM Peak	Peak Hour Factor	0.76
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	2020 Existing		



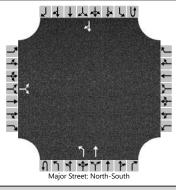
	1								1				1	<b>6</b>			
Approach			ound			West				_	bound				bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	1	1	0	0	0	1	0	
Configuration			LR							L	Т					TR	
Volume (veh/h)		1		1						2	34				110	2	
Percent Heavy Vehicles (%)		3		3						3							
Proportion Time Blocked																	
Percent Grade (%)		(	0												°		
Right Turn Channelized																	
Median Type   Storage				Left	Only								1				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.43		6.23						4.13							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.53		3.33						2.23							
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)			3							3							
Capacity, c (veh/h)			832							1428							
v/c Ratio			0.00							0.00							
95% Queue Length, Q <sub>95</sub> (veh)			0.0							0.0							
Control Delay (s/veh)			9.3							7.5							
Level of Service (LOS)			A							А							
Approach Delay (s/veh)		9	.3						0.4								
Approach LOS			Ą														

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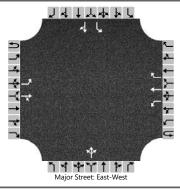
HCS7 Two-Way Stop-Control Report							
General Information		Site Information					
Analyst	ВЈН	Intersection	Cornerstone at Access				
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County				
Date Performed	11/24/2020	East/West Street	Site Access				
Analysis Year	2020	North/South Street	Cornerstone Drive				
Time Analyzed	PM Peak	Peak Hour Factor	0.71				
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25				
Project Description	2020 Existing						
Lanes							



Venicie volumes and Adj	1					<b>14</b> /2-21			1	NL: al-	le e - e el			C	le e se el		
Approach		1	ound				bound			_	bound			1	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	10	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	1	1	0	0	0	1	0	
Configuration			LR							L	Т					TR	
Volume (veh/h)		4		10						6	128				49	2	
Percent Heavy Vehicles (%)		3		3						3							
Proportion Time Blocked																	
Percent Grade (%)			0														
Right Turn Channelized																	
Median Type   Storage				Left	Only								1				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.43		6.23						4.13							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.53		3.33						2.23							
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)			20							8							
Capacity, c (veh/h)			895							1522							
v/c Ratio			0.02							0.01							
95% Queue Length, Q <sub>95</sub> (veh)			0.1							0.0							
Control Delay (s/veh)			9.1							7.4							
Level of Service (LOS)			Α							А							
Approach Delay (s/veh)		9	.1						0.3								
Approach LOS			4														

Generated: 2/23/2021 11:10:12 AM

HCS7 Two-Way Stop-Control Report								
General Information		Site Information						
Analyst	ВЈН	Intersection	Murdock at Cornerstone					
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County					
Date Performed	11/24/2020	East/West Street	Murdock Drive					
Analysis Year	2020	North/South Street	Cornerstone Drive					
Time Analyzed	AM Peak	Peak Hour Factor	0.83					
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25					
Project Description	2020 Existing							
Lanes								

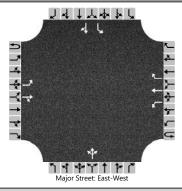


Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	Т	R			LTR			L		TR
Volume (veh/h)		10	518	4		4	251	26		0	0	1		80	2	29
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										(	0				0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		12				5					1			96		37
Capacity, c (veh/h)		1220				948					482			344		653
v/c Ratio		0.01				0.01					0.00			0.28		0.06
95% Queue Length, Q <sub>95</sub> (veh)	Í	0.0				0.0					0.0			1.1		0.2
Control Delay (s/veh)		8.0				8.8					12.5			19.5		10.8
Level of Service (LOS)		A				A					В			С		В
Approach Delay (s/veh)		0	.2		0.1 12.5 17.1					17.1						
Approach LOS											В		С			

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HCSTM TWSC Version 7.9 Murdock at Cornerstone Existing AM.xtw Generated: 2/23/2021 11:11:02 AM

HCS7 Two-Way Stop-Control Report								
General Information		Site Information						
Analyst	ВЈН	Intersection	Murdock at Cornerstone					
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County					
Date Performed	11/24/2020	East/West Street	Murdock Drive					
Analysis Year	2020	North/South Street	Cornerstone Drive					
Time Analyzed	PM Peak	Peak Hour Factor	0.79					
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25					
Project Description 2020 Existing								
Lanes								

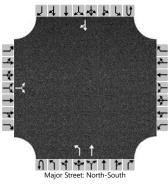


Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	Т	R			LTR			L		TR
Volume (veh/h)		36	318	0		0	344	96		7	2	12		52	0	7
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										(	0				0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		46				0					27			66		9
Capacity, c (veh/h)		1009				1151					428			353		619
v/c Ratio		0.05				0.00					0.06			0.19		0.01
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0					0.2			0.7		0.0
Control Delay (s/veh)		8.7				8.1					14.0			17.5		10.9
Level of Service (LOS)		A				A					В			С		В
Approach Delay (s/veh)		0	.9		0.0 14.0 16.7					6.7	-					
Approach LOS	1										В		С			

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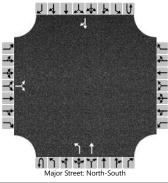
HCSTM TWSC Version 7.9 Murdock at Cornerstone Existing PM.xtw Generated: 2/23/2021 11:11:46 AM

HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	ВЈН	Intersection	Cornerstone at Access							
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County							
Date Performed	11/24/2020	East/West Street	Site Access							
Analysis Year	2022	North/South Street	Cornerstone Drive							
Time Analyzed	AM Peak	Peak Hour Factor	0.76							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description	2022 Background									
Lanes	Lanes									



