

BUTLER PROPERTY INDUSTRIAL PARK *Knox County, Tennessee*

TRAFFIC IMPACT STUDY

Prepared For:

**THE DEVELOPMENT CORPORATION
of KNOXVILLE**

Prepared By:



November 2013

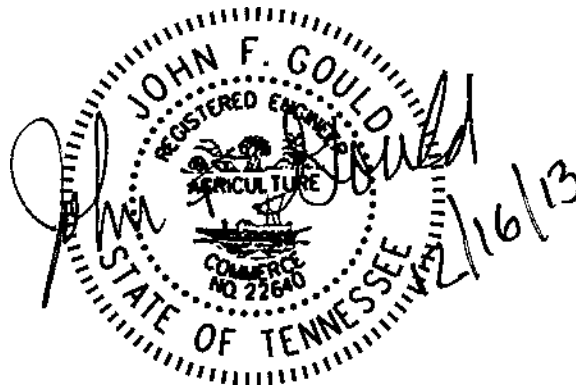
BUTLER PROPERTY DEVELOPMENT

Knox County, Tennessee

TRAFFIC IMPACT STUDY

Prepared for

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December 2013

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EXECUTIVE SUMMARY

This study was commissioned to address the impact and access of a industrial development in Knox County, Tennessee. The study required the collection of traffic data, generation of anticipated traffic volumes from the proposed site and development of projected traffic volumes from normal growth and from the development site. Analysis of the resulting traffic projections was conducted to determine the capacity and levels of service for the proposed site access. This study will develop measures necessary to mitigate traffic impacts including improved roadway geometrics and traffic control devices within the environs of the proposed development. Discussions with the Knox County determined that the study scope would address the proposed development access to the adjacent roadway facility and the adjacent intersections of Garrison Drive at Byington Beaver Ridge Road and the planned Karns Valley Connector.

The proposed development is approximately 70 acres, currently vacant and proposed for light manufacturing use. Current zoning of the study property is agriculture with a designation for low density residential in the current land use plan. The proposed industrial development, at build-out, may be a maximum of 855,000 square-feet of industrial space.

Background traffic, traffic that may be anticipated regardless of the proposed development, was determined using a 2-percent annual growth rate until the assumed build-out year of 2018. Trips associated with a planned residential development by others was also distributed and included in the background traffic condition.

The manufacturing land use was assumed for this development, for it more reflected the goal employment of 580 and the uses identified by the Development Corporation. Trips generated for 855,000 square feet of light industrial space were found to be 3,300 daily trips, 680 AM peak hour trips, and 650 PM peak hour trips. A conservative 10-percent of the trips were also assigned to Garrison Drive.

Using the identified turning movements for the existing and projected traffic conditions, with and without the proposed development, capacity and level of service analyses were conducted using the **2010 Highway Capacity Manual**. Analyses determined that with recommendation of this study, acceptable levels of service could be maintained. Signalization and turn lanes were determined necessary to mitigated poor levels of service that may otherwise occur without any mitigation.

The intersection of Byington Beaver Ridge Road and Garrison Drive currently meets the Peak Hour signal warrant, thereby mitigating a failing level of service. Signalization of the Byington Beaver Ridge Road and Garrison Drive intersection should provide a very good level of service. The industrial site, at build-out will warrant signalization of the Karns Valley Connector and Garrison Road intersection. The realignment of the industrial access road and Garrison Drive intersection provides for the primary traffic movement between the Karns Valley Connector and the industrial access. Teeing Garrison Drive to the industrial access with a STOP control provides for an acceptable level of service and should deter traffic on Garrison Drive. Sufficient storage should be provided for the westbound approach to Karns Valley Connector to minimize

delay and queues. With a 2018 projected 1,111 daily traffic for Garrison Drive, the proposed development reflects approximately 30-percent of the traffic, representing a maximum impact given good access to Karns Valley Connector.

