# ANCIENT LORE VILLAGE AT BOYD HOLLOW

Traffic Impact Study Chapman Highway (SR 71) Knoxville, TN

## A Traffic Impact Study for the Ancient Lore Village at Boyd Hollow

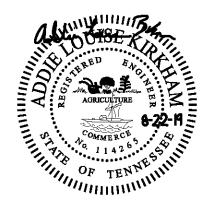
Submitted to

## **Knoxville – Knox County Planning Commission**

August 22, 2019 FMA Project No. 588.004

Submitted By:





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- 4 TRIP GENERATION
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## **Executive Summary**

Ancient Lore Village is proposing a commercial development (i.e. Ancient Lore Village at Boyd Hollow) located in Knox County, Tennessee. The development will include a resort style hotel, fine dining restaurant, and a wedding venue. The project is located near the intersection of Sevierville Pike at Nixon Road. Construction is proposed to take place this year with an expected opening date in 2021.

Traffic for this development will enter and exit via the proposed signalized intersection of Chapman Highway (SR 71) at Nixon Road/Dick Ford Lane. There are two gated entrances for the development located in the rear of the property that will be used only by employees and/or emergency access vehicles.

In order to maintain or provide an acceptable level-of-service for each of the intersections studied, some recommendations are presented.

#### Chapman Highway (SR 71) @ W Dick Ford Lane

Prior to the completion of the Ancient Lore Village the existing intersection of Chapman Highway (SR 71) at Nixon Road will be realigned to intersect with W Dick Ford Lane. Signal timing and roadway alignment to be coordinated with the City of Knoxville and Tennessee Department of Transportation.

The full buildout conditions at the signalized intersection of Chapman Highway (SR 71) at Nixon Road/W Dick Ford Lane were analyzed using the Highway Capacity Software (HCS7). The intersection will operate at a LOS A during both the AM and PM peak hours using optimized signal timing.

#### Chapman Highway (SR 71) @ Nixon Road

Prior to the completion of the Ancient Lore Village the existing intersection of Chapman Highway (SR 71) at Nixon Road will be realigned to intersect with W Dick Ford Lane.

#### Sevierville Pike @ Employee Parking Access

The minimum required sight distance for a road with a posted speed limit of 40 mph is 400 feet in each direction in accordance with the "Subdivision Regulations" for Knoxville and Knox County. The existing landscaping within the right-of-way along Sevierville Pike will need to be modified in order to achieve a sight distance of 400 feet in each direction. FMA recommends the sight distance be verified in the field after the completion of the Ancient Lore Village and that any necessary landscaping be installed to maintain the sight distance and comply with Knox County standards.

#### Sevierville Pike @ Nixon Road

After the completion of the Ancient Lore Village the intersection of Sevierville Pike at Nixon Road will be closed to through traffic.

#### **Nixon Road**

The existing conditions of Nixon Road do not meet the current minimum Knox County and City of Knoxville roadway standards. Improvements on Nixon Road between Chapman Highway (SR 71) and Sevierville Pike including road widening, striping plan, etc. need to be coordinated with both the City of Knoxville and Knox County Engineering and Public Works.

#### 1 Introduction

#### 1.1 Project Description

This report provides a summary of a traffic impact study that was performed for the Ancient Lore Village at Boyd Hollow. The project is located near the intersection of Sevierville Pike at Nixon Road in Knox County, Tennessee. The location of the site is shown in Figure 1.

The full buildout of the development will consist of 150 resort style rooms, a 175 seat fine dining restaurant, a wedding venue and amphitheater seating. Construction is proposed to take place this year with an expected opening date in 2021.

As a part of the Ancient Lore Village development Nixon Road will be realigned to create a signalized intersection on Chapman Highway (SR 71) at W Dick Ford Lane.

Traffic for this development will enter and exit via the proposed signalized intersection of Chapman Highway (SR 71) at Nixon Road/W Dick Ford Lane. There are two gated entrances for the development located in the rear of the property that will be used only by employees and/or emergency access vehicles..

The proposed site layout is shown in Figure 2.

The purpose of this study is to evaluate the impacts to the traffic conditions caused by the proposed development.

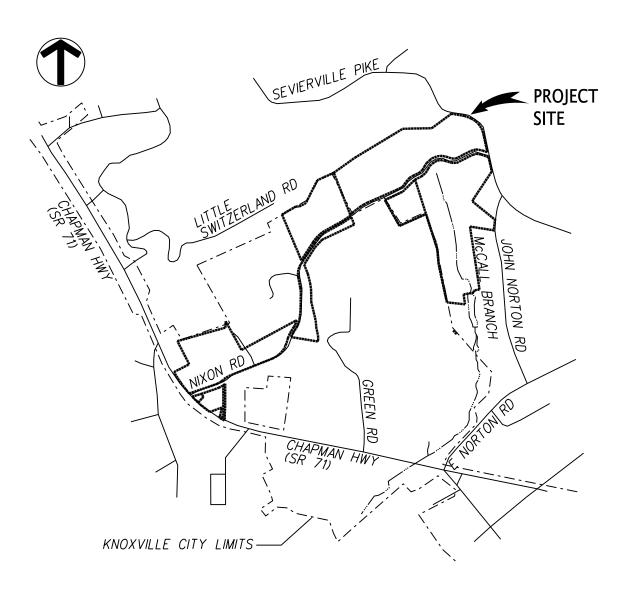


Figure 1: Location Map



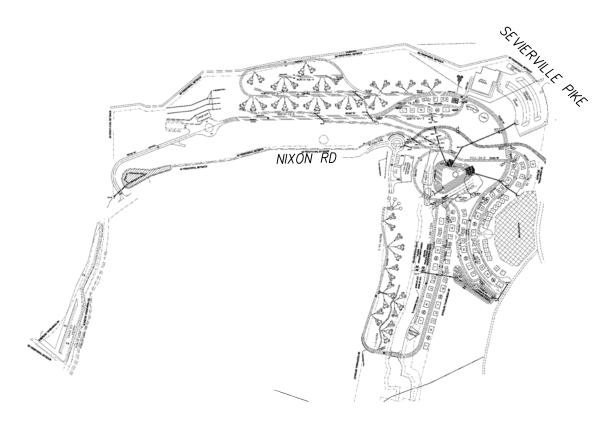


Figure 2: Site Plan

#### 1.2 Existing Site Conditions

Nixon Road at the intersection with Chapman Highway (SR 71) is a two-lane road. The Knoxville-Knox County Planning Commission does not classify Nixon Road per the Major Road Plan therefore; it is considered a local street. There is no posted speed limit on Nixon Road. The existing width of Nixon Road varies from 15 feet to 19 feet between the intersection of Chapman Highway (SR 71) and Sevierville Pike.

Chapman Highway (SR 71) is a five-lane road with a two-way left turn lane at the intersection with Nixon Road. The Knoxville-Knox County Planning Commission classifies Chapman Highway (SR 71) as a major arterial with a 100 foot right-of-way per the Major Road Plan. The posted speed limit on Chapman Highway is 50 mph.

Sevierville Pike at the intersection with Nixon Road is a two-lane road. The Knoxville-Knox County Planning Commission classifies Sevierville Pike at the intersection with Nixon Road Highway (SR 71) as a major collector with a 60 foot right-of-way per the Major Road Plan. The posted speed limit on Sevierville Pike is 40 mph.

W Dick Ford Lane at the intersection with Chapman Highway (SR 71) is a two lane road. The Knoxville-Knox County Planning Commission does not classify W Dick Ford Lane per the Major Road Plan therefore; it is considered a local street. The posted speed limit on W Dick Ford Lane is 25 mph.

Aerial photos of the existing intersections are included in Attachment 1.

The City of Knoxville and Knoxville-Knox County Planning is conducting a study along the Chapman Highway Corridor between the Henley Street Bridge and Mountain Grove Drive. This effort will evaluate previous studies, confirm their recommendations, identify new issues, and develop a strategy for road improvements. The Chapman Highway Corridor Study is not a funded roadway project.

#### 1.3 Transit Network

The Knoxville Area Transit (KAT) operates in the vicinity of the proposed development.

Route 41 (Chapman Highway) stops include Knoxville Station – Platform P, Chapman Highway past Moody, Walmart, Chapman Highway past Young High Pike and Knoxville Station. Traveling along Henley Street and Chapman Highway, this route provides headways of approximately 60 minutes on Weekdays, Saturdays and Sundays.

The nearest KAT stop along Chapman Highway northbound is currently located in front of the Burger King south of the intersection with Nixon Road. The nearest southbound stop is at the 7117 Block of Chapman Highway.

A map of KAT bus route 41 is included in Attachment 9.

## **2 Existing Traffic Volumes**

FMA conducted a turning movement count at the intersection of Chapman Highway at Nixon Road on Wednesday February 13, 2019, at the intersection of Sevierville Pike at Nixon Road on Wednesday March 6, 2019 and at the intersection of Chapman Highway at W Dick Ford Lane on Tuesday August 20, 2019.

The current AM and PM peak hours were determined using the turning movement counts that FMA conducted. At the intersection of Chapman Highway (SR 71) at Nixon Road and Chapman Highway (SR 71) at W Dick Ford Lane the AM peak hour occurred between 7:15 a.m. and 8:15 a.m., and the PM peak hour occurred between 4:45 p.m. and 5:45 p.m.

The existing volumes including the AM and PM peak hour traffic volumes at the count locations are shown in Figure 3, and the count data collected is included in Attachment 2.

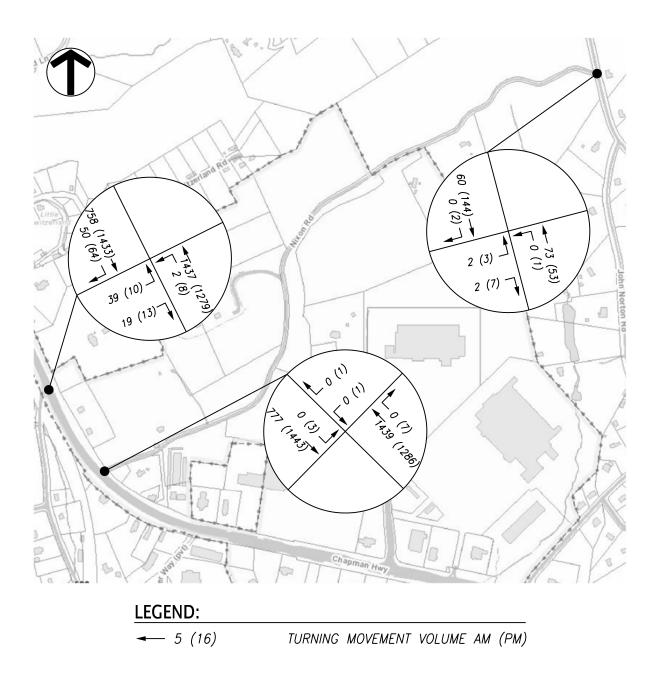


Figure 3: 2019 Existing Peak Hour Traffic

## 3 Background Growth

The Tennessee Department of Transportation (TDOT) and the Knoxville Regional Transportation Planning Organization (TPO) maintain count stations in the vicinity of the proposed development.

TDOT count station #000111 is located on Chapman Highway (SR 71) south of Knoxville and north of the project location. The annual growth rate for this station over the last ten years is approximately 0.00% and the 2017 ADT was 30,935 vehicles per day.

Knoxville TPO count station ID: 093C154 is located on Chapman Highway (SR 71) north of Green Road. The annual growth rate for this station over the last ten years is approximately -1.14% and the 2017 ADT was 27,580 vehicles per day.

Knoxville TPO count station ID: 093M250 is located on Sevierville Pike north of Governor John Sevier Highway. The annual growth rate for this station over the last seven years is approximately 1.20% and the 2016 ADT was 2,410 vehicles per day.

For the purpose of this study, an annual growth rate of 1.0% was assumed for traffic along Chapman Highway, Nixon Road, W Dick Ford Lane and Sevierville Pike until full occupancy is reached in 2022. Attachment 3 shows the trend line growth charts for the Knoxville TPO count stations.

Figure 4 demonstrates the projected background peak hour volumes at the intersections of Chapman Highway (SR 71) at Nixon Road, Chapman Highway (SR 71) at W Dick Ford Lane and Sevierville Pike at Nixon Road after applying the background growth rate to the existing conditions.