Approach		Eastb	ound			West	oound			North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	1	1	0	0	0	1	0	
Configuration			LR							L	Т					TR	
Volume (veh/h)		1		1						2	35				114	2	
Percent Heavy Vehicles (%)		3		3						3							
Proportion Time Blocked																	
Percent Grade (%)			0														
Right Turn Channelized																	
Median Type   Storage				Left	Only								1				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.43		6.23						4.13							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.53		3.33						2.23							
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)			3							3							
Capacity, c (veh/h)			827							1422							
v/c Ratio			0.00							0.00							
95% Queue Length, Q <sub>95</sub> (veh)			0.0							0.0							
Control Delay (s/veh)			9.4							7.5							
Level of Service (LOS)			A							A							
Approach Delay (s/veh)		9	.4						0.4								
Approach LOS			4														

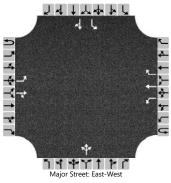
HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	ВЈН	Intersection	Cornerstone at Access							
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County							
Date Performed	11/24/2020	East/West Street	Site Access							
Analysis Year	2022	North/South Street	Cornerstone Drive							
Time Analyzed	PM Peak	Peak Hour Factor	0.71							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description	2022 Background									
Lanes										



•																	
Approach		Eastk	ound			West	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	1	1	0	0	0	1	0	
Configuration			LR							L	Т					TR	
Volume (veh/h)		4		10						6	133				51	2	
Percent Heavy Vehicles (%)		3		3						3							
Proportion Time Blocked																	
Percent Grade (%)			0														
Right Turn Channelized																	
Median Type   Storage				Left	Only								1				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.43		6.23						4.13							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.53		3.33						2.23							
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)			20							8							
Capacity, c (veh/h)			890							1518							
v/c Ratio			0.02							0.01							
95% Queue Length, Q <sub>95</sub> (veh)			0.1							0.0							
Control Delay (s/veh)			9.1							7.4							
Level of Service (LOS)			A							A							
Approach Delay (s/veh)		9	.1						0.3								
Approach LOS			A														



HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	ВЈН	Intersection	Murdock at Cornerstone							
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County							
Date Performed	11/24/2020	East/West Street	Murdock Drive							
Analysis Year	2022	North/South Street	Cornerstone Drive							
Time Analyzed	AM Peak	Peak Hour Factor	0.83							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	2022 Background									
Lanes										



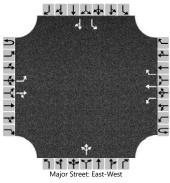
Approach		Eastb	ound			West	bound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	т	R			LTR			L		TR
Volume (veh/h)		10	539	4		4	261	27		0	0	1		83	2	30
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										(	D				0	
Right Turn Channelized						N	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		12				5					1			100		39
Capacity, c (veh/h)		1206				928					466			332		642
v/c Ratio		0.01				0.01					0.00			0.30		0.06
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0			1.2		0.2
Control Delay (s/veh)		8.0				8.9					12.7			20.4		11.0
Level of Service (LOS)		А				A					В			С		В
Approach Delay (s/veh)		0	.1		0.1				12.7				17.8			
Approach LOS										I	В		С			

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HCSTM TWSC Version 7.9 Murdock at Cornerstone Background AM.xtw

Generated: 2/23/2021 11:22:52 AM

HCS7 Two-Way Stop-Control Report										
General Information Site Information										
Analyst	ВЈН	Intersection	Murdock at Cornerstone							
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County							
Date Performed	11/24/2020	East/West Street	Murdock Drive							
Analysis Year	2022	North/South Street	Cornerstone Drive							
Time Analyzed	PM Peak	Peak Hour Factor	0.79							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	2022 Background									
Lanes										



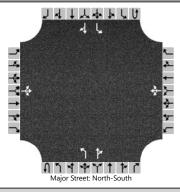
Approach		Eastb	ound			West	bound			North	bound			Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0		
Configuration		L		TR		L	Т	R			LTR			L		TR		
Volume (veh/h)		37	331	0		0	358	100		7	2	12		54	0	7		
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3		
Proportion Time Blocked																		
Percent Grade (%)										(	C				0			
Right Turn Channelized						Ν	10											
Median Type   Storage				Left	Only								1					
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2		
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33		
Delay, Queue Length, an	d Leve	l of Se	ervice											<u> </u>				
Flow Rate, v (veh/h)		47				0					27			68		9		
Capacity, c (veh/h)		989				1135					413			341		605		
v/c Ratio		0.05				0.00					0.06			0.20		0.01		
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0					0.2			0.7		0.0		
Control Delay (s/veh)		8.8				8.2					14.3			18.2		11.0		
Level of Service (LOS)		A				A					В			С		В		
Approach Delay (s/veh)		0	.9		0.0 14.3						17.4							
Approach LOS											В				С			

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HCSTM TWSC Version 7.9 Murdock at Cornerstone Background PM.xtw

Generated: 2/23/2021 11:23:50 AM

HCS7 Two-Way Stop-Control Report								
General Information		Site Information						
Analyst	ВЈН	Intersection	Cornerstone at Access					
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County					
Date Performed	11/24/2020	East/West Street	Site Access					
Analysis Year	2022	North/South Street	Cornerstone Drive					
Time Analyzed	AM Peak	Peak Hour Factor	0.76					
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25					
Project Description	2022 Combined							
Lanes								