The projected traffic volumes, analyses conducted, and Knox County policies identified the following improvements necessary for the maintenance of an acceptable LOS during the peak hours:

1. Provide minimum 375-foot left- and right-turn lanes for the westbound approach to the Karns Valley Connector.
2. Provide a minimum 350-foot left-turn lane for the southbound movement from Karns Valley Connector to Garrison Drive.
3. Provide a minimum 250-foot right-turn lane for the northbound movement from Karns Valley Connector to Garrison Drive.
4. Signalize the intersection of Byington Beaver Ridge Road and Garrison Drive for the mitigation of existing conditions.
5. Monitor the intersection of Karns Valley Connector and Garrison Drive for signalization. Install a traffic signal when warrant can be satisfied and provide for right-turn overlaps to and from Garrison Drive.
6. Realign the intersection of Garrison Drive and the industrial access assigning the right of way to the industrial development.
7. Minimize landscaping, using low growing vegetation and signing at the planned accesses to insure that safe sight-distance is maintained.
8. Roadway and intersection design should conform to the recommended standards and practices adopted by the Institute of Transportation Engineers (ITE), American Association of State and Highway Officials (AASHTO), Lee County, and the Virginia Department of Transportation.

A summary of recommendations are provided in **Figure ES1**. The blue color represents improvements identified due to the proposed project impacts and the orange color represents improvements identified due to impacts of others.