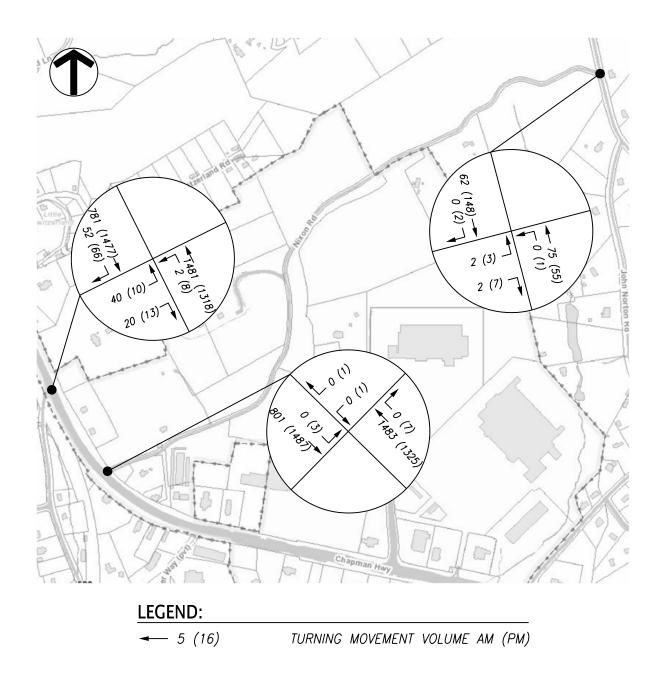


Figure 4: 2022 Background Peak Hour Traffic

## 4 Trip Generation and Trip Distribution

The trip generation was calculated using the fitted curve equations where provided from *Trip Generation*, 10<sup>th</sup> *Edition*, published by the Institute of Transportation Engineers. Resort Hotel (Land Use 330) was used to calculate the AM and PM peak hour trips and Hotel (Land Use 310) was used to calculate the daily trips since that information was not available for the Resort Hotel (Land Use 330).

Quality Restaurant (Land Use 931) was assumed in order to calculate the estimated trips for a 175 seat fine dining restaurant. It was assumed that 25% of the proposed trips will be new trips and 75% of the proposed trips will be internal trips within the resort. The restaurant will not be open during the AM peak hour to the public; therefore no trips were assumed for the AM peak hour.

The land use worksheets are included in Attachment 4.

The total number of new trips generated by the Ancient Lore Village at Boyd Hollow will be 1,326 new daily trips, 28 trips during the AM peak hour and 35 trips during the PM peak hour. A trip generation summary is shown in Table 4-1.

**Table 4 - Trip Generation Summary Ancient Lore Village at Boyd Hollow** 

Land Use	Density	Daily AM Peak Hour Trips Enter Exit		PM Pe Enter	ak Hour Exit							
Hotel												
Hotel (LUC 310/330)	150 Rooms	1,267	20	8	10	13						
Quality Restaurant (LUC 931)	175 Seats	235	-	-	33	16						
75% Internal Reduction	on	-176	-	-	-25	-12						
Total New Trips		1,326	20	8	18	17						
Amphitheater	550 seats	- S <sub>I</sub>	pecial Events	s Only -								

The amphitheater will have approximately 550 seats and is expected to be used as a gathering place for hotel guests and seating for on-site weddings. At this time any special events that will be scheduled for the amphitheater are planned to occur on the weekends and will not interfere with weekday peak hour traffic.

Chapman Highway (SR 71) at the intersection with Nixon Road has a trip distribution of 65% northbound and 35% southbound during the AM peak hour and 50% northbound and 50% southbound during the PM peak hour.

The directional distribution of the traffic generated by the Ancient Lore Village was determined using the existing traffic volumes at the intersection of Chapman Highway (SR 71) at W Dick Ford Lane. It was assumed that the exiting traffic would be 67% northbound and 33% southbound during the AM peak and 50% northbound and 50% southbound during the PM peak hour. The entering traffic will be predominately from downtown Knoxville at 10% northbound and 90% southbound during both the AM and PM peak hours.

Figure 5 shows the AM peak hour trip distribution and Figure 6 shows the PM peak hour trip distribution.

Figure 7 shows the peak hour site traffic from the development and Figure 8 shows the peak hour full buildout traffic.

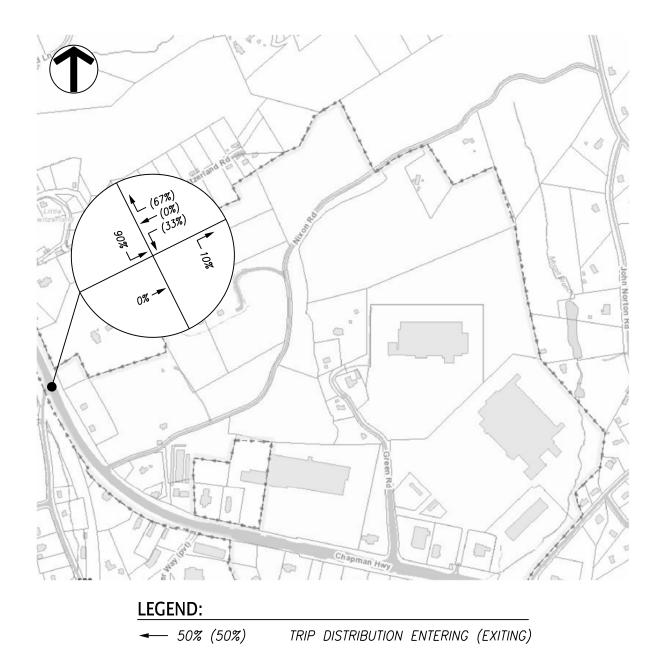


Figure 5: AM Peak Hour Trip Distribution

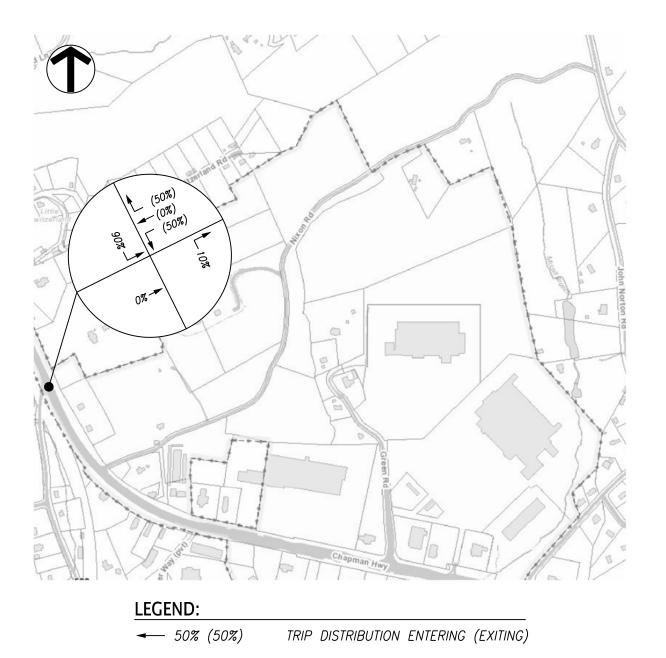
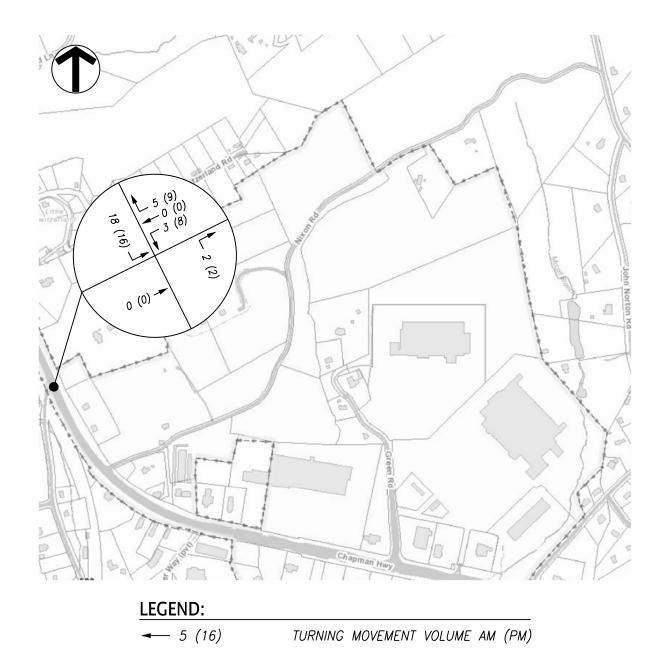


Figure 6: PM Peak Hour Trip Distribution



**Figure 7: Peak Hour Site Traffic** 

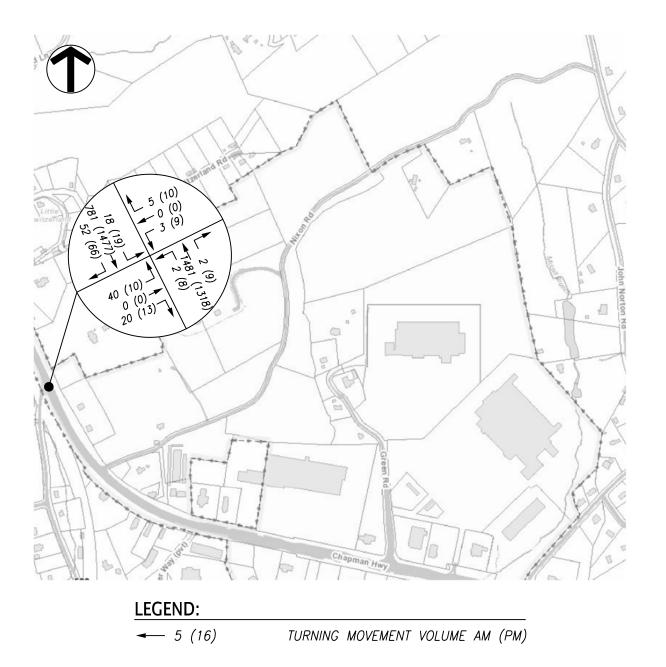


Figure 8: Peak Hour Full Buildout Traffic

## 5 Projected Capacity and Level of Service

Unsignalized intersection capacity analyses were performed using the Highway Capacity Software (HCS7) for the AM and PM peak hours to evaluate the traffic conditions at the intersections of Chapman Highway (SR 71) at Nixon Road, Chapman Highway (SR 71) at W Dick Ford Lane and Sevierville Pike at Nixon Road for the existing and background conditions.

Prior to the completion of the Ancient Lore Village the existing intersection of Chapman Highway (SR 71) at Nixon Road will be realigned to intersect with W Dick Ford Lane. The proposed new intersection of Chapman Highway (SR 71) at W Dick Ford Lane/Nixon Road will be signalized.

Signalized intersection capacity analyses were performed using Highway Capacity Software (HCS7) at the proposed intersection of Chapman Highway (SR 71) at Nixon Road/Dick Ford Lane. Optimized signal timing was assumed for the full buildout conditions.

The results from the analyses are expressed with a term "level of service" (LOS), which is based on the amount of delay experienced at the intersection. The LOS index ranges from LOS A, indicating excellent traffic conditions with minimal delay, to LOS F indicating very congested conditions with excessive delay. LOS D generally is considered the minimum acceptable condition in urban areas. The existing, background and full buildout HCS7 worksheets are included in Attachments 5, 6 and 7.

Table 5-1 shows the results of the capacity analyses.

Table 5-1
Intersection Analysis - Level of Service (LOS) Summary

		Delay (sec)/LOS								
Chapman Highway (SR 71) @ Nixon Road (Existing 2019)										
AM Peak	WB Approach SB Approach									
PM Peak	WB Approach SB Approach									

Chapm	an Highway (SR 71) @	W Dick Ford Lane (Existing 2019)
AM Peak	EB Approach NB Approach	119.4 / F 10.2 / B
PM Peak	EB Approach NB Approach	141.2 / F 15.5 / C
	Sevierville Pike @ Ni	xon Road (Existing 2019)
AM Peak	EB Approach NB Approach	9.2 / A 7.4 / A
PM Peak	EB Approach NB Approach	9.5 / A 7.6 / A
Chapr	man Highway (SR 71) @	Nixon Road (Background 2022)
AM Peak	WB Approach SB Approach	5.0 / A 13.8 / B
PM Peak	WB Approach SB Approach	54.3 / F 12.3 / B
Chapmar	n Highway (SR 71) @ W	Dick Ford Lane (Background 2022)
AM Peak	EB Approach NB Approach	145.2 / F 10.4 / B
PM Peak	EB Approach NB Approach	170.8 / F 16.1 / C
	Sevierville Pike @ Nixo	n Road (Background 2022)
AM Peak	EB Approach NB Approach	9.2 / A 7.4 / A
PM Peak	EB Approach NB Approach	9.5 / A 7.6 / A
Chapn	nan Highway (SR 71) @	Nixon Road (Full Buildout 2022)
AM Peak	Intersection	7.2 / A
PM Peak	Intersection	8.1 / A

## **6** Turn Lane Warrant Analysis

The intersection of Sevierville Pike at the proposed employee parking access Road was evaluated to determine if a northbound left turn lane or a southbound right turn lane is warranted. The Knox County Department of Engineering and Public Works handbook, "Access Control and Driveway Design Policy," was used to analyze the information.