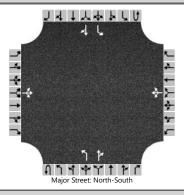


venicle volumes and Adj	ustine													<b>6</b>		
Approach		1	ound				oound				bound				bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	1	0	0	1	1	0
Configuration			LTR				LTR			L		TR		L		TR
Volume (veh/h)		1	0	1		30	0	4		2	33	8		1	114	2
Percent Heavy Vehicles (%)		3	3	3		3	3	3		3				3		
Proportion Time Blocked																
Percent Grade (%)			0				0									
Right Turn Channelized																
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.23		7.13	6.53	6.23		4.13				4.13		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)			3				45			3				1		
Capacity, c (veh/h)			808				764			1422				1545		
v/c Ratio			0.00				0.06			0.00				0.00		
95% Queue Length, Q <sub>95</sub> (veh)			0.0				0.2			0.0				0.0		
Control Delay (s/veh)			9.5				10.0			7.5				7.3		
Level of Service (LOS)			A				В			A				A		
Approach Delay (s/veh)		9	.5			1(	).0			0	.4			0	.1	
Approach LOS			4				В									

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HCS TW TWSC Version 7.8.5 Cornerstone at Site Access Combined AM.xtw Generated: 11/24/2020 5:35:13 PM

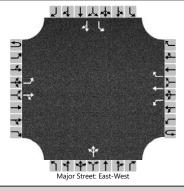
	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	ВЈН	Intersection	Cornerstone at Access
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County
Date Performed	11/24/2020	East/West Street	Site Access
Analysis Year	2022	North/South Street	Cornerstone Drive
Time Analyzed	PM Peak	Peak Hour Factor	0.71
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	2022 Combined		
Lanes			



Assessed	1		ound			\ <b>A</b> /a at	bound			N a set la	bound			Cauth	bound	
Approach	-	1								_				1		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	1	0	0	1	1	0
Configuration			LTR				LTR			L		TR		L		TR
Volume (veh/h)		4	0	10		25	0	4		6	135	30		4	51	2
Percent Heavy Vehicles (%)		3	3	3		3	3	3		3				3		
Proportion Time Blocked																
Percent Grade (%)			0				0									
Right Turn Channelized																
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.23		7.13	6.53	6.23		4.13				4.13		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.33		3.53	4.03	3.33		2.23				2.23		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)			20				41			8				6		
Capacity, c (veh/h)			860				671			1518				1329		
v/c Ratio			0.02				0.06			0.01				0.00		
95% Queue Length, Q <sub>95</sub> (veh)			0.1				0.2			0.0				0.0		
Control Delay (s/veh)			9.3				10.7			7.4				7.7		
Level of Service (LOS)			A				В			А				A		
Approach Delay (s/veh)		9	.3			1(	).7			0	.3			0	.5	
Approach LOS			Ą				В									

Generated: 11/24/2020 5:36:48 PM

	HCS7 Two-Way Sto	p-Control Report	
General Information		Site Information	
Analyst	ВЈН	Intersection	Murdock at Cornerstone
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County
Date Performed	11/24/2020	East/West Street	Murdock Drive
Analysis Year	2022	North/South Street	Cornerstone Drive
Time Analyzed	AM Peak	Peak Hour Factor	0.83
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	2022 Combined		
Lanes			

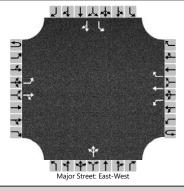


Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	т	R			LTR			L		TR
Volume (veh/h)		13	543	4		4	274	32		0	0	1		100	2	43
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)											0				0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice											<u>.</u>		
Flow Rate, v (veh/h)		16				5					1			120		54
Capacity, c (veh/h)		1184				924					463			324		648
v/c Ratio		0.01				0.01					0.00			0.37		0.08
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0			1.7		0.3
Control Delay (s/veh)		8.1				8.9					12.8			22.6		11.1
Level of Service (LOS)		Α				A					В			С		В
Approach Delay (s/veh)		0	.2			0	.1			12	2.8			19	9.0	
Approach LOS											В				С	

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HCSTM TWSC Version 7.9 Murdock at Cornerstone Combined AM.xtw Generated: 2/23/2021 11:14:37 AM

	HCS7 Two-Way Sto	p-Control Report	
General Information		Site Information	
Analyst	ВЈН	Intersection	Murdock at Cornerstone
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County
Date Performed	11/24/2020	East/West Street	Murdock Drive
Analysis Year	2022	North/South Street	Cornerstone Drive
Time Analyzed	PM Peak	Peak Hour Factor	0.79
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	2022 Combined		
Lanes			

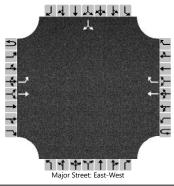


	1									NI 41			1	<b>C</b> 11		
Approach			ound				bound				bound			1	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	Т	R			LTR			L		TR
Volume (veh/h)		50	344	0		0	369	117		7	2	12		68	0	18
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										(	) )				0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	Τ	63				0					27			86		23
Capacity, c (veh/h)		960				1119					379			316		594
v/c Ratio		0.07				0.00					0.07			0.27		0.04
95% Queue Length, Q <sub>95</sub> (veh)		0.2				0.0					0.2			1.1		0.1
Control Delay (s/veh)		9.0				8.2					15.2			20.6		11.3
Level of Service (LOS)		A				A					С			С		В
Approach Delay (s/veh)		1	.1			0	.0			15	5.2			18	3.6	
Approach LOS										(	C				С	

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HCSTM TWSC Version 7.9 Murdock at Cornerstone Combined PM.xtw Generated: 2/23/2021 11:15:22 AM