SUMMARY OF RECOMMENDATIONS
Butler Property Industrial Park
Karns, TN

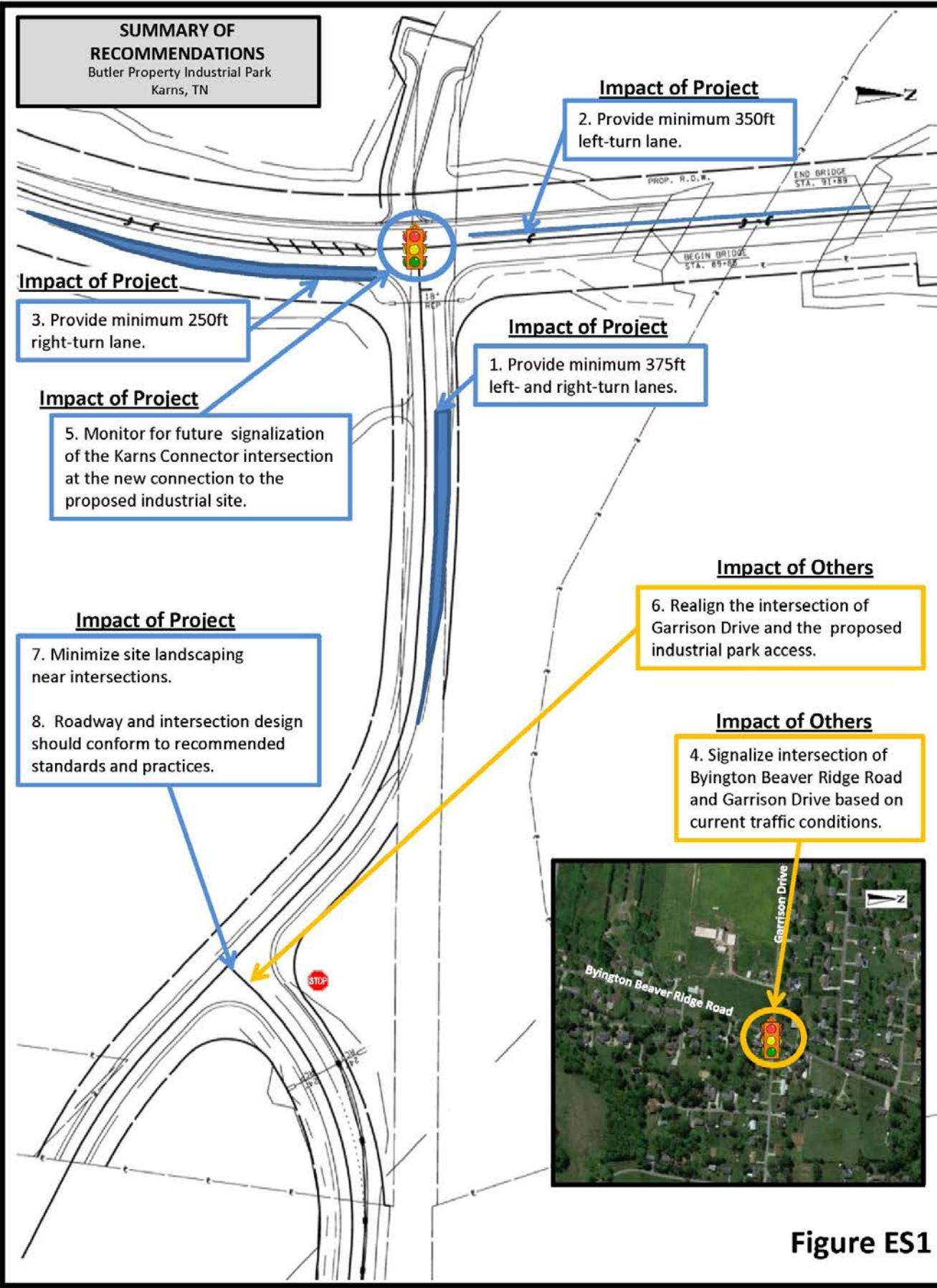


Figure ES1

INTRODUCTION

This study was commissioned to address the impact and access of an industrial development in Knox County, Tennessee. The study required the collection of traffic data, generation of anticipated traffic volumes from the proposed site and development of projected traffic volumes from normal growth and from the development site. Analysis of the resulting traffic projections was conducted to determine the capacity and levels of service for the proposed site access. This study will develop measures necessary to mitigate traffic impacts including improved roadway geometrics and traffic control devices within the environs of the proposed development. Discussions with Knox County determined that the study scope would address the proposed development access to the adjacent roadway facility and the intersections of Garrison Drive with the existing Byington Beaver Ridge Road and planned Karns Valley Connector.

Proposed Site Uses

The proposed development is approximately 70 acres, currently vacant and proposed for light manufacturing use. Current zoning of the study property is agriculture with a designation for low density residential in the current land use plan. The proposed industrial development, at build-out, may be a maximum of 855,000 square-feet of industrial space. The employment goal for the site is approximately 580. **Figure 1** illustrates the concept site plan for the development.

Site Location

The location of the site is in Knox County, south of Garrison Drive, between the proposed Karns Connector and Byington Beaver Ridge Road. The site is located northwest of the Knoxville central business district (CBD). **Figure 2** is a vicinity map illustrating the site relative to local and regional access.

SITE CONCEPT PLAN
 Butler Property Industrial Park
 Karns, TN



LOCAL AND REGIONAL ACCESS

The proposed development will add trips to both the local and regional facilities. Intersections between these facilities will be evaluated to determine the development's impact upon them. A brief description of these facilities is provided in the section.

Local Access

Access to the site is from a proposed street intersecting with Garrison Drive. Garrison Drive is a 16-foot minor collector facility extending from Byington Solway Road to Byington Beaver Ridge Road (S.R. 131). The 2012 average daily traffic on Garrison Drive is 540. Byington Beaver Ridge Road, east of the site, is a 2-lane secondary state route extending north and south of the site, intersecting Oak Ridge Highway (S.R. 62) north of the site and Byington Solway Road to the south.

Regional Access

Byington Beaver Ridge Road (S.R. 131), to the south, intersects Byington Solway Road, which to the west, intersects Ball Camp Byington Road providing for the state route to continue to Hardin Valley Road and Middlebrook Pike. From Middlebrook Pike, State Route 131 continues to Kingston Pike as Lovell Road intersecting Pellissippi Parkway (S.R. 162) and Interstate 40/75. Ball Camp Byington Road intersection with Byington Solway Road has a one lane Railroad underpass that limits the efficient flow of traffic. Ball Camp Byington Road has an 2012 ADT of approximately 11,170.

Plans exist for the connection of Karns Valley Drive north and south of Oak Ridge Highway, providing an improved connection from Hardin Valley Road via Westcott Boulevard north to West Emory Road, intersecting Oak Ridge Highway. With completion of the Karns Valley Road Connection to the Oak Ridge Highway and an associated reduction in traffic on Byington Beaver Ridge and Ball Camp Byington Road, access should improve at the existing Ball Camp Byington Road railroad crossing to the south.