Neither a right turn lane nor a left turn lane are warranted at the intersection of Sevierville Pike at the employee access driveway. The turn lane warrant worksheets and analysis are included in Attachment 8.

#### 7 Conclusions and Recommendations

#### 7.1 Chapman Highway (SR 71) @ W Dick Ford Lane

The existing traffic conditions at the unsignalized intersection of Chapman Highway (SR 71) at W Dick Ford Lane were analyzed using the Highway Capacity Software (HCS7). The eastbound approach will operate at a LOS F during both the AM and PM peak hours and the northbound approach will operate at a LOS B during the AM peak hour and LOS C during the PM peak hour.

The background traffic conditions at the unsignalized intersection of Chapman Highway (SR 71) at Dick Ford Lane were analyzed using the Highway Capacity Software (HCS7). The eastbound approach will operate at a LOS F during both the AM and PM peak hours and the northbound approach will operate at a LOS B during the AM peak hour and LOS C during the PM peak hour.

Prior to the completion of the Ancient Lore Village the existing intersection of Chapman Highway (SR 71) at Nixon Road will be realigned to intersect with W Dick Ford Lane. Signal timing and roadway alignment to be coordinated with the City of Knoxville and Tennessee Department of Transportation.

The full buildout conditions at the signalized intersection of Chapman Highway (SR 71) at Nixon Road/W Dick Ford Lane were analyzed using the Highway Capacity Software (HCS7). The intersection will operate at a LOS A during both the AM and PM peak hours using optimized signal timing.

At signalized intersections the recommended sight distance per AASHTO "Geometric Design of Highways and Streets" is that the "first vehicle stopped on one approach

should be visible to the driver of the first vehicle stopped on each of the other approaches" and that "left-turning vehicles should have sufficient sight distance to select gaps in oncoming traffic and complete left turns."

FMA recommends any landscaping be installed so as to maintain the sight distance and continue to comply with TDOT standards.

#### 7.2 Chapman Highway (SR 71) @ Nixon Road

The existing traffic conditions at the unsignalized intersection of Chapman Highway (SR 71) at Nixon Road were analyzed using the Highway Capacity Software (HCS7). The southbound approach will operate at a LOS B during both the AM and PM peak hours and the westbound approach will operate at a LOS A during the AM peak hour and LOS E during the PM peak hour.

The background traffic conditions at the unsignalized intersection of Chapman Highway (SR 71) at Nixon Road were analyzed using the Highway Capacity Software (HCS7). The southbound approach will operate at a LOS B during both the AM and PM peak hours and the westbound approach will operate at a LOS A during the AM peak hour and a LOS F during the PM peak hour.

Prior to the completion of the Ancient Lore Village the existing intersection of Chapman Highway (SR 71) at Nixon Road will be realigned to intersect with W Dick Ford Lane.

## 7.3 Sevierville Pike @ Employee Parking Access

The minimum required sight distance for a road with a posted speed limit of 40 mph is 400 feet in each direction in accordance with the "Subdivision Regulations" for Knoxville and Knox County. The existing landscaping within the right-of-way along Sevierville Pike will need to be modified in order to achieve a sight distance of 400 feet in each direction. FMA recommends the sight distance be verified in the field after the completion of the Ancient Lore Village and that any necessary landscaping be installed to maintain the sight distance and comply with Knox County standards.

#### 7.4 Sevierville Pike @ Nixon Road

The existing traffic conditions at the unsignalized intersection of Sevierville Pike at Nixon Road were analyzed using the Highway Capacity Software (HCS7). The eastbound approach will operate at a LOS A during both the AM and PM peak hours and the southbound approach will operate at a LOS A during both the AM and PM peak hours.

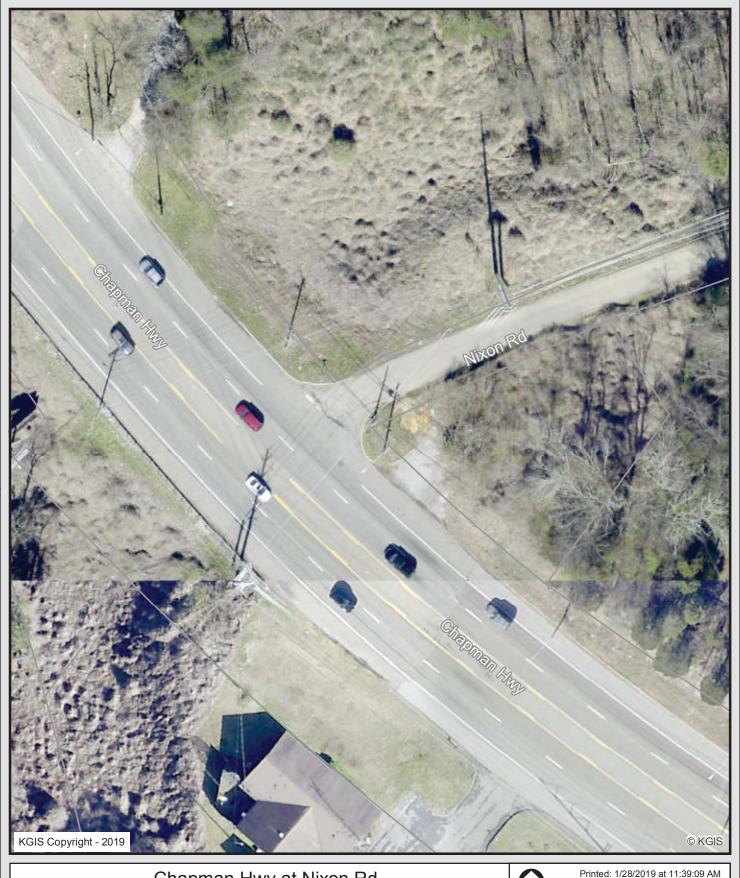
The background traffic conditions at the unsignalized intersection of Sevierville Pike at Nixon Road were analyzed using the Highway Capacity Software (HCS7). The eastbound approach will operate at a LOS A during both the AM and PM peak hours and the southbound approach will operate at a LOS A during both the AM and PM peak hours.

After the completion of the Ancient Lore Village the intersection of Sevierville Pike at Nixon Road will be closed to through traffic.

#### 7.5 Nixon Road

The existing width of Nixon Road between the intersection with Chapman Highway (SR 71) and Sevierville Pike varies between 15 feet to 19 feet. The existing conditions of Nixon Road do not meet the current minimum City of Knoxville and Knox County roadway standards. Improvements on Nixon Road between Chapman Highway (SR 71) and Sevierville Pike including road widening, striping plan, etc. need to be coordinated with both the City of Knoxville and Knox County Engineering and Public Works.

## Attachment 1 Aerial Photo

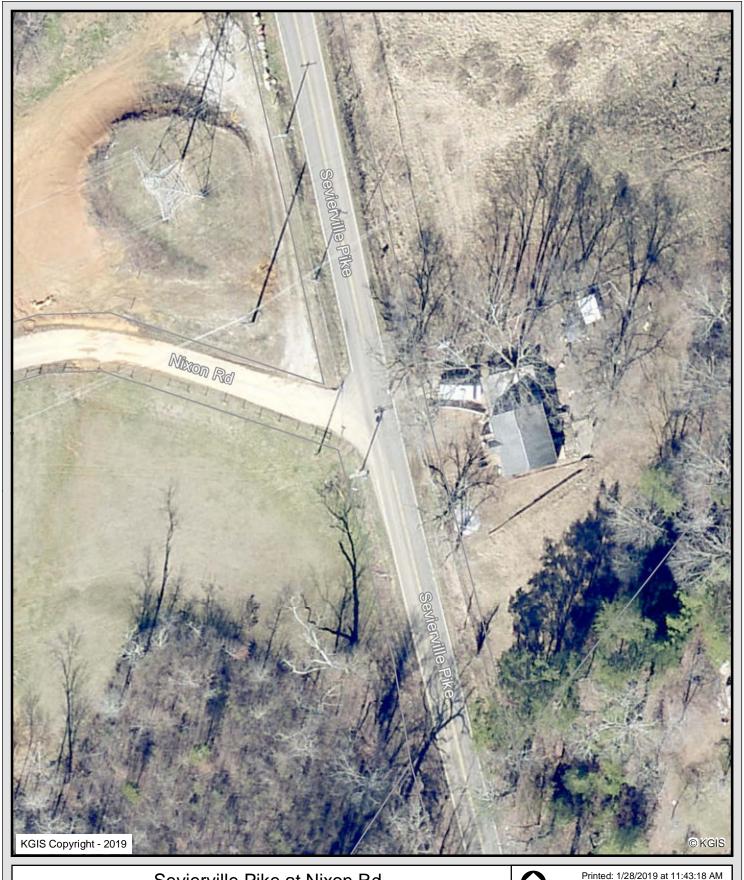




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#### Sevierville Pike at Nixon Rd

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## Chapman Hwy at W Dick Ford Lane

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## **Attachment 2 Traffic Counts**

**Project: Ancient Lore Village at Boyd Hollow Intersection: Chapman Highway at Nixon Road** 

Date Conducted: 2/13/2019

		man High		Chapman Highway			N			
		orthbound			uthbound		V	1		
Start	Thru	Right	Total	Left	Thru	Total	Left	Right	Total	Int. Total
7:00 AM	328	0	328	0	156	156	0	0	0	484
7:15 AM	372	0	372	0	180	180	0	0	0	552
7:30 AM	409	0	409	0	188	188	0	0	0	597
7:45 AM	339	0	339	0	213	213	0	0	0	552
Total	1448	0	1448	0	737	737	0	0	0	2185
8:00 AM	319	0	319	0	196	196	0	0	0	515
8:15 AM	279	0	279	0	218	218	0	0	0	497
8:30 AM	226	0	226	0	187	187	0	0	0	413
8:45 AM	177	0	177	0	148	148	0	0	0	325
Total	1001	0	1001	0	749	749	0	0	0	1750
2:00 PM	233	0	233	0	232	232	1	0	1	466
2:15 PM	217	2	219	0	266	266	0	0	0	485
2:30 PM	192	1	193	2	239	241	0	0	0	434
2:45 PM	227	1	228	0	209	209	1	1	2	439
Total	869	4	873	2	946	948	2	1	3	1824
3:00 PM	260	0	260	1	304	305	0	1	1	566
3:15 PM	263	0	263	1	279	280	0	1	1	544
3:30 PM	262	0	262	1	292	293	0	1	1	556
3:45 PM	303	0	303	1	306	307	1	0	1	611
Total	1088	0	1088	4	1181	1185	1	3	4	2277
4:00 PM	256	1	257	1	357	358	1	0	1	616
4:15 PM	262	0	262	0	308	308	0	1	1	571
4:30 PM	281	0	281	0	339	339	0	0	0	620
4:45 PM	322	1	323	0	344	344	0	0	0	667
Total	1121	2	1123	1	1348	1349	1	1	2	2474
5:00 PM	325	4	329	1	369	370	1	1	2	
5:15 PM	315	2	317	2	348	350	0	0	0	667
5:30 PM	324	0	324	0	382	382	0	0	0	706
5:45 PM	269	2	271	0	333	333	0	0	0	604
Total	1233	8	1241	3	1432	1435	1	1	2	2678
Grand Total	1 (700	1 1	ا م	10	(202	(402	-	•	11	12100
	6760	14	6774	10	6393	6403	5 45 5	6	11	13188
Approach %	99.8	0.2	F1 ,	0.2	99.8	40.6	45.5	54.5	0.1	
Total %	51.3	0.1	51.4	0.1	48.5	48.6	0.0	0.0	0.1	