	HCS7 Two-Wa	ay Stop-Control Report	
General Information		Site Information	
Analyst	ВЈН	Intersection	Murdock at Site Access
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County
Date Performed	11/24/2020	East/West Street	Murdock Drive
Analysis Year	2022	North/South Street	Site Access
Time Analyzed	AM Peak	Peak Hour Factor	0.83
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Combined 2022		
Lanes			

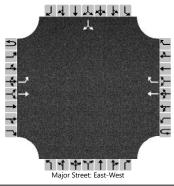


Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0
Configuration		L	Т				Т	R							LR	
Volume (veh/h)		4	640				294	11						38		13
Percent Heavy Vehicles (%)		3												3		3
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.13												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.23												3.53		3.33
Delay, Queue Length, an	d Leve	l of Se	ervice								<u>.</u>			<u>.</u>		
Flow Rate, v (veh/h)		5													61	$\square$
Capacity, c (veh/h)		1186													394	
v/c Ratio		0.00													0.16	
95% Queue Length, Q <sub>95</sub> (veh)		0.0													0.5	
Control Delay (s/veh)		8.0													15.8	
Level of Service (LOS)		A													С	
Approach Delay (s/veh)		0	.0								-			. 1!	5.8	
Approach LOS															С	

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Generated: 2/23/2021 11:15:58 AM

	HCS7 Two-W	ay Stop-Control Report	
General Information		Site Information	
Analyst	ВЈН	Intersection	Murdock at Site Access
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County
Date Performed	11/24/2020	East/West Street	Murdock Drive
Analysis Year	2022	North/South Street	Site Access
Time Analyzed	PM Peak	Peak Hour Factor	0.79
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Combined 2022		
Lanes			



Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0
Configuration		L	Т				Т	R							LR	
Volume (veh/h)		13	411				475	38						30		11
Percent Heavy Vehicles (%)		3												3		3
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								1			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.13												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.23												3.53		3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		16													52	
Capacity, c (veh/h)		932													381	
v/c Ratio		0.02													0.14	
95% Queue Length, Q <sub>95</sub> (veh)		0.1													0.5	
Control Delay (s/veh)		8.9													15.9	
Level of Service (LOS)		A													С	
Approach Delay (s/veh)		0	.3											1!	5.9	
Approach LOS	1												İ		С	_

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Generated: 2/23/2021 11:16:33 AM

# **APPENDIX D** SIGNAL WARRANTS

APPENDIX D - SIGNAL WARRANT SPREADSHEETS



#### **TRAFFIC SIGNAL WARRANT ANALYSIS - VOLUME WARRANTS**

Intersection : City or County : Murdock Drive at Cornerstone Drive

State

Knox County

Tennessee

Date of Count: 5-Nov-20 Day of Week of Count: Thursday

Warrant #2

(Four Hour Vols.)

Volume ----

0

0

0

250

230

240

160

180

160

0

0

0

Warrant =

reductions due to Covid.

T. Darcy Sullivan, P.E.

0

200

200

Warrant Percent

of

\*\*\*\*\*

46

44

\*\*\*\*\*

\*\*\*\*\*

28

34

24

38

26

33

\*\*\*\*\*

\*\*\*\*\*

\*\*\*\*\*

0

Tennessee Transportation Assistance Program (TTAP)