Emory Road (S.R. 131) is the extension of the State's secondary facility to the northeast intersecting Clinton Highway (U.S. 25W), Interstate 75 North, Norris Freeway (U.S. 441), Maynardville Pike (S.R. 33), and Tazewell Pike (S.R. 331). Emory Road extends to Washington Pike which connects to State Hwy 61 to the east. This east and west facility provides many connections to the north.

North of the site, Oak Ridge Highway (S.R. 62) is a two-lane primary state route extending east to Knoxville and west to Pellissippi Parkway (S.R. 162) which extends into Oak Ridge. The 2012 average daily traffic (ADT) on the Oak Ridge Highway is 15,510 to the east and 11,200 to the west near the Anderson county line.

Pellissippi Parkway (S.R. 162) is a 4-lane divided expressway extending north to Oak Ridge and south becoming I-140 with an interchange with Interstate 40/75. Oak Ridge Highway (S.R. 62) has an interchange with Interstate 640 in Knoxville to the east. To the south, S.R. 131 also intersects Pellissippi Parkway (S.R. 162). Interstates 40 and 75 provides regional access throughout Tennessee, I-40 extends east and west from Memphis to Asheville, North Carolina, and I-75 extending north and south connects Lexington, Kentucky and Chattanooga Tennessee through Knoxville.

EXISTING TRAFFIC CONDITIONS

Existing Traffic Volumes and Intersection Geometry

A turning movement count (TMC) was conducted in October of 2013 for the intersection of Byington Beaver Ridge Road and Garrison Drive. The count was conducted 7:00-9:00AM and 2:00-6:00PM. The AM and PM peak hours were found between 7:15-8:15AM and 5:00-6:00PM. The posted speed limit for Garrison Drive is 30mph. Traffic control and intersection geometry are illustrated in **Figure 3**, and **Figure 4** illustrates the existing peak-hour traffic volumes.