**Project: Ancient Lore Village at Boyd Hollow Date Conducted: 2/13/2019** 

AM Peak Hour	7:15 AM - 8:15 AM	2216
PM Peak Hour	4:45 PM - 5:45 PM	2741

	Chapn	. ,		Chapr	Chapman Highway			Nixon Road		
	No	rthbour	nd	Southbound			Westbound			
Start	Thru	Right	Total	Left	Thru	Total	Left	Right	Total	Int. Total
Peak Hour Analysis from 7	:00 AM t	o 9:00 A	M							<u>.</u>
AM Peak Hour begins at 7:	15 AM									
7:15 AM	372	0	372	0	180	180	0	0	0	552
7:30 AM	409	0	409	0	188	188	0	0	0	597
7:45 AM	339	0	339	0	213	213	0	0	0	552
8:00 AM	319	0	319	0	196	196	0	0	0	515
Total Volume	1439	0	1439	0	777	777	0	0	0	2216
Future (1% over 3 yrs)	1483	0		0	801		0	0		2283
PHF	0.88	-		-	0.91		-	-		0.93
Peak Hour Analysis from 2	:30 PM to	o 6:00 P	M							
PM Peak Hour begins at 5:	00 PM					_				
4:45 PM	322	1	323	0	344	344	0	0	0	667
5:00 PM	325	4	329	1	369	370	1	1	2	701
5:15 PM	315	2	317	2	348	350	0	0	0	667
5:30 PM	324	0	324	0	382	382	0	0	0	706
Total Volume	1286	7	1293	3	1443	1446	1	1	2	2741
Future (1% over 3 yrs)	1325	7		3	1487		1	1		2824
PHF	0.99	0.44		0.38	0.94		0.25	0.25		0.97

**Project: Ancient Lore Village at Boyd Hollow Intersection: Sevierville Pike at Nixon Road** 

Date Conducted: 3/6/2019

	Sev	ierville P	ike	Sevierville Pike			Nixon Road				
	So	outhboun	d	No	orthbound	d	Е	astbound			
Start	Thru	Right	Total	Left	Thru	Total	Left	Right	Total	Int. Total	
7:00 AM	2	0	2	0	4	4	0	0	0	6	
7:15 AM	13	0	13	0	19	19	1	0	1	33	
7:30 AM	20	0	20	0	31	31	0	0	0	51	
7:45 AM	16	0	16	0	16	16	1	0	1	33	
Total	51	0	51	0	70	70	2	0	2	123	
8:00 AM	11	0	11	0	7	7	0	0	0	18	
8:15 AM	6	0	6	2	12	14	0	0	0	20	
8:30 AM	9	0	9	0	10	10	0	0	0	19	
8:45 AM	8	0	8	0	6	6	1	0	1	15	
Total	34	0	34	2	35	37	1	0	1	72	
2:00 PM	12	0	12	4	9	13	0	0	0	25	
2:15 PM	6	0	6	0	10	10	0	2	2	18	
2:30 PM	12	1	13	0	9	9	0	0	0	22	
2:45 PM	11	0	11	0	8	8	0	1	1	20	
Total	41	1	42	4	36	40	0	3	3	85	
3:00 PM	9	0	9	1	14	15	0	0	0	24	
3:15 PM	13	0	13	0	14	14	0	0	0	27	
3:30 PM	11	0	11	0	13	13	0	2	2	26	
3:45 PM	24	1	25	2	12	14	2	0	2	41	
Total	57	1	58	3	53	56	2	2	4	118	
4:00 PM	32	0	32	0	10	10	1	1	2	44	
4:15 PM	19	0	19	0	9	9	0	1	1	29	
4:30 PM	25	1	26	1	17	18	1	3	4	48	
4:45 PM	35	0	35	0	6	6	1	3	4	45	
Total	111	1	112	1	42	43	3	8	11	166	
5:00 PM	34	1	35	0	14	14	0	1	1	50	
5:15 PM	50	0	50	0	16	16	1	0	1	67	
5:30 PM	31	0	31	0	8	8	0	0	0	39	
5:45 PM	23	1	24	0	15	15	1	0	1	40	
Total	138	2	140	0	53	53	2	1	3	196	
Grand Total	432	5	437	10	289	299	10	14	24	760	
Approach %	98.9	1.1	<del>4</del> 3/	3.3	96.7	233	41.7	58.3	∠4	700	
Total %	56.8	0.7	5 <i>7</i> .5	1.3	38.0	39.3		1.8	3.2		
i Otai 70	1 50.0	0.7	37.3	1.3	50.0	39.3	1.3	1.0	3.2		

**Project: Ancient Lore Village at Boyd Hollow Date Conducted: 3/6/2019** 

AM Peak Hour	7:15 AM - 8:15 AM	13 <i>7</i>
PM Peak Hour	4:30 PM - 5:30 PM	210

	Sevi	Sevierville Pike		Sevi	Sevierville Pike			Nixon Road		
	So	uthbou	nd	No	Northbound			Eastbound		
Start	Thru	Right	Total	Left	Thru	Total	Left	Right	Total	Int. Total
Peak Hour Analysis from 7	:00 AM t	o 9:00 /	M							<u> </u>
AM Peak Hour begins at 7:	:15 AM									
7:15 AM	13	0	13	0	19	19	1	1	2	34
7:30 AM	20	0	20	0	31	31	0	0	0	51
7:45 AM	16	0	16	0	16	16	1	1	2	34
8:00 AM	11	0	11	0	7	7	0	0	0	18
Total Volume	60	0	60	0	73	73	2	2	4	137
Future (1% over 3 yrs)	62	0		0	<i>7</i> 5		2	2		141
PHF	0.75	-		-	0.59		0.50	0.50		0.67
Peak Hour Analysis from 2	:30 PM t	o 6:00 P	M							
PM Peak Hour begins at 4:	30 PM					_				
4:30 PM	25	1	26	1	17	18	1	3	4	48
4:45 PM	35	0	35	0	6	6	1	3	4	45
5:00 PM	34	1	35	0	14	14	0	1	1	50
5:15 PM	50	0	50	0	16	16	1	0	1	67
Total Volume	144	2	146	1	53	54	3	7	10	210
Future (1% over 3 yrs)	148	2	_	1	55		3	7		216
PHF	0.72	0.50		0.25	0.78		0.75	0.58		0.78

**Project: Ancient Lore Village at Boyd Hollow Intersection: Chapman Highway at Nixon Road** 

Date Conducted: 8/20/2019

	Chapman Highway			Chapman Highway			W D	ick Ford L			
	No	orthbour	nd	S	outhbound	l k	W	estbound/	ł		
Start	Left	Thru	Total	Thru	Right	Total	Left	Right	Total	Int. Total	
7:00 AM	1	-	1	-	1	1	11	4	15	17	
7:15 AM	0	-	0	-	8	8	13	5	18	26	
7:30 AM	0	-	0	-	10	10	9	3	12	22	
7:45 AM	1	-	1	-	13	13	8	6	14	28	
Total	2	-	2	-	32	32	41	18	59	93	
8:00 AM	1	_	1	-	19	19	9	5	14	34	
8:15 AM	1	-	1	-	9	9	6	8	14	24	
8:30 AM	0	-	0	-	8	8	7	6	13	21	
8:45 AM	0	-	0	-	4	4	4	2	6	10	
Total	2	-	2	-	40	40	26	21	47	89	
4:00 PM	1	-	1	-	13	13	3	1	4	18	
4:15 PM	1	-	1	-	12	12	1	2	3	16	
4:30 PM	2	-	2	-	12	12	4	2	6	20	
4:45 PM	3	-	3	-	20	20	3	2	5	28	
Total	7	-	7	-	5 <i>7</i>	57	11	7	18	82	
5:00 PM	1	-	1	-	15	15	1	3	4	20	
5:15 PM	1	-	1	-	14	14	4	5	9	24	
5:30 PM	3	-	3	-	15	15	2	3	5	23	
5:45 PM	1	-	1	-	22	22	5	0	5	28	
Total	6	-	6	-	66	66	12	11	23	95	
			_							_	
Grand Total	17	-	1 <i>7</i>	-	195	195	90	5 <i>7</i>	147	359	
Approach %	100.0	-		-	100.0		61.2	38.8			
Total %	4.7	-	4.7	-	54.3	54.3	25.1	15.9	40.9		

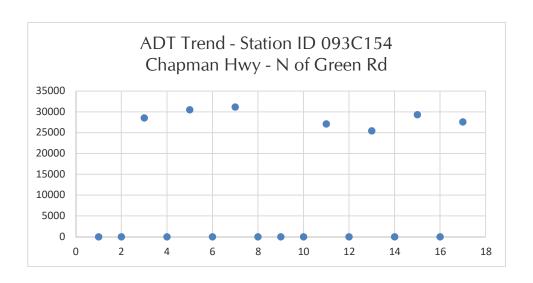
Project: Ancient Lore Village at Boyd Hollow Date Conducted: 8/20/2019

AM Peak Hour	7:15 AM - 8:15 AM	110
PM Peak Hour	4:45 PM - 5:45 PM	95

	Chapr	nan Hig	ghway	Chapi	man Hig	shway	W Di	ck Ford	Lane	
	No	orthbou	nd	Sc	outhboui	nd	W	estbour	nd	
Start	Left	Thru	Total	Thru	Right	Total	Left	Right	Total	Int. Total
Peak Hour Analysis from 7:00 AM to 9:00 AM							<u>.</u>			
AM Peak Hour begins at 7:15 AM										
7:15 AM	0	-	0	-	8	8	13	5	18	26
7:30 AM	0	-	0	-	10	10	9	3	12	22
7:45 AM	1	-	1	-	13	13	8	6	14	28
8:00 AM	1	-	1	-	19	19	9	5	14	34
Total Volume	2	-	2	-	50	50	39	19	58	110
Future (1% over 3 yrs)	2	-		-	52		40	20		113
PHF	0.50	-		-	0.66		0.75	0.79		0.81
Peak Hour Analysis from 2:30 PM to 6:00 PM										
PM Peak Hour begins at 4:	45 PM		_			_				
4:45 PM	3	-	3	-	20	20	3	2	5	28
5:00 PM	1	-	1	-	15	15	1	3	4	20
5:15 PM	1	-	1	-	14	14	4	5	9	24
5:30 PM	3	-	3	-	15	15	2	3	5	23
Total Volume	8	-	8	-	64	64	10	13	23	95
Future (1% over 3 yrs)	8	-	_	-	66		10	13		98
PHF	0.67	-	·	-	0.80		0.63	0.65		0.85

## Attachment 3 ADT Trends

	Adjusted
	Average Daily
Year	Traffic
2001	0
2002	0
2003	28549
2004	0
2005	30502
2006	0
2007	31141
2008	0
2009	0
2010	0
2011	27090
2012	0
2013	25450
2014	0
2015	29320
2016	0
2017	27580



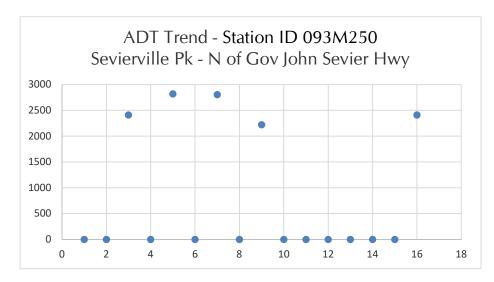
Most Recent Trend Line Growth

Year ADT 2007 31141 2017 27580

Annual Percent Growth -1.14%

Adjusted Average Daily Traffic

Year	Traffic
2001	0
2002	0
2003	2410
2004	0
2005	2820
2006	0
2007	2805
2008	0
2009	2223
2010	0
2011	0
2012	0
2013	0
2014	0
2015	0
2016	2410

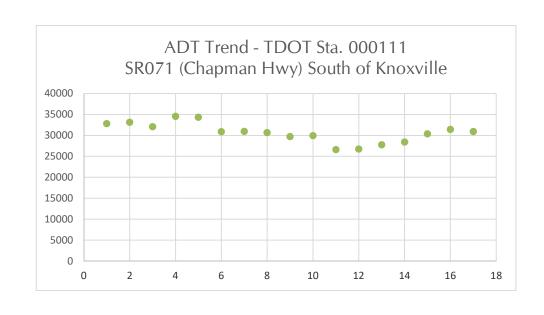


#### Most Recent Trend Line Growth

Year ADT 2009 2223 2016 2410

Annual Percent Growth	1.20%
-----------------------	-------

	Adjusted	
	Average Daily	
Year	Traffic	
2001	32816	
2002	33145	
2003	32083	
2004	34551	
2005	34338	
2006	30895	
2007	30946	
2008	30672	
2009	29714	
2010	29966	
2011	26620	
2012	26764	
2013	27768	
2014	28414	
2015	30379	
2016	31449	
2017	30935	