\*\*\*\*\* Major Street volume is so low that no Minor Street warrant exists

Warranting Volumes

From MUTCD Fig. 4-7

Total Hours Meeting

Warrant Me No

\*\*\*\*\*

Warrant

Major Street . . 1 Number of Lanes: Minor Street . . . 1

Total       Volume       Total       Volume       Total       Major       Minor       Major       Major       Minor       <			Majo	r Street		Minoi	Street		Warra (8 Hr N	ant #1A lin. Vol.)	Warran (8 Hr Int		Corr (Warra
6:00 am       0 </th <th></th> <th></th> <th></th> <th></th> <th>Total</th> <th>Volume</th> <th>Total</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>Perce  Maj</th>					Total	Volume	Total						Perce  Maj
7:00       413       223       636       636       91       91       127       61       85       121         9:00 am       0 <td< td=""><td></td><td>0</td><td>0</td><td>0</td><td> 0</td><td></td><td></td><td></td><td></td><td>- 0</td><td></td><td>0</td><td> 0</td></td<>		0	0	0	0					- 0		0	0
9:00 am       0 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td></td> <td></td> <td></td> <td>-</td> <td>-</td> <td>106</td>							-				-	-	106
10:00       0 <td>8:00</td> <td>379</td> <td>253</td> <td>632</td> <td>632</td> <td>88</td> <td>88</td> <td></td> <td>126</td> <td>59</td> <td>84</td> <td>117</td> <td>10</td>	8:00	379	253	632	632	88	88		126	59	84	117	10
11:00       205       301       506       506       71       71       101       47       67       95         12:00 noon       256       290       546       546       78       78       109       52       0							-					-	(
12:00 noon       256       290       546       546       78       78       109       52       73       104       0			-				-		-		-	-	(84
1:00       0									101				
2:00       240       288       528       528       58       58       106       39       70       77       77         3:00 pm       287       464       751       751       60       60       150       40       136       31       91       61       99       75       75 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td>-</td><td></td><td>9</td></t<>							-				-		9
4:00       311       371       682       682       46       46       46       136       31       91       61       99       71         6:00 pm       0       <		-					-		-	-	-		8
5:00       307       436       743       743       53       53       149       35       99       71       149       149       35       99       71       149       149       149       35       99       71       149       149       35       99       71       149       149       35       99       71       149       149       35       99       71       149       149       35       99       71       149       149       35       99       71       149       149       35       99       71       149       149       35       99       71       149       149       149       35       99       71       149       149       149       35       99       71       149	3:00 pm			751	751	60	60		150	40	100	80	12
6:00 pm       0 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td></td> <td></td> <td></td> <td>-</td> <td>-</td> <td>114</td>							-				-	-	114
7:00       0	5:00	307	436	743	743	53	53		149	35	99	71	124
8:00       0			-				-				-	-	(
Note:       No adjus ment made         -       Where more than one minor approach exists use the higher approach volume         .       Number of hours shown is the minimum meeting the MUTCD requirements. Additional hours outside of the count period may meet the MUTCD specified volume levels.       Total Hours Meeting Warrant = 0. Warrant Met No       Warrant Met No         Comments:       (include any information which may be useful to the reviewer)       Major Street = Murdock Drive Minor Street = Cornerstone Drive All volumes included.       All approaches considered single Raw traffic data factored by 1.2 to the second street and the second st													(
Note:       No adjus ment made         -       Where more than one minor approach exists use the higher approach volume         .       Number of hours shown is the minimum meeting the MUTCD requirements. Additional hours outside of the count period may meet the MUTCD specified volume levels.       Total Hours Meeting Warrant = 0. Warrant Met No       Warrant Met No         Comments:       (include any information which may be useful to the reviewer)       Major Street = Murdock Drive Minor Street = Cornerstone Drive All volumes included.       All approaches considered single Raw traffic data factored by 1.2 to the second street and the second street approaches considered single Raw traffic data factored by 1.2 to the second street approaches considered street approaches included.								ŀ	Warranting	Volumes	Warranting	Volumes	Warra
approach volume       Warrant = 0.       Warrant Met       Warrant = 0.       Warrant Met       Warrant = 0.       Warrant Met       Warrant Warrant Warrant Met       Warr									500	150			60
Number of hours shown is the minimum meeting the MUTCD requirements. Additional hours outside of the count period may meet the MUTCD specified volume levels.       Warrant Met No       Warrant Met No       Warrant Met No         Comments:       (include any information which may be useful to the reviewer)       Major Street = Murdock Drive Minor. Street = Cornerstone Drive All volumes included.       All approaches considered single Raw traffic data factored by 1.2 to the reviewer.				ne minor	r approach e	xists use th	ne higher			5			Total I Warra
meet the MUTCD specified volume levels.         Comments:       (include any information which may be useful to the reviewer)         Major Street = Murdock Drive         Minor Street = Comerstone Drive         All volumes included.				own is t	he minimum	meeting th	e MUTCD						Warra
Major Street = Murdock Drive       All approaches considered single         Minor Street = Cornerstone Drive       Raw traffic data factored by 1.2 to         All volumes included.       Provide the strength of the strengt of the strength of the strength of the strength of the strengt of		•					int period may						
Major Street = Murdock Drive Minor Street = Cornerstone Drive All volumes included. Raw traffic data factored by 1.2 to Raw traffic data factored by 1.2 to	Comments:	(include	any infor	mation	which may b	e useful to	the reviewer)			I	All approache	es considere	d single lar
All volumes included.													
Analysis Prepared by: CANNON AND CANNON INC. Date: 12/09/20 Deve					ne Drive								
Analysis Prenared by: CANNON AND CANNON INC. Deve													
Brian J. Haas, P.E., PTOE Time: 13:41 Distri	Analysis Prei	pared by:	CANNO	N AND	CANNON, I	NC.				Date:	12/09/20		Develo

D-2

No

Warrant #3

(Peak Hour Vols.)

Warrant Percent

Volume Warrant

0

340

340

0

0

410

390

400

290

320

290

0

0

0

Warrant =

Warranting Volumes

From MUTCD Fig. 4-5

Total Hours Meeting

Warrant Met No

0

of

\*\*\*\*\*

\*\*\*\*\*

\*\*\*\*

17

20

15

21

14

18

\*\*\*\*\*

\*\*\*\*\*

\*\*\*\*\*

0

VC/R1

\*\*\*\*\*

27

26

1.00

#### **TRAFFIC SIGNAL WARRANT ANALYSIS - VOLUME WARRANTS**

2022 Background

Intersection : City or County : State

Knox County

Tennessee

Murdock Drive at Cornerstone Drive Date of Count:

Day of Week of Count: Thursday

Are warranting volumes to be adjusted for speeds or built up area?..... 