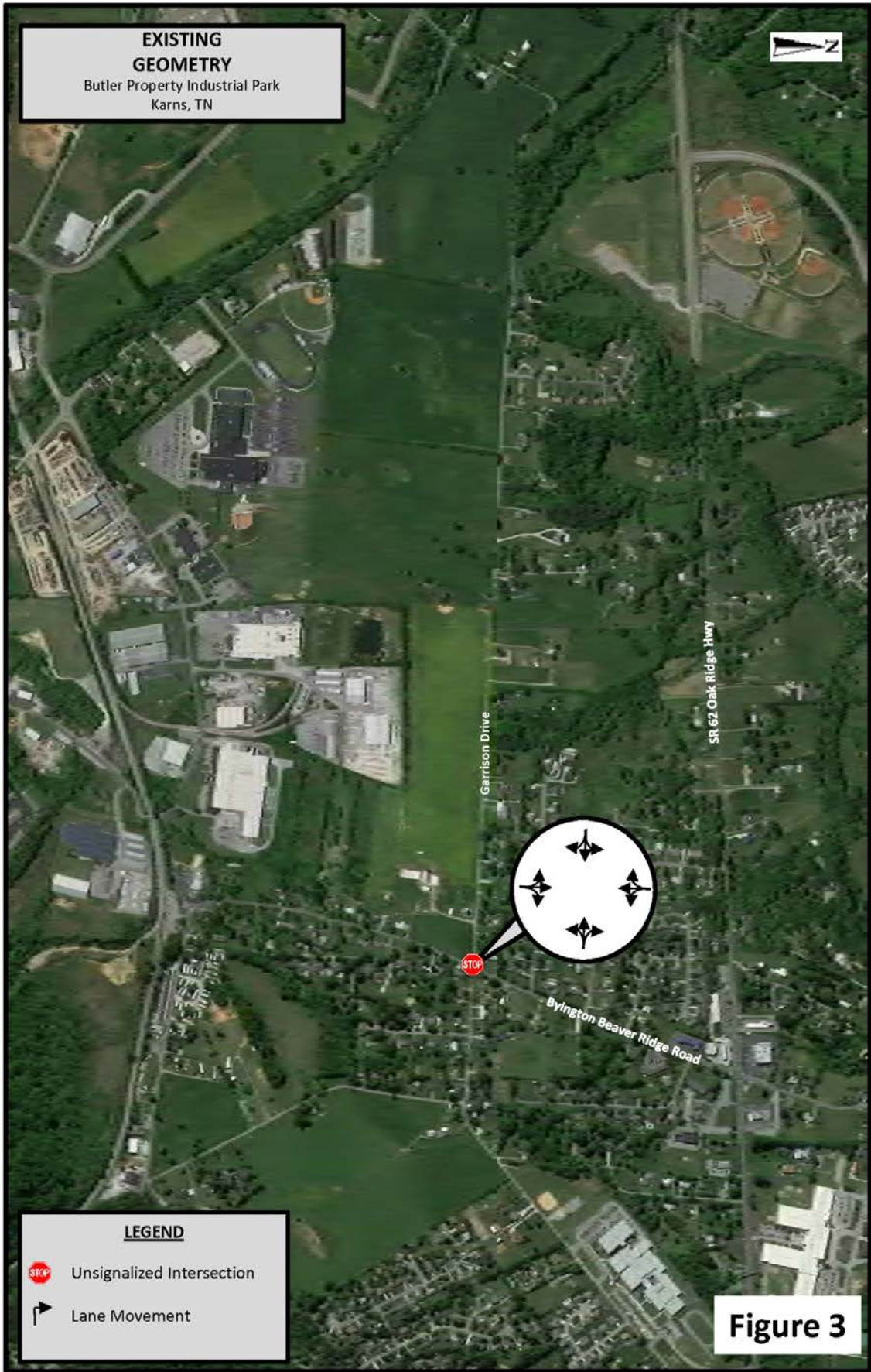
Intersection Delay Study

In addition to the turning movement count conducted for the intersection of Garrison Drive with Byington Beaver Ridge Road, a delay study was conducted for the Garrison Road approaches to Byington Beaver Ridge Road. This delay study found significant delays for the westbound approach of Garrison Road to Byington Beaver Ridge Road. This delay was found to be approximately 69 seconds during the morning peak hours and 38 seconds during the afternoon school peak hour. The delay associated with the PM peak hour was found to be 22 seconds.

Traffic Signal Warrant Evaluation

For the Garrison Drive intersection with Byington Beaver Ridge Road, an evaluation for traffic control signals was conducted for the existing 2013 traffic conditions. There are eight warrants published in the **Manual on Uniform Traffic Control Devices, 2009 Edition**. Three traffic volume warrants were examined including the Eight-Hour Vehicular Volume (Warrant 1), Four-hour (Warrant 2), and Peak-hour Volume (Warrant 3B). The Eight-Hour Vehicular Volume Warrant is further subdivided into three parts, the Minimum Volume (Warrant 1A), Interruption to Continuous Traffic Flow (Warrant 1B), and Combination (Warrant 1A & B). Any part of Warrant 1 must be met for a minimum of eight hours. Warrant 2 must be met for four hours, and one hour must be met for the Peak-Hour Warrant (Warrant 3B). The volume thresholds required to satisfy the volume warrants are reduced when prevailing speeds are in excess of 40mph.

With the prevailing speed in excess of 40mph, traffic volumes at the intersection of Garrison Drive with Byington Beaver Ridge Road satisfies the Peak-Hour (Warrant 3B) signal warrant and approaches the Four-Hour warrant with three hours satisfied.





Existing Capacity and Level of Service

In order to evaluate the current operations of the traffic control devices, capacity and level of service were calculated using the **2010 Highway Capacity Manual, Special Report 209** published by the Transportation Research Board (TRB). Signalized and unsignalized intersections are evaluated based on estimated intersection delays, which may be related to level of service (LOS).

Level of service and capacity are the measurements of an intersection's ability to accommodate traffic volumes. Levels of service for intersections range from A to F. LOS A is the best, and LOS F is failing. For signalized intersections, a LOS of A has an average estimated intersection delay of less than 10 seconds, and LOS F has an estimated delay of greater than 80 seconds. A LOS of C and D are typical design values. Within urban areas, a LOS D, delay between 35 and 55 seconds, is considered acceptable by the Institute of Transportation Engineers (ITE) for signalized intersections.