#### Most Recent Trend Line Growth

Year ADT 2007 30946 2017 30935

	Annual	Percent Growth	0.00%
--	--------	----------------	-------

# Attachment 4 Trip Generation

**Project: Ancient Lore Village at Boyd Hollow** 

Date Conducted: 1/21/2019

(LUC 310) Hotel 150 Rooms

#### **Average Daily Traffic**

T = 11.29(X) - 426.97

T = 11.29(150) - 426.97

T = 1267

(LUC 330) Resort Hotel 150 Rooms

## Peak Hour of Adjacent Street Traffic One Hour Between 7 and 9 a.m.

T = 0.38(X) - 28.58

T = 0.38(150) - 28.58

T = 28

## Peak Hour of Adjacent Street Traffic One Hour Between 4 and 6 p.m.

T = 0.52(X) - 55.42

T = 0.52(150) - 55.42

T = 23

		Perd	cent	Nun	nber
Time Period	Total Trips	Enter	Exit	Enter	Exit
Weekday (24 hours)	1267	50%	50%	634	634
AM Peak Hour	28	72%	28%	20	8
PM Peak Hour	23	43%	57%	10	13

Project: Ancient Lore Village at Boyd Hollow

Date Conducted: 8/19/2019

## (LUC 931) Quality Restaurant 175 Seats

#### **Average Daily Traffic**

T = 3.90 (X) - 447.07

T = 3.90 (175) - 447.07

T = 235

# Peak Hour of Adjacent Street Traffic One Hour Between 4 and 6 p.m.

Average Rate = 0.28

T = 0.28 \* (175)

T = 49

**Total Trips** 

		Perd	cent	Nun	nber
Time Period	Total Trips	Enter	Exit	Enter	Exit
Weekday (24 hours)	235	50%	50%	118	118
PM Peak Hour	49	67%	33%	33	16

#### 25% New Trips

		Perd	cent	Nun	nber
Time Period	Total Trips	Enter	Exit	Enter	Exit
Weekday (24 hours)	59	50%	50%	29	29
PM Peak Hour	12	67%	33%	8	4

#### 75% Internal Trips

		Per	cent	Nun	nber
Time Period	Total Trips	Enter	Exit	Enter	Exit
Weekday (24 hours)	176	50%	50%	88	88
PM Peak Hour	37	67%	33%	25	12

# Quality Restaurant (931)

Vehicle Trip Ends vs: Seats

On a: Weekday

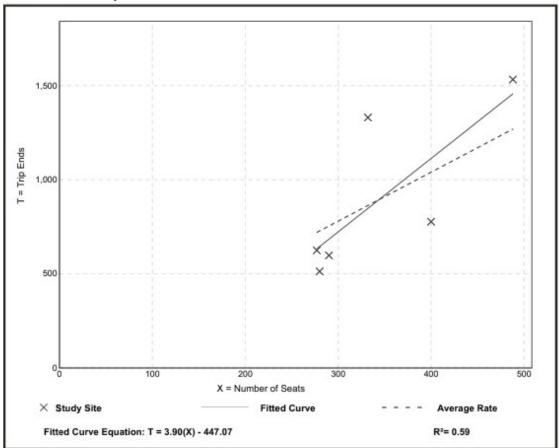
Setting/Location: General Urban/Suburban

Number of Studies: 6 Avg. Num. of Seats: 345

Directional Distribution: 50% entering, 50% exiting

#### Vehicle Trip Generation per Seat

Average Rate	Range of Rates	Standard Deviation
2.60	1.83 - 4.01	0.85





# Quality Restaurant (931)

Vehicle Trip Ends vs: Seats

On a: Weekday,

> Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

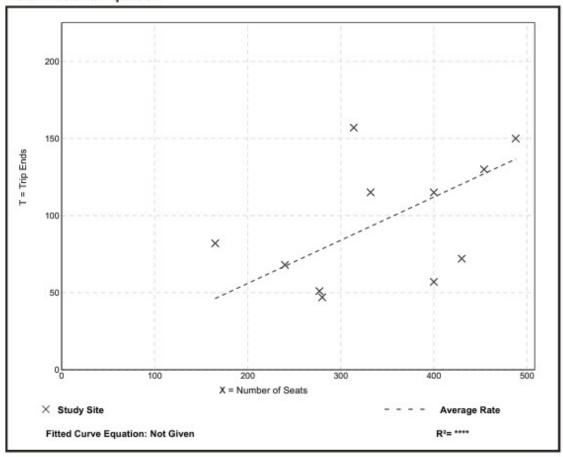
Setting/Location: General Urban/Suburban

Number of Studies: Avg. Num. of Seats: 344

Directional Distribution: 67% entering, 33% exiting

#### Vehicle Trip Generation per Seat

Average Rate	Range of Rates	Standard Deviation
0.28	0.14 - 0.50	0.11





## Hotel

(310)

Vehicle Trip Ends vs: Rooms On a: Weekday

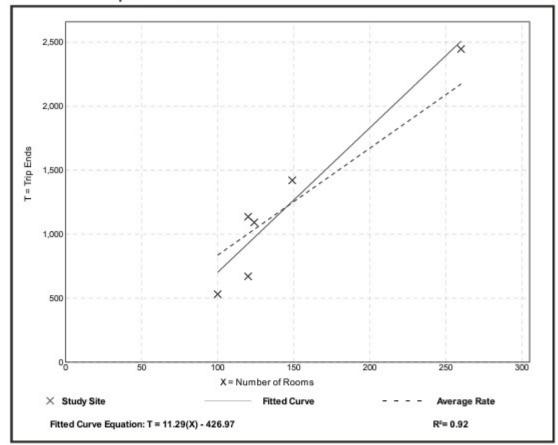
Setting/Location: General Urban/Suburban

Number of Studies: 6

Avg. Num. of Rooms: 146
Directional Distribution: 50% entering, 50% exiting

### Vehicle Trip Generation per Room

Average Rate	Range of Rates	Standard Deviation
8.36	5.31 - 9.53	1.86





#### **Resort Hotel**

(330)

Vehicle Trip Ends vs: Rooms

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

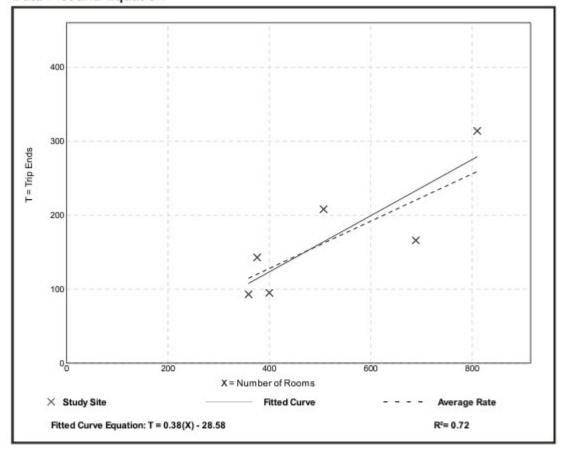
Setting/Location: General Urban/Suburban

Number of Studies: 6

Avg. Num. of Rooms: 524
Directional Distribution: 72% entering, 28% exiting

#### Vehicle Trip Generation per Room

Average Rate	Range of Rates	Standard Deviation
0.32	0.24 - 0.41	0.08





#### **Resort Hotel**

(330)

Vehicle Trip Ends vs: Rooms

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

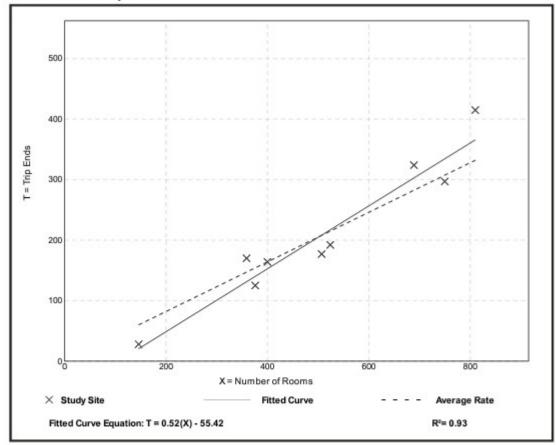
Setting/Location: General Urban/Suburban

Number of Studies: 9 Avg. Num. of Rooms: 507

Directional Distribution: 43% entering, 57% exiting

#### Vehicle Trip Generation per Room

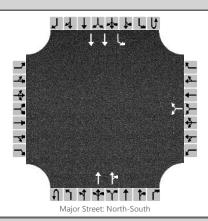
Average Rate	Range of Rates	Standard Deviation
0.41	0.19 - 0.51	0.08





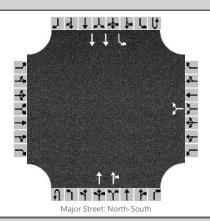
## Attachment 5 Intersection Worksheets – Existing AM/PM Peaks

	HCS7 Two-Way Stop	op-Control Report								
General Information		Site Information								
Analyst	Addie Kirkham	Intersection	Chapman Hwy at Nixon Rd							
Agency/Co.	FMA	Jurisdiction	Knox County							
Date Performed	3/27/2019	East/West Street	Nixon Road							
Analysis Year	2019	North/South Street	Chapman Hwy (SR 71)							
Time Analyzed	Existing AM Peak	Peak Hour Factor	0.93							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description	588.004 Ancient Lore Village									



<b>Vehicle Volumes and Ad</b>	ljustmo	ents																
Approach		Eastb	ound			Westl	oound		Northbound					South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	0	0		0	1	0	0	0	2	0	0	1	2	0		
Configuration							LR				Т	TR		L	T			
Volume, V (veh/h)						0		0			1439	0		0	777			
Percent Heavy Vehicles (%)						2		2						2				
Proportion Time Blocked						0.000		0.000						0.000				
Percent Grade (%)						(	)											
Right Turn Channelized		Ν	10			Ν	lo			Ν	lo			N	О			
Median Type/Storage				Undi	vided								•					
Critical and Follow-up H	leadwa	ıys																
Base Critical Headway (sec)						7.5		6.9						4.1				
Critical Headway (sec)						6.84		6.94						4.14				
Base Follow-Up Headway (sec)						3.5		3.3						2.2				
Follow-Up Headway (sec)						3.52		3.32						2.22				
Delay, Queue Length, ar	nd Leve	el of S	ervice	<b>e</b>														
Flow Rate, v (veh/h)							0							0				
Capacity, c (veh/h)							0							425				
v/c Ratio														0.00				
95% Queue Length, Q <sub>95</sub> (veh)														0.0				
Control Delay (s/veh)							5.0							13.5				
Level of Service, LOS							Α							В				
Approach Delay (s/veh)						5.0							0.0					
Approach LOS						A												

	HCS7 Two-Way Stop	op-Control Report								
General Information		Site Information								
Analyst	Addie Kirkham	Intersection	Chapman Hwy at Nixon Rd							
Agency/Co.	FMA	Jurisdiction	Knox County							
Date Performed	3/27/2019	East/West Street	Nixon Road							
Analysis Year	2019	North/South Street	Chapman Hwy (SR 71)							
Time Analyzed	Existing PM Peak	Peak Hour Factor	0.97							
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25							
Project Description	588.004 Ancient Lore Village									



Vehicle '	Volumes and	l Adjustments
-----------	-------------	---------------

Approach		Eastbound			Westbound					North	bound		Southbound				
Movement	U	L	Т	R	U	L	T	R	U	L	T	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	0	0		0	1	0	0	0	2	0	0	1	2	0	
Configuration							LR				Т	TR		L	Т		
Volume, V (veh/h)						1		1			1286	7		3	1443		
Percent Heavy Vehicles (%)						2		2						2			
Proportion Time Blocked						0.000		0.000						0.000			
Percent Grade (%)						(	)										
Right Turn Channelized		No			No				Ν	lo		No					
Median Type/Storage	Undiv				vided												

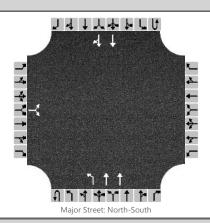
### **Critical and Follow-up Headways**

Base Critical Headway (sec)			7.5	6.9			4.1	
Critical Headway (sec)			6.84	6.94			4.14	
Base Follow-Up Headway (sec)			3.5	3.3			2.2	
Follow-Up Headway (sec)			3.52	3.32			2.22	