		Majo	or Street		Minor	Street	(8)		ant #1A lin. Vol.)	Warrant (8 Hr Int		)	Combin (Warrants	ation 1A & 1B)	Warrar (Four Hou		Warrar (Peak Hor	
Time	A	ctual Volu	ume	Adjusted Total	Actual Volume	Adjusted Total	Pe		of Warrant	Percent of \		,	Percent o	f Warrant	Warrant	Percent	Warrant	Pe
Beginning	App #1	App #2	Total	Volum,	-	Volum,	М	lajor	Minor	Major	Minor		Major	Minor	Volume	Warrant	Volume	W
6:00 am	0	0	0	0	0	0		0	0	0	0		0	0	0	****	0	**
7:00 8:00	430 394	232 263	662 657	662 657	95 92	95 92		32 31	63 61	88 88	127 123		110 110	79 77	190 190	50 48	330 330	
9:00 am	0	0	0	0	0	0		0	0	0	0		0	0	0	****	0	**
10:00 11:00	0 213	0 313	0 526	0 526	0 74	0 74	1	0 05	0 49	0 70	0 99		0 88	0 62	0 240	***** 31	0 400	**
12:00 noon	266	302	568	568	81	81	1	14	54	76	108		95	68	220	37 *****	380	**
1:00 2:00	0 250	0 300	0 550	0 550	0 60	0 60	1	0 10	0 40	0 73	0 80		0 92	0 50	0 230	26	0 390	*
3:00 pm 4:00	298 323	483 386	781 709	781 709	62 48	62 48		56 42	41 32	104 95	83 64		130 118	52 40	150 170	41 28	280 310	
4.00 5:00	319	453	709	709	40 55	48 55		42 54	32 37	95 103	73		129	40 46	150	20 37	280	
6:00 pm 7:00	0	0	0 0	0 0	0	0		0 0	0	0	0 0		0	0 0	0	****	0	**
8:00	0	0	0	0	0	0		0	0	0	0		0	0	0	****	0	**
Note: . N	No adjus m	ent made	2					ranting 00	Volumes 150	Warranting 750	olumes/ 75		Warranting 600	Volumes 120	Warranting From MUTC	g Volumes D Fig. 4-7	Warrantin From MUTC	
<i>,</i>	Where mo	re than or		approach e	exists use t	he higher	Tota		s Meeting	Total Hours Warrant =	Meeting		Total Hours Warrant =	-	Total Hours Warrant =	°	Total Hours Warrant =	
. 1	approach Number of		own is t	ne minimum	meeting th	ne MUTCD		rant = rant Me	0. et No	Warrant = Warrant Met	0 No	•	Warrant = Warrant Me		Warrant = Warrant N		Warrant = Warrant N	/le <sup>:</sup>
				ours outside I volume lev		unt period may										ijor Street vol linor Street w	ume is so low t arrant exists	hat r
Comments:	(include	any infor	mation	which may b	e useful to	the reviewer)												
	,	treet = M								All approache Volumes sho				inual growth	from Year 202	0 to Year 202	22.	
		treet = Co mes inclue		ne Drive														
Analysis Pre	nared by:	CANNO		CANNON, I	NC				Date	: 12/09/20			Developed b	v <sup>.</sup> T Dar	rcy Sullivan, P.	F		

#### **TRAFFIC SIGNAL WARRANT ANALYSIS - VOLUME WARRANTS**

Intersection : City or County : State

Knox County

Tennessee

Murdock Drive at Cornerstone Drive Date of Count:

Day of Week of Count: Thursday

2022 Combined

Are warranting volumes to be adjusted for speeds or built up area?..... 

 Alle wairanting volumes to be adjusted to specie of specie of specie of specie of species 
No 1.00

		Majo	or Street		Mino	Street	(8	Warra 8 Hr M	int #1A in. Vol.)	Warran (8 Hr Int		on)	Combir (Warrants	ation s 1A & 1B)	Warrar (Four Hou		Warrar (Peak Hou	
Time	A	ctual Volu	ume	Adjusted Total	Actual Volume	Adjusted Total	P		f Warrant	Percent of				of Warrant		,	Warrant	
Beginning	App #1	App #2	Total	Volum,	-	Volum,		Major	Minor	Major	Min	or	Major	Minor	Volume	Warrant	Volume	W
6:00 am	0	0	0	0	0	0		0	0	0	(	-	0	0	0	****	0	**
7:00 8:00	437 401	250 281	687 682	687 682	125 122	125 122		137 136	83 81	92 91	167 163		115 114	104 102	180 180	69 68	320 320	
9:00 am	0	0	0	0	0	0		0	0	0	(		0	0	0	****	0	*
10:00 11:00	0 220	0 331	0 551	0 551	0 104	0 104		0 110	0 69	0 73	( 139		0 92	0 87	0 230	***** 45	0 390	*
12:00 noon	273	320	593	593	111	111		119	74	79	148	3	99	93	210	53	360	
1:00 2:00	0 276	0 328	0 604	0 604	0 85	0 85		0 121	0 57	0 81	( 113		0 101	0 71	0 210	***** 40	0 360	*
3:00 pm	324	511	835	835	87	87		167	58	111	116	6	139	73	140	62	260	
4:00 5:00	349 345	414 481	763 826	763 826	73 80	73 80		153 165	49 53	102 110	97 107		127 138	61 67	160 140	46 57	290 260	
6:00 pm	0	0	0	0	0	0		0	0	0	(	)	0	0	0	****	0	*:
7:00 8:00	0	0 0	0 0	0 0	0	0 0		0 0	0 0	0 0	(	-	0 0	0 0	0 0	***** ****	0 0	*
									Volumes	Warranting			Warranting		Warranting		Warrantin	
,	No adjus m Where mor			· approach e	exists use t	he higher		500 tal Hours	150 Meeting	750 Total Hours	75 Meetin		600 Total Hours	120 s Meeting	From MUTC Total Hours		From MUTC Total Hours	
. 1	approach Number of		own is t	ne minimum	meetina tl	ne MUTCD		arrant = arrant Me	0. et No	Warrant = Warrant Me		2. Io	Warrant = Warrant Me	2. et No	Warrant = Warrant N	0 le No	Warrant = Warrant N	Лe
				ours outside I volume lev		unt period may	1					U	l			ijor Street vol linor Street w	lume is so low th	hat
	meet me		specified		013.										IV.			_
Comments:	(include	any infor	rmation	which may b	e useful to	the reviewer)												
	,	treet = M									wn are	2022 0	combined with					
		treet = Co nes inclu		ne Drive													am, 11am, and <sup>.</sup> pm, 4pm, and 5j	
Analysis Pre	pared by:	CANNO		CANNON I	NC.				Date	e: 12/09/20			Developed b	ov: T. Da	rcy Sullivan, P.	E.		
				P.E., PTOE					Time			1	Distributed b		essee Transpol		ance Program (	TT.