Unsignalized intersection levels of service have lower thresholds of delays. A LOS of F exceeds estimated delays of 50 seconds. For urban arterials, minor approaches may frequently experience levels of service E. A full level of service description for unsignalized and signalized intersections is presented in **Tables 1 and 2**, respectively.

Analyses of existing conditions are conducted using the Synchro, Version 8, Software, developed by Trafficware. **Table 3** presents the analyses of the access intersection. **Figure 5** illustrates the existing traffic control and geometry and LOS for the study peak hours.

**Table 1
LEVEL-OF-SERVICE (LOS) DESCRIPTION
FOR TWO-WAY STOP INTERSECTIONS**

Level of Service	Average Control Delay per Vehicle (seconds)
A	≤ 10.0
B	> 10.0 and ≤ 15.0
C	> 15.0 and ≤ 25.0
D	> 25.0 and ≤ 35.0
E	> 35.0 and ≤ 50.0
F	> 50.0

SOURCE: Highway Capacity Manual, TRB Special Report 209

**Table 2
LEVEL-OF-SERVICE (LOS) DESCRIPTION
FOR SIGNALIZED INTERSECTIONS**

LOS	Average Control Delay per Vehicle (seconds)	Description
A	≤ 10.0	Very low delay with extremely favorable progression. Most vehicles don't stop.
B	> 10.0 and ≤ 20.0	Generally good progression. Increase number of stops from that described for LOS "A" resulting in higher delays
C	> 20.0 and ≤ 35.0	Fair progression with increased delay. Number of stopping vehicles become significant; however, many still pass through the intersection without stopping. Stable flow.
D	> 35.0 and ≤ 55.0	The influence of congestion becomes more noticeable. Longer delays resulting from unfavorable progression, longer cycles, or high V/C ratios. Approaching unstable flow.
E	> 55.0 and ≤ 80.0	Limit of acceptable delay. Long delays associated with poor progression, long cycles, or high V/C ratios.
F	> 80.0	Unacceptable operation resulting from oversaturation (flow rates exceed capacity). Poor progression, long cycles, and high V/C ratios.