#### **Delay, Queue Length, and Level of Service**

Flow Rate, v (veh/h)				2				3		
				_				3		
Capacity, c (veh/h)				82				514		
v/c Ratio				0.02				0.01		
95% Queue Length, Q <sub>95</sub> (veh)				0.1				0.0		
Control Delay (s/veh)				49.9				12.1		
Level of Service, LOS				E				В		
Approach Delay (s/veh)			49	9.9				0	.0	
Approach LOS				E						

	HCS7 Two-Way Stop	p-Control Report	
General Information		Site Information	
Analyst	Addie Kirkham	Intersection	Chapman Hwy at W Dick For
Agency/Co.	FMA	Jurisdiction	City of Knoxville
Date Performed	8/21/2019	East/West Street	W Dick Ford Lane
Analysis Year	2019	North/South Street	Chapman Highway
Time Analyzed	Existing AM Peak	Peak Hour Factor	0.81
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	588.004 Ancient Lore Village		



Approach		Eastbound				Westl	oound	Westbound			bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	1	2	0	0	0	2	0
Configuration			LR							L	T				Т	TR
Volume, V (veh/h)		39		19						2	1437				758	50
Percent Heavy Vehicles (%)		2		2						2						
Proportion Time Blocked																
Percent Grade (%)		(	0													
Right Turn Channelized		N	lo			No				Ν	lo			Ν	lo	
Median Type/Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		7.5		6.9						4.1						
Critical Headway (sec)		6.84		6.94						4.14						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.52		3.32						2.22						
Delay, Queue Length, an	d Leve	of S	ervic	9												
Flow Rate, v (veh/h)	Т		71							2						
Capacity, c (veh/h)			92							689						

0.77

4.0

119.4

F

119.4

v/c Ratio

95% Queue Length,  $Q_{95}$  (veh)

Control Delay (s/veh)

Level of Service, LOS

Approach LOS

Approach Delay (s/veh)

**Vehicle Volumes and Adjustments** 

0.00

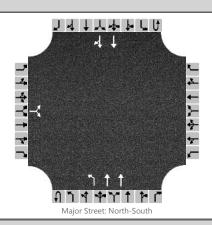
0.0

10.2

В

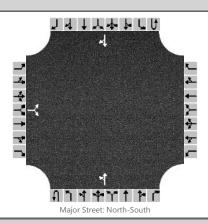
0.0

	HCS7 Two-Way Stop	p-Control Report	
General Information		Site Information	
Analyst	Addie Kirkham	Intersection	Chapman Hwy at W Dick For
Agency/Co.	FMA	Jurisdiction	City of Knoxville
Date Performed	8/21/2019	East/West Street	W Dick Ford Lane
Analysis Year	2019	North/South Street	Chapman Highway
Time Analyzed	Existing PM Peak	Peak Hour Factor	0.85
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	588.004 Ancient Lore Village		



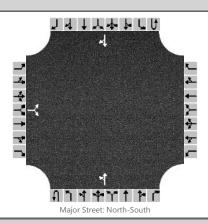
Vehicle Volumes and Ad	ljustme	ents																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	1	0		0	0	0	0	1	2	0	0	0	2	0		
Configuration			LR							L	Т				Т	TR		
Volume, V (veh/h)		10		13						8	1279				1433	64		
Percent Heavy Vehicles (%)		2		2						2								
Proportion Time Blocked																		
Percent Grade (%)			0															
Right Turn Channelized		Ν	10			No No								No				
Median Type/Storage				Undi	vided													
Critical and Follow-up H	leadwa	ıys																
Base Critical Headway (sec)		7.5		6.9						4.1								
Critical Headway (sec)		6.84		6.94						4.14								
Base Follow-Up Headway (sec)		3.5		3.3						2.2								
Follow-Up Headway (sec)		3.52		3.32						2.22								
Delay, Queue Length, ar	nd Leve	el of S	ervice	9														
Flow Rate, v (veh/h)			27							9								
Capacity, c (veh/h)			50							351								
v/c Ratio			0.54							0.03								
95% Queue Length, Q <sub>95</sub> (veh)			2.1							0.1								
Control Delay (s/veh)			141.2							15.5								
Level of Service, LOS			F							С								
Approach Delay (s/veh)		14	1.2	•						0	.1							
Approach LOS			F															

HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Addie Kirkham	Intersection	Sevierville Pike at Nixon								
Agency/Co.	FMA	Jurisdiction	Knox County								
Date Performed	4/1/2019	East/West Street	Nixon Road								
Analysis Year	2019	North/South Street	Sevierville Pike								
Time Analyzed	Existing AM Peak	Peak Hour Factor	0.67								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description 588.004 Ancient Lore Village											



Vehicle Volumes and Ad	justm	ents															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	U L T R			U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0	
Configuration			LR							LT						TR	
Volume, V (veh/h)		2		2						0	73				60	0	
Percent Heavy Vehicles (%)		2		2						2							
Proportion Time Blocked																	
Percent Grade (%)			0														
Right Turn Channelized		Ν	lo			Ν	lo			Ν	lo			Ν	lo		
Median Type/Storage				Undi	vided												
Critical and Follow-up H	leadwa	ıys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.42		6.22						4.12							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.52		3.32						2.22							
Delay, Queue Length, ar	nd Leve	el of S	ervic	<b>e</b>													
Flow Rate, v (veh/h)			6							0							
Capacity, c (veh/h)			869							1504							
v/c Ratio			0.01							0.00							
95% Queue Length, Q <sub>95</sub> (veh)			0.0							0.0							
Control Delay (s/veh)			9.2							7.4							
Level of Service, LOS		A							А								
Approach Delay (s/veh)		9.2							0.0								
Approach LOS		А															

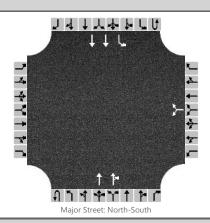
HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Addie Kirkham	Intersection	Sevierville Pike at Nixon								
Agency/Co.	FMA	Jurisdiction	Knox County								
Date Performed	4/1/2019	East/West Street	Nixon Road								
Analysis Year	2019	North/South Street	Sevierville Pike								
Time Analyzed	Existing PM Peak	Peak Hour Factor	0.78								
Intersection Orientation	0.25										
Project Description 588.004 Ancient Lore Village											



Vehicle Volumes and Ad	justm	ents															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	U L T R			U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0	
Configuration			LR							LT						TR	
Volume, V (veh/h)		3		7						1	53				144	2	
Percent Heavy Vehicles (%)		2		2						2							
Proportion Time Blocked																	
Percent Grade (%)			0														
Right Turn Channelized		Ν	lo			Ν	lo			Ν	lo			Ν	lo		
Median Type/Storage				Undi	vided												
Critical and Follow-up H	eadwa	ıys															
Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		6.42		6.22						4.12							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.52		3.32						2.22							
Delay, Queue Length, an	d Leve	el of S	ervic	9													
Flow Rate, v (veh/h)			13							1							
Capacity, c (veh/h)			813							1385							
v/c Ratio			0.02							0.00							
95% Queue Length, Q <sub>95</sub> (veh)			0.0							0.0							
Control Delay (s/veh)			9.5							7.6							
Level of Service, LOS		A							A								
Approach Delay (s/veh)		9.5							0.1								
Approach LOS		А															

## Attachment 6 Intersection Worksheets – Background AM/PM Peaks

HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Addie Kirkham	Intersection	Chapman Hwy at Nixon Rd								
Agency/Co.	FMA	Jurisdiction	Knox County								
Date Performed	3/27/2019	East/West Street	Nixon Road								
Analysis Year	2022	North/South Street	Chapman Hwy (SR 71)								
Time Analyzed	Background AM Peak	Peak Hour Factor	0.93								
Intersection Orientation North-South Analysis Time Period (hrs) 0.25											
Project Description 588.004 Ancient Lore Village											



Vehicle	<b>Volumes</b>	and Ad	justments
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Approach		Eastb	ound		Westbound				North	bound		Southbound				
Movement	U	L	T	R	U	L	T	R	U	L	Т	R	U	L	T	R
Priority		10	11	12		7	8	9	10	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	2	0	0	1	2	0
Configuration							LR				Т	TR		L	T	
Volume, V (veh/h)						0		0			1483	0		0	801	
Percent Heavy Vehicles (%)						2		2						2		
Proportion Time Blocked						0.000		0.000						0.000		
Percent Grade (%)						(	)									
Right Turn Channelized		N	lo			N	lo			N	lo			N	О	
Median Type/Storage				Undi	vided											

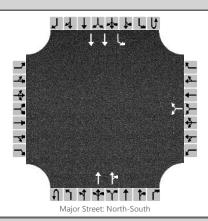
### **Critical and Follow-up Headways**

Base Critical Headway (sec)			7.5	6.9			4.1	
Critical Headway (sec)			6.84	6.94			4.14	
Base Follow-Up Headway (sec)			3.5	3.3			2.2	
Follow-Up Headway (sec)			3.52	3.32			2.22	

#### Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)				0				0		
Capacity, c (veh/h)				0				407		
v/c Ratio								0.00		
95% Queue Length, Q <sub>95</sub> (veh)								0.0		
Control Delay (s/veh)				5.0				13.8		
Level of Service, LOS				А				В		
Approach Delay (s/veh)			5	.0				0.	.0	
Approach LOS			,	Α						

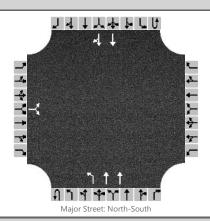
HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Addie Kirkham	Intersection	Chapman Hwy at Nixon Rd								
Agency/Co.	FMA	Jurisdiction	Knox County								
Date Performed	3/27/2019	East/West Street	Nixon Road								
Analysis Year	2022	North/South Street	Chapman Hwy (SR 71)								
Time Analyzed	Background PM Peak	Peak Hour Factor	0.97								
Intersection Orientation North-South Analysis Time Period (hrs) 0.25											
Project Description 588.004 Ancient Lore Village											



Vehicle Volumes and Adjustments																
			oound			\A/1	oound		ı	NI - alla	la a sal		г	C - 11-		
Approach	+	Eastr	oouna			westi	oouna		Northbound					South	bound	
Movement	U	L	Т	R	U	L	T	R	U	L	T	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	2	0	0	1	2	0
Configuration							LR				Т	TR		L	Т	
Volume, V (veh/h)						1		1			1325	7		3	1487	
Percent Heavy Vehicles (%)						2		2						2		
Proportion Time Blocked						0.000		0.000						0.000		
Percent Grade (%)							0									
Right Turn Channelized		١	No.			Ν	lo			١	10			Ν	lo	
Median Type/Storage				Undi	vided											
Critical and Follow-up H	leadwa	ays														
Base Critical Headway (sec)						7.5		6.9						4.1		
Critical Headway (sec)						6.84		6.94						4.14		
Base Follow-Up Headway (sec)						3.5		3.3						2.2		
Follow-Up Headway (sec)						3.52		3.32						2.22		
Delay, Queue Length, ar	nd Leve	el of S	Servic	е												
Flow Rate, v (veh/h)							2							3		
Capacity, c (veh/h)							75							496		
v/c Ratio							0.03							0.01		
95% Queue Length, Q <sub>95</sub> (veh)							0.1							0.0		
Control Delay (s/veh)							54.3							12.3		
Level of Service, LOS						F							В			
Approach Delay (s/veh)					54.3								0.0			
	_								-				-			

Approach LOS

HCS7 Two-Way Stop-Control Report											
General Information Site Information											
Analyst	Addie Kirkham	Intersection	Chapman Hwy at W Dick For								
Agency/Co.	FMA	Jurisdiction	City of Knoxville								
Date Performed	8/21/2019	East/West Street	W Dick Ford Lane								
Analysis Year	2022	North/South Street	Chapman Highway								
Time Analyzed	Background AM Peak	Peak Hour Factor	0.81								
Intersection Orientation	North-South	Analysis Time Period (hrs) 0.25									
Project Description 588.004 Ancient Lore Village											



Vehicle Volumes	and	Adjustments
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Approach		Eastb	ound		Westbound				Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	1	2	0	0	0	2	0	
Configuration			LR							L	Т				Т	TR	
Volume, V (veh/h)		40		20						2	1481				781	52	
Percent Heavy Vehicles (%)		2		2						2							
Proportion Time Blocked																	
Percent Grade (%)		(	0														
Right Turn Channelized		N	lo		No No				No								
Median Type/Storage				Undi	ndivided												