### **APPENDIX E** TURN LANE WARRANTS

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APPENDIX E - TURN LANE WARRANT SHEETS



#### Cornerstone Drive at Site Access

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#### TABLE 4B

### RIGHT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 35 MPH OR LESS

RIGHT-TURN	THRO	UGH VOLUM	E PLUS LEI	FT-TURN	VOLUME	; *. 
VOLUME	< 100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399
Fewer Than 25 25 - 49 50 - 99	$\bigwedge$	<				
100 - 149 150 - 199	AM	l Peak	PM Pea	 ak _	   	
200 - 249 250 - 299						Yes
300 - 349 350 - 399				Yes	Yes Yes	Yes Yes
400 - 449 450 - 499			Yes Yes	Yes Yes	Yes Yes	Yes Yes
500 - 549 550 - 599		Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
600 or More	Yes	Yes	Yes	Yes	Yes	Yes

RIGHT-TURN	THRO	UGH VOLUM	E PLUS LEI	T-TURN	VOLUMI	<u>;</u> *
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 600	+ / > 600
Fewer Than 25 25 - 49 50 - 99					Yes	Yes Yes
100 - 149 150 - 199			Yes	Yes Yes	Yes Yes	Yes Yes
200 - 249 250 - 299	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
300 - 349 350 - 399	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
400 - 449 450 - 499	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
500 - 549 550 - 599	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
600 or More	Yes	Yes	Yes	Yes	Yes	Yes

\* Or through volume only if a left-turn lane exists.

AM Peak:

- Right Turn Volume = 8

- Through Volume = 33

Right turn lane IS NOT warranted.

PM Peak:

- Right Turn Volume = 30

- Through Volume = 135

#### TABLE 5B

### RIGHT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *									
VOLUME	<100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399				
Fewer Than 25 25 - 49 50 - 99		AM Peak		~						
100 - 149 150 - 199										
200 - 249 250 - 299					Yes	Yes Yes				
300 - 349 350 - 399			Yes	Ves Yes	Yes Yes	Yes Yes				
400 - 149 450 - 499	<u> </u>	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes				
500 - 549 550 - 599	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes				
600 or More	Yes	Yes	Yes	Yes	Yes	Yes				

RIGHT-TURN	THR	OUGH VOLU	ME PLUS LI	EFT-TURN	VOLUM	E *	
VOLUME	350 - 399 400 - 449		450 - 499	500 - 549	550 - 600	+ / > 600	
Fewer Than 25 25 - 49 50 - 99	F	PM Peak	$\rightarrow$	Yes	Yes Yes	Yes Yes	
100 - 149 150 - 199		Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
200 - 249 250 - 299	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
300 - 349 350 - 399	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
400 - 449 450 - 499	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
500 - 549 550 - 599	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
600 or More	Yes	Yes	Yes	Yes	Yes	Yes	

\* Or through volume only if a left-turn lane exists.

AM Peak:

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Right Turn Volume = 11Through Volume = 294

Right turn lane IS NOT warranted.

PM Peak:

- Right Turn Volume = 38

- Through Volume = 475



APPENDIX F - MPC COMMENTS





Date: February 23, 2021

Project Name: 875 Cornerstone Drive Apartments

To: Knoxville-Knox County Planning

Subject: TIS Comment Response Document for 875 Cornerstone Drive Apartments Review Comments Dated: February 18, 2021 (Knoxville-Knox County Planning)

Dear Knoxville-Knox County Planning staff,

The following comment response document is submitted to address comments dated February 18, 2021:

#### Knoxville-Knox County Planning (February 18, 2021)

1. <u>Reviewer Comment</u>: On page 4, please correct the discussion of Murdock Drive to "it is classified as a Minor Arterial per Knoxville-Knox County Planning Major Road Plan."

Response: Requested correction made and reflected on page 4 of the Revised TIS.

2. <u>Reviewer Comment</u>: On page 5, please correct "Knoxville-Knox County Metropolitan Planning Commission" to "Knoxville-Knox County Planning".

Response: Requested correction made and reflected on page 5 of the Revised TIS.

3. <u>Reviewer Comment</u>: On page 6, with both traffic counts being completed at the same time, there is some discrepancy with the volumes balancing. The volumes between the two count locations are for the same peak hours and they do not have any other intersections between them. Therefore, for example, we would expect the northbound movement from Murdock Drive at Cornerstone Drive to be the same in the AM and PM as the northbound movement at the project driveway on Cornerstone Drive (i.e. the volume coming out of one intersection equals the volume coming into another intersection). This is true for the AM peak, northbound PM peak, and southbound PM peak volumes. The traffic heading north on Cornerstone Drive from Murdock Drive is 312 vehicles, but only 113 northbound vehicles arrive at the US Cellular driveway. Please revise or explain why the difference since there is no place for traffic to go.

<u>Response</u>: This discrepancy is due to a typo for the westbound right turn volume at the intersection of Murdock Drive at Cornerstone Drive when transposing the data collected and shown in Appendix A to FIGURE 4. The westbound right turn volume was depicted as 280 vehicles for the PM peak hour and should have been depicted as 80 vehicles. The peak hour volumes between the intersections of Murdock Drive at Cornerstone Drive and US Cellular Office Access at Cornerstone Drive now balance as expected. Revised FIGURE 4 can be found on page 6.

This discrepancy in the reported westbound right turn volume resulted in a "domino effect" of revising the subsequent analyzed scenarios depicted in FIGURE 5, FIGURE 6, and FIGURE 9. Most notably, the corrected volumes resulted in the originally proposed westbound right turn lane into



KNOXVILLE 8550 Kingston Pike Knoxville, TN 37919 BOWLING GREEN FAX 865.670.8866 the site access along Murdock Drive no longer being warranted / recommended based on the revised volumes. The revised turn lane warrant spreadsheet has been provided in Appendix E-3.