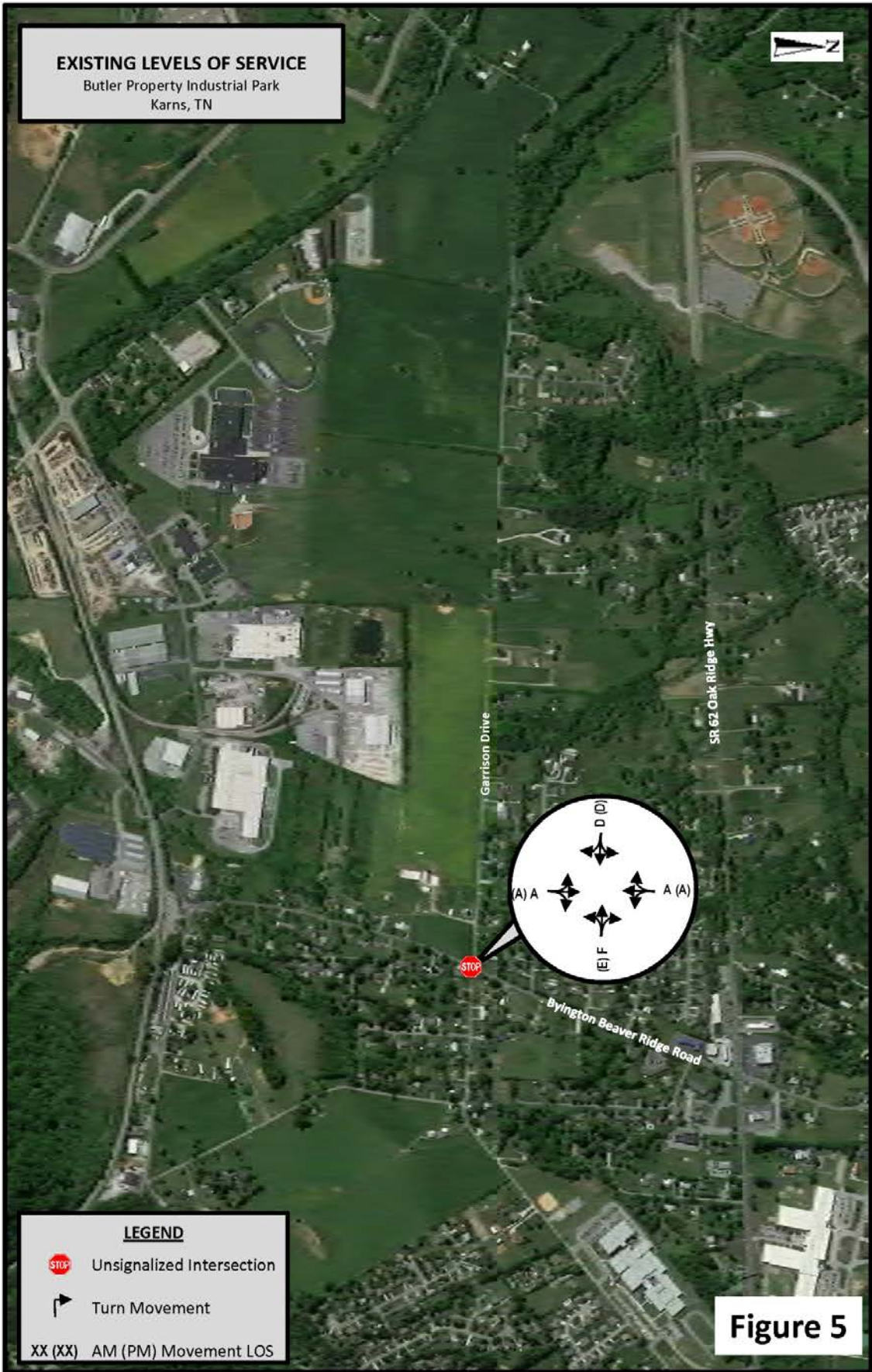
SOURCE: Highway Capacity Manual, TRB Special Report 209

**Table 3
2013 Capacity and Level of Service**

INTERSECTION	TRAFFIC CONTROL	PEAK PERIOD	V/C	DELAY	LOS
Byington Beaver Ridge Road at Garrison Drive	STOP	AM	0.11 / 1.43	33.8 / 278.9	D / F
	EB/WB	PM	0.18 / 0.43	32.6 / 37.2	D / E
<i>INSTALL SIGNAL</i>	<i>SIGNAL</i>	<i>AM</i>	<i>0.69</i>	<i>10.3</i>	<i>B</i>
	<i>MITIGATION</i>	<i>PM</i>	<i>0.49</i>	<i>5.6</i>	<i>A</i>

Note: Average vehicle delay estimated in seconds. STOP control analyses conducted in HCM 2010 TWSC methodology are presented by total minor approaches.

The existing intersection of Byington Beaver Ridge Road and Garrison Drive is operating with a westbound LOS F and E during the AM and PM peak hours, respectively. These delays were confirmed with the delay study conducted for the intersection. With signalization of the intersection, a very good LOS may be achieved.



BACKGROUND TRAFFIC CONDITIONS

Future traffic conditions or background conditions are the anticipated conditions regardless of the proposed development. Traffic through the study area should continue to grow as the region develops. West of the proposed site, plans exist for the connection of Karns Valley Drive north and south of Oak Ridge Highway, providing an improved connection from Hardin Valley Road via Westcott Boulevard north to West Emory Road, intersecting Oak Ridge Highway. This connection will provide an improve railroad crossing. **Figure 6** illustrates this connection and the planned intersection geometry.