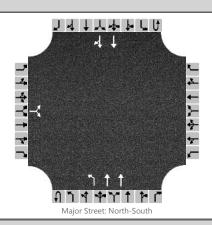
### **Critical and Follow-up Headways**

Base Critical Headway (sec)	7.5	6.9			4.1			
Critical Headway (sec)	6.84	6.94			4.14			
Base Follow-Up Headway (sec)	3.5	3.3			2.2			
Follow-Up Headway (sec)	3.52	3.32			2.22			

#### **Delay, Queue Length, and Level of Service**

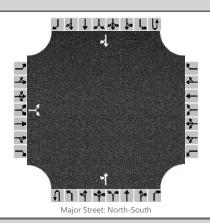
Flow Rate, v (veh/h)		74				2				
Capacity, c (veh/h)		86				671				
v/c Ratio		0.86				0.00				
95% Queue Length, Q <sub>95</sub> (veh)		4.6				0.0				
Control Delay (s/veh)		145.2				10.4				
Level of Service, LOS		F				В				
Approach Delay (s/veh)	14	5.2				0	.0			
Approach LOS		F								

HCS7 Two-Way Stop-Control Report											
General Information		Site Information									
Analyst	Addie Kirkham	Intersection	Chapman Hwy at W Dick For								
Agency/Co.	FMA	Jurisdiction	City of Knoxville								
Date Performed	8/21/2019	East/West Street	W Dick Ford Lane								
Analysis Year	2022	North/South Street	Chapman Highway								
Time Analyzed	Background PM Peak	Peak Hour Factor	0.85								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description 588.004 Ancient Lore Village											



<b>Vehicle Volumes and Ad</b>	justme	ents																
Approach	T	Easth	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	1	0		0	0	0	0	1	2	0	0	0	2	0		
Configuration			LR							L	Т				Т	TR		
Volume, V (veh/h)		10		13						8	1318				1477	66		
Percent Heavy Vehicles (%)		2		2						2								
Proportion Time Blocked																		
Percent Grade (%)			0															
Right Turn Channelized		١	lo			Ν	lo			Ν	lo		No					
Median Type/Storage				Undi	vided													
Critical and Follow-up H	d Follow-up Headways																	
Base Critical Headway (sec)	Т	7.5		6.9						4.1								
Critical Headway (sec)		6.84		6.94						4.14								
Base Follow-Up Headway (sec)		3.5		3.3						2.2								
Follow-Up Headway (sec)		3.52		3.32						2.22								
Delay, Queue Length, ar	d Leve	el of S	ervice	9														
Flow Rate, v (veh/h)	Т		27							9								
Capacity, c (veh/h)			44							334								
v/c Ratio			0.61							0.03								
95% Queue Length, Q <sub>95</sub> (veh)			2.3							0.1								
Control Delay (s/veh)			170.8							16.1								
Level of Service, LOS		F								С								
Approach Delay (s/veh)						0.1												
Approach LOS		F																

HCS7 Two-Way Stop-Control Report											
General Information		Site Information									
Analyst	Addie Kirkham	Intersection	Sevierville Pike at Nixon								
Agency/Co.	FMA	Jurisdiction	Knox County								
Date Performed	4/1/2019	East/West Street	Nixon Road								
Analysis Year	2022	North/South Street	Sevierville Pike								
Time Analyzed	Background AM Peak	Peak Hour Factor	0.67								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description 588.004 Ancient Lore Village											



Vehicle	Volumes	and A	Adjustments
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Approach		Eastbound Westbound			Northbound				Southbound							
Movement	U	L	Т	R	U	L	T	R	U	L	Т	R	U	L	T	R
Priority		10	11	12		7	8	9	10	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume, V (veh/h)		2		2						0	75				62	0
Percent Heavy Vehicles (%)		2		2						2						
Proportion Time Blocked																
Percent Grade (%)		(	)													
Right Turn Channelized		N	lo		No			No No								
Median Type/Storage				Undi	livided											

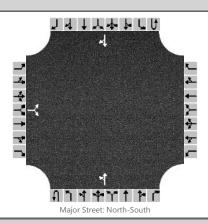
## Critical and Follow-up Headways

Base Critical Headway (sec)	7.1	6.2			4.1			
Critical Headway (sec)	6.42	6.22			4.12			
Base Follow-Up Headway (sec)	3.5	3.3			2.2			
Follow-Up Headway (sec)	3.52	3.32			2.22			

#### **Delay, Queue Length, and Level of Service**

Flow Rate, v (veh/h)		6				0				
Capacity, c (veh/h)		864				1500				
v/c Ratio		0.01				0.00				
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0				
Control Delay (s/veh)		9.2				7.4				
Level of Service, LOS		А				А				
Approach Delay (s/veh)	9	.2				0	.0			
Approach LOS	-	4								

HCS7 Two-Way Stop-Control Report											
General Information		Site Information									
Analyst	Addie Kirkham	Intersection	Sevierville Pike at Nixon								
Agency/Co.	FMA	Jurisdiction	Knox County								
Date Performed	4/1/2019	East/West Street	Nixon Road								
Analysis Year	2022	North/South Street	Sevierville Pike								
Time Analyzed	Background PM Peak	Peak Hour Factor	0.78								
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25								
Project Description 588.004 Ancient Lore Village											



Vehicle Volumes and Ad	justmo	ents														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume, V (veh/h)		3		7						1	55				148	2
Percent Heavy Vehicles (%)		2		2						2						
Proportion Time Blocked																
Percent Grade (%)			0													
Right Turn Channelized		١	lo			Ν	lo			Ν	lo			١	10	
Median Type/Storage				Undi	vided											
Critical and Follow-up H	eadwa	ıys														
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		6.42		6.22						4.12						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.52		3.32						2.22						
Delay, Queue Length, ar	d Leve	el of S	ervice	9												
Flow Rate, v (veh/h)	Т		13							1						
Capacity, c (veh/h)			806							1379						
v/c Ratio			0.02							0.00						
95% Queue Length, Q <sub>95</sub> (veh)			0.0							0.0						
Control Delay (s/veh)			9.5							7.6						
Level of Service, LOS			А							Α						
Approach Delay (s/veh)	13 806 0.02 0.0 9.5								0	.1						
Approach LOS			Α													

## Attachment 7 Intersection Worksheets – Full Buildout AM/PM Peaks

#### **HCS7 Signalized Intersection Results Summary** 7474176 **General Information Intersection Information** FMA Duration, h 0.25 Agency Analyst Addie Kirkham Analysis Date 8/21/2019 Area Type Other PHF 0.92 Jurisdiction City of Knoxville Time Period Buildout AM Peak **Urban Street** Chapman Highway Analysis Year 2022 **Analysis Period** 1>7:00 Chapman Hwy At Nixon/... File Name Buildout AM Peak Signalized.xus Intersection **Project Description** 588.004 Ancient Lore Village **Demand Information** EΒ **WB** NB SB Approach Movement R L R L R L R Demand (v), veh/h 40 0 20 3 0 5 2 1481 2 18 781 52 **Signal Information** Л. Cycle, s 120.0 Reference Phase 2 Offset, s 0 Reference Point End 0.0 Green 0.4 90.3 5.8 1.5 0.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 0.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 1.5 1.5 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 5 6 Case Number 12.0 11.0 1.0 4.0 8.3 Phase Duration, s 11.3 7.0 5.9 101.7 95.8 5.5 Change Period, (Y+Rc), s 5.5 5.5 5.5 5.5 Max Allow Headway ( MAH ), s 3.1 3.1 3.0 0.0 0.0 Queue Clearance Time ( $g_s$ ), s 6.5 2.4 2.0 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 Phase Call Probability 0.89 0.25 0.07 0.00 Max Out Probability 1.00 0.00 WB SB **Movement Group Results** EΒ NB Approach Movement L Т R L Т R Т R L Т R L **Assigned Movement** 7 4 14 3 18 5 2 12 1 6 16 8 65 3 5 2 806 806 466 459 Adjusted Flow Rate (v), veh/h 1781 1870 1869 1688 1665 Adjusted Saturation Flow Rate (s), veh/h/ln 1711 1585 1781 0.2 Queue Service Time ( $g_s$ ), s 4.5 0.4 0.0 18.0 18.0 0.0 11.3 Cycle Queue Clearance Time ( $g_c$ ), s 4.5 0.2 0.4 0.0 18.0 18.0 9.9 11.3 Green Ratio (g/C) 0.05 0.01 0.01 0.77 0.80 0.80 0.75 0.75 Capacity (c), veh/h 83 22 20 471 1499 1499 1301 1253 Volume-to-Capacity Ratio (X) 0.789 0.145 0.272 0.005 0.538 0.538 0.358 0.367 Back of Queue (Q), ft/ln (95 th percentile) 93.1 4.7 7.9 206.5 204 137.7 141 0.4 Back of Queue (Q), veh/ln (95 th percentile) 3.7 0.2 0.3 0.0 8.1 8.2 5.5 5.6 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1), s/veh 56.5 58.6 58.7 3.9 4.2 4.2 4.9 5.1 Incremental Delay ( d 2 ), s/veh 6.1 1.1 2.7 0.0 1.4 1.4 8.0 8.0 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d), s/veh 62.6 59.7 61.4 3.9 5.5 5.5 5.7 5.9 Level of Service (LOS) Ε F Е Α Α Α Α Α 62.6 Ε 60.8 Ε Α 5.8 Approach Delay, s/veh / LOS 5.5 Α Intersection Delay, s/veh / LOS 7.2 Α **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS С С 2.9 2.8 2.2 В 2.0 В Bicycle LOS Score / LOS 0.6 Α 0.5 Α 1.8 В 1.3 Α

#### **HCS7 Signalized Intersection Results Summary** 7474176 **General Information Intersection Information** FMA Duration, h 0.25 Agency Analyst Addie Kirkham Analysis Date 8/21/2019 Area Type Other PHF 0.92 Jurisdiction City of Knoxville Time Period Buildout PM Peak **Urban Street** Chapman Highway Analysis Year 2022 **Analysis Period** 1>7:00 Chapman Hwy At Nixon/... File Name Buildout PM Peak Signalized.xus Intersection **Project Description** 588.004 Ancient Lore Village **Demand Information** EΒ **WB** NB SB Approach Movement L R L R L R R Demand (v), veh/h 10 0 13 9 0 10 8 1318 9 19 1477 66 **Signal Information** Л. Cycle, s 120.0 Reference Phase 2 Offset, s 0 Reference Point End 0.0 Green 1.5 90.1 3.4 3.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 0.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 1.5 1.5 1.5 1.5 0.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 4 8 2 5 6 Case Number 12.0 11.0 1.0 4.0 8.3 Phase Duration, s 8.9 8.5 7.0 102.6 95.6 5.5 Change Period, (Y+Rc), s 5.5 5.5 5.5 5.5 Max Allow Headway ( MAH ), s 3.1 3.1 3.0 0.0 0.0 Queue Clearance Time ( $g_s$ ), s 3.8 2.8 2.1 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 Phase Call Probability 0.57 0.50 0.25 0.00 0.00 0.00 Max Out Probability WB SB **Movement Group Results** EΒ NB Approach Movement L Т R L Т R Т R L Т R L **Assigned Movement** 7 4 14 3 18 5 2 12 1 6 16 8 25 10 11 9 722 721 883 815 Adjusted Flow Rate (v), veh/h 1781 1866 1787 1675 Adjusted Saturation Flow Rate (s), veh/h/ln 1665 1585 1781 1870 14.4 14.4 Queue Service Time ( $g_s$ ), s 1.8 0.6 8.0 0.1 0.0 28.3 28.3 Cycle Queue Clearance Time ( $g_c$ ), s 1.8 0.6 8.0 0.1 14.4 14.4 26.7 Green Ratio (g/C) 0.03 0.02 0.02 0.78 0.81 0.81 0.75 0.75 Capacity (c), veh/h 47 44 39 234 1514 1510 1373 1258 Volume-to-Capacity Ratio (X) 0.531 0.221 0.275 0.037 0.477 0.477 0.643 0.647 Back of Queue (Q), ft/ln (95 th percentile) 35.3 13.4 15.1 158.3 156.4 330.3 318.4 2 Back of Queue (Q), veh/ln (95 th percentile) 1.4 0.5 0.6 0.1 6.2 6.3 13.2 12.7 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1), s/veh 57.5 57.4 57.4 6.9 3.6 3.6 7.1 7.2 Incremental Delay ( d 2 ), s/veh 3.4 0.9 1.4 0.0 1.1 1.1 2.3 2.6 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d), s/veh 60.9 58.3 58.8 6.9 4.6 4.6 9.4 9.8 Level of Service (LOS) Ε F Е Α Α Α Α Α 60.9 Ε 58.6 Ε 4.6 Α 9.6 Approach Delay, s/veh / LOS Α Intersection Delay, s/veh / LOS 8.1 Α **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS С С 2.2 2.9 2.8 В 2.0 В Bicycle LOS Score / LOS 0.5 Α 0.5 Α 1.7 В 1.9