4. <u>Reviewer Comment</u>: On page 17, please reset the numbering of the recommended improvements to begin at 1 instead of 5.

Response: Requested correction made and reflected on page 17 of the Revised TIS.

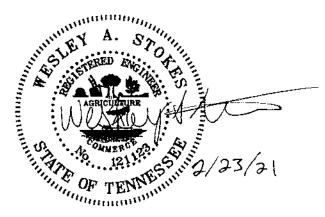
a. The right-turn lane taper and storage lengths of the proposed access off Murdock Drive need to be increased. Per TDOT Design Guidelines, the taper length should be WS/3 where W is the lateral offset in feet and S is the speed in MPH. In this case, the recommended taper length is  $12 \times 40/3 = 160$  feet. The guidelines go on to say that the total length of taper plus storage should provide adequate deceleration length for a complete stop. At 40 MPH, that length is 275 feet. We would propose that the rightturn lane at the new driveway be extended, perhaps to 150 feet each for storage and taper lengths, to provide the 275 feet of deceleration length.

<u>Response</u>: Right-turn lane no longer warranted / recommended as discussed in previous Comment 3 response.

5. <u>Reviewer Comment</u>: This current site of the proposed apartment complex has a heavily used greenway on the parcel. Please mention what mitigation will be done to complete the connection to other portions of the greenway. Please add a section discussing Pedestrian connections, which is a requirement for the study per the Transportation Impact Analysis Guidelines.

<u>Response</u>: Comment address in revised TIS in new *PEDESTRIAN CONNECTION ASSESSMENT* section on page 16.

Sincerely,



Wesley Stokes, P.E.

Planning KNOXVILLE   KNOX COUNTY	<b>Development</b> <ul> <li>Development Plan</li> <li>Planned Development</li> <li>Use on Review / Special Use</li> <li>Hillside Protection COA</li> </ul>	SUBDIVIS Concer Final P	<b>ION</b> ot Plan	<b>ZONING</b> <ul> <li>Plan Amendment</li> <li>SP OYP</li> <li>Rezoning</li> </ul>
875 Cornerstone Mu	Iltifamily Develeopment			
Applicant Name			Affiliation	า
01/22/2021 Date Filed	21 03/11/2021 Meeting Date (if applicable)			File Number(s)
CORRESPONDENCE	All correspondence related to this applicat	tion should be direct	ed to the appr	roved contact listed below.
🗌 Applicant 🗌 Owner 🔲	Option Holder 🛛 Project Surveyor 📡	🕻 Engineer 🛛 Arc	chitect/Landsc	ape Architect
Alan Grissom		nnon & Canr	non, Inc.	
8550 Kingston Pike	Kno Cit	xville	<b>TN</b> State	<b>37919</b> ZIP
865-670-8555	agrissom@cannon-ca	annon.com		
CURRENT PROPERTY INFO	Email			
USCC Real Estate C	orp. P.O. 31369 Ch	icago, IL 606	31	
Owner Name (if different)	Owner Address			Owner Phone
875 Cornerstone Driv	ve Knoxville, TN 37932	131 069		
Property Address		Parcel ID		
FUD	FUD			N
Sewer Provider	Water Provi	der		Septic (Y/N)
STAFF USE ONLY				
E/S Cornerstone Dr., N General Location	I/S Murdock Dr., SE/S Lovell Ro	d.	23.3 Tract Size	30 acres
Gth	OB/TO	PP		
City 🛛 County District	Zoning District	Existing La	nd Use	
Northwest County	Ο		Plai	nned Growth
Planning Sector	Sector Plan Land Use Classific	ation	Growth P	olicy Plan Designation

### DEVELOPMENT REQUEST

🗖 Development Plan	📉 Use on Review / Special Use	Hillside Protection COA	Related City Permit Number(s)
📉 Residential	🗌 Non-Residential		
Home Occupation (spe	cify)		
Other (specify)	Apartment Complex		

SUBDIVISION REQUEST

			F	Related Rezoning File Number
Proposed Subdivision Name				
Unit / Phase Number	Divide Parcel Total N	umber of Lots C	reated	
Other (specify)				
Attachments / Additional Requirements				
ZONING REQUEST				
				Pending Plat File Number
Zoning Change Proposed Zoning				
Plan Amendment Change				
Proposed Plan E	Designation(s)			
Proposed Density (units/acre)	Previous Rezoning Requests			
Other (specify)				
STAFF USE ONLY				
PLAT TYPE		Fee 1		Total
🗖 Staff Review 👘 📉 Planning Commission			1	- A CONTRA
ATTACHMENTS		0401	\$1,50	00
Property Owners / Option Holders 🛛 Var	iance Request	Fee 2		
ADDITIONAL REQUIREMENTS			1	
Design Plan Certification (Final Plat)				
S Use on Review / Special Use (Concept Plan)		Fee 3		
🔀 Traffic Impact Study			ĩ	100 JUL 100 JUL 1
COA Checklist (Hillside Protection)				\$1,500
AUTHORIZATION By signing below,	l certify I am the property own	ner, applicant or	the owners	authorized representative.
Marlin	Alan Grissom		01	/22/2021
Applicant Signature	Please Print		UI	Date
865-670-8555	agrissom@canno	n-cannon	com	
Phone Number	Email	, ourmon		
Sherry Michieni	Sherry Michienzi	3	01	/22/2021
Staff Signature	Please Print			Date