# Attachment 8 Turn Lane Warrant Analysis

**Project: Ancient Lore Village at Boyd Hollow** 

Sevierville Pike at at Employee Parking Lot	VOLUMES				
LEFT TURN	Opposing	Thru	LT	LT MAX	Warrant Met
AM	62	75	-	250	NO
PM	148	55	-	250	NO
Sevierville Pike at at Employee Parking Lot	VOLUMES				
RIGHT TURN		Thru	RT	RT MAX	Warrant Met
AM	_	62	-	549	NO
PM		148	-	449	NO

TABLE 5A

## LEFT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

(If the left-turn volume exceeds the table value a left -turn lane is needed)

OPPOSING	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *									
VOLUME	100 - 149	150 - 199	200 - 249	250 - 299	300 - 349	350 - 399				
100 - 149 150 - 199	250 AM/P	M Peak 80	140 105	110 90	80 70	70 60				
200 - 249	160	115	85	75	65	55				
250 - 299	130	100	75	65	60	50				
300 - 349	110	90	70	60	55	45				
350 - 399		80	65	55	50	40				
490 - 449	90	70	60	50	45	35				
450 - 499	S0	65	55	45	40	30				
500 - 549	70	60	45	35	35	25				
550 - 599	65	55	40	35	30	25				
600 - 649	60	45	35	30	25	25				
650 - 699	55	35	35	30	25	20				
700 - 749	50	35	30	25	20	20				
750 or More	45	35	25	25	20	20				

OPPOSING	THROU	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *								
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 599	=/ >600				
100 - 149	70	60	50	45	40	35				
150 - 199	60	55	45	40	35	30				
200 - 249	55	50	40	35	30	30				
250 - 299	50	45	35	30	30	30				
300 - 349	45	40	35	30	25	25				
350 - 399	40 ·	35	30	25	25	20				
400 - 449	35	30	30	25	20	20				
450 - 499	30	25	25	20	20	20				
500 - 549	25	25	20	20	20	15				
550 - 599	25	20	20	20	20	15				
600 - 649	25	20	20	20	20	15				
650 - 699	20	20	20	20	20	15				
790 - 749 750 or More	20 20	20 20	20 20	15 15	15 15	15 15				

<sup>\*</sup> Or through volume only if a right-turn lane exists

TABLE 5B

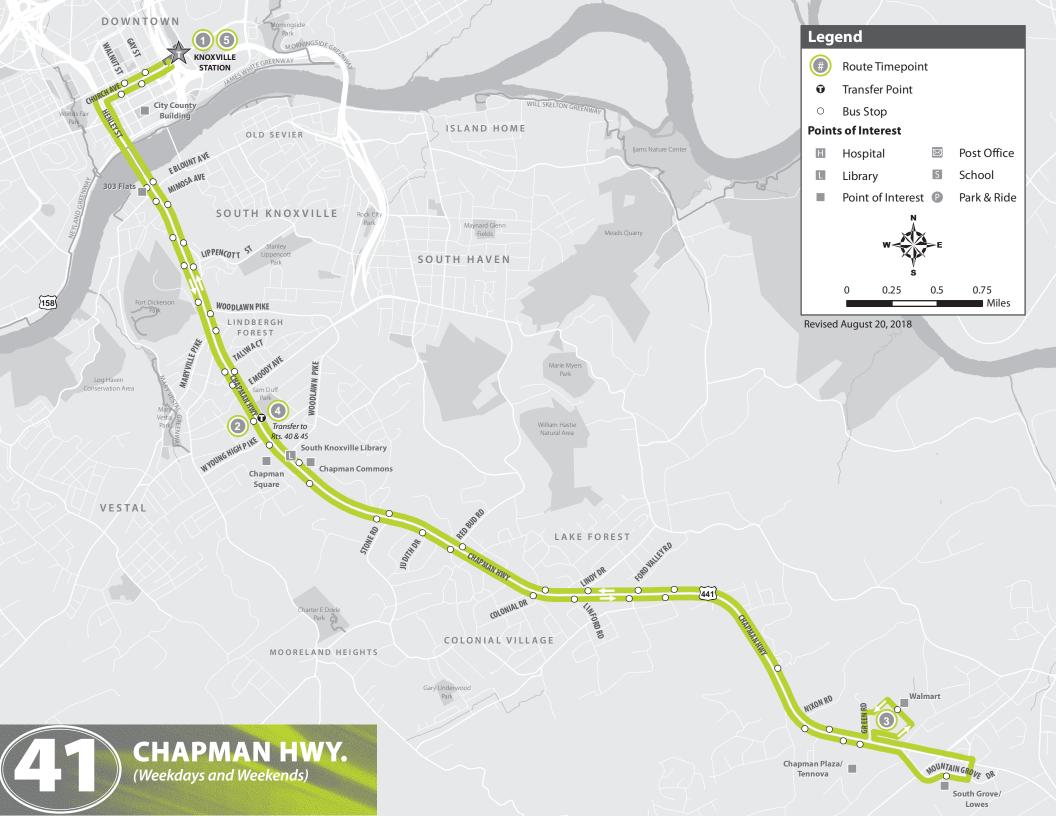
## RIGHT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

RIGHT-TURN	THRO	THROUGH VOLUME PLUS LEFT-TURN VOLUME *									
VOLUME	<100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399					
Fewer Than 25 25 - 49 50 - 99	AM Peak	PM Peak									
100 - 149 150 - 199				ļ. <u> </u>	_						
200 - 249 250 - 299			<u> </u>		Yes	Yes Yes					
300 - 349 350 - 399			Yes	Ves Yes	Yes Yes	Yes Yes					
400 - 449 450 - 499		Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes					
500 - 549 550 - 599	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes					
600 or More	Yes	Yes	Yes	Yes	Yes	Yes					

RIGHT-TURN	THR	THROUGH VOLUME PLUS LEFT-TURN VOLUME *							
VOLUME	350 - 399	350 - 399 400 - 449		500 - 549	550 - 600	+/>600			
Fewer Than 25 25 - 49 50 - 99				Yes	Yes Yes	Yes Yes			
100 - 149 150 - 199		Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
200 - 249 250 - 299	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
300 - 349 350 - 399	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
400 - 449 450 - 499	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
500 - 549 550 - 599	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
600 or More	Yes	Yes	Yes	Yes	Yes	Yes			

<sup>\*</sup> Or through volume only if a left-turn lane exists.

## Attachment 9 Transit Network





## **CHAPMAN HIGHWAY**

(Weekdays and Weekends)

IMPROVED SATURDAY SERVICE HOURS AUGUST 2018

#### **SERVES:**

Chapman Commons
Chapman Plaza
Chapman Square
Knoxville Station/Downtown
South Grove Shopping Center

South Knoxville Branch Library Tennova South Walmart



**Effective Date: January 2019** 

8:20

8:40

Going a	way from Dowi	ntown	Going toward	Downtown
Knoxville Station— Platform P	Chapman Hwy. past Moody	Walmart	Chapman Hwy. past Young High Pike	Knoxville Station
1	2	3	4	5

	V	VEEKDA'	Y SCHED	ULE	
A.M.			5:41	5:50	6:10
	6:15	6:26	6:41	6:50	7:10
	6:45	6:56	7:11	7:20	7:40
	7:15	7:26	7:41	7:50	8:10
	7:45	7:56	8:11	8:20	8:40
	8:15	8:26	8:41	8:50	9:10
	8:45	8:56	9:11	9:20	9:40
	9:15	9:26	9:41	9:50	10:10
	9:45	9:56	10:11	10:20	10:40
	10:15	10:26	10:41	10:50	11:10
	10:45	10:56	11:11	11:20	11:40
	11:15	11:26	11:41	11:50	12:10
	11:45	11:56	12:11	12:20	12:40
P.M.	12:15	12:26	12:41	12:50	1:10
	12:45	12:56	1:11	1:20	1:40
	1:15	1:26	1:41	1:50	2:10
	1:45	1:56	2:11	2:20	2:40
	2:15	2:26	2:41	2:50	3:10
	2:45	2:56	3:11	3:20	3:40
	3:15	3:26	3:41	3:50	4:10
	3:45	3:56	4:11	4:20	4:40
	4:15	4:26	4:41	4:50	5:10
	4:45	4:56	5:11	5:20	5:40
	5:15	5:26	5:41	5:50	6:10
	5:45	5:56	6:11	6:20	6:40
	6:15	6:26	6:41	6:50	7:10
	6:45	6:56	7:11	7:20	7:40
	7:15	7:26	7:41	7:50	8:10
	7:45	7:56	8:11	8:20	8:40
	8:15	8:26	8:41	8:50	9:10
	8:45	8:56	9:11	9:20	9:40
	0.15	9:26	9:41	9:50	10:10
	9:15				
	9:15	9:56	10:11	10:20	10:40
		9:56 10:26	10:11 10:41	10:20 10:50	10:40 11:10

Going a	way from Down	town	Going towar	d Downtown					
Knoxville Station— Platform P	Chapman Hwy. past Moody	Walmart	Chapman Hwy. past Young High Pike	Knoxville Station					
1	2	3	4	5					
SATURDAY SCHEDULE									

8:11

7:45

	7.45	7.50	0.11	0.20	0.40
	8:15	8:26	8:41	8:50	9:10
	8:45	8:56	9:11	9:20	9:40
	9:15	9:26	9:41	9:50	10:10
	9:45	9:56	10:11	10:20	10:40
	10:15	10:26	10:41	10:50	11:10
	10:45	10:56	11:11	11:20	11:40
	11:15	11:26	11:41	11:50	12:10
	11:45	11:56	12:11	12:20	12:40
P.M.	12:15	12:26	12:41	12:50	1:10
	12:45	12:56	1:11	1:20	1:40
	1:15	1:26	1:41	1:50	2:10
	1:45	1:56	2:11	2:20	2:40
	2:15	2:26	2:41	2:50	3:10
	2:45	2:56	3:11	3:20	3:40
	3:15	3:26	3:41	3:50	4:10
	3:45	3:56	4:11	4:20	4:40
	4:15	4:26	4:41	4:50	5:10
	4:45	4:56	5:11	5:20	5:40
	5:15	5:26	5:41	5:50	6:10
	5:45	5:56	6:11	6:20	6:40
	6:15	6:26	6:41	6:50	7:10
	6:45	6:56	7:11	7:20	7:40
	7:15	7:26	7:41	7:50	8:10
	7:45	7:56	8:11	8:20	8:40
	8:15	8:26	8:41	8:50	9:10
	8:45	8:56	9:11	9:20	9:40
	9:15	9:26	9:41	9:50	10:10
	9:45	9:56	10:11	10:20	10:40
	10:15	10:26	10:41	10:50	11:10
	10:45	10:56	11:11	11:20	11:40
	11:15	11:26	11:41	11:50	To Garage
		SUNDAY	SCHED	UI F	
A.M.	8:15	8:26	8:41	8:50	9:10
A.IVI.	9:15	9:26	9:41	9:50	10:10
	10:15	10:26	10:41	10:50	11:10
	11:15	11:26	11:41	11:50	12:10
P.M.	12:15	12:26	12:41	12:50	1:10
L·IAI.	1:15	1:26	1:41	1:50	2:10
	2:15	2:26	2:41	2:50	3:10
	3:15	3:26	3:41	3:50	4:10
	4:15	4:26	4:41	4:50	5:10
	5:15	5:26	5:41	5:50	6:10
	6:15	6:26	6:41	6:50	7:10
	7:15	7:26	7:41	7:50	8:10
	7:15				0:10
	8:15	8:26	8:41	To Garage	

Need help reading this schedule?

Need other general information on how to ride?

Click here to Download the General Schedule Information pdf available from katbus.com