Background Traffic Volumes

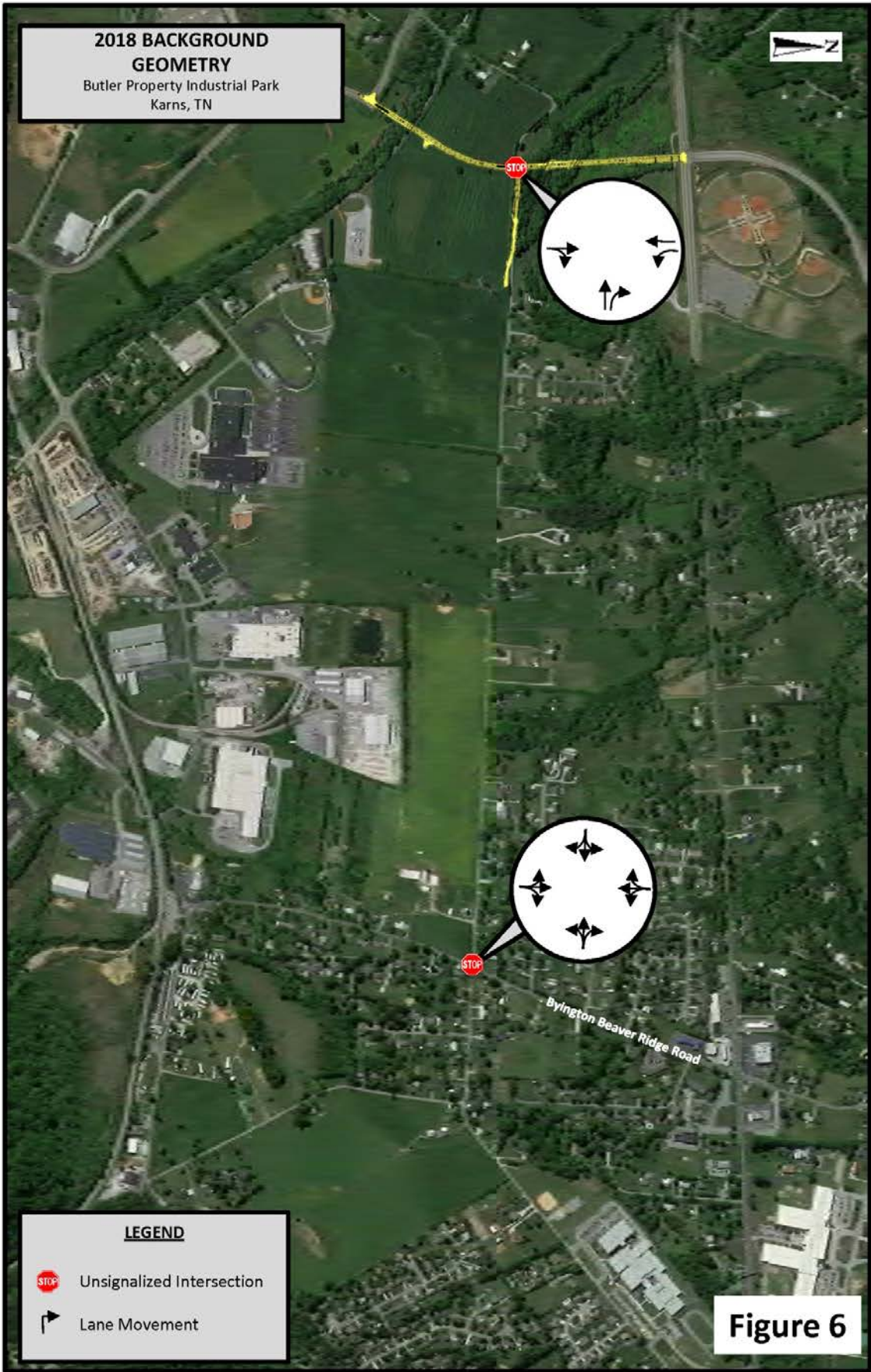
The count history available through the TDOT count station located on S.R. 131 south of the site suggests a 1.5-percent annual growth rate between 2002 and 2012. Another study in the vicinity of this site utilized a 2-percent growth rate. Therefore, for study purposes, an annual growth rate of 2-percent was assumed.

The completion of the industrial development is assumed for 2018. This actual build out of the development will depend on economic and market conditions, but should be acceptable for this study. Using the horizon years of 2018, the growth rate for the project vicinity, background traffic may be estimated for the transportation system. An annual growth rate of 2-percent results in a growth factor of 1.10 for 2018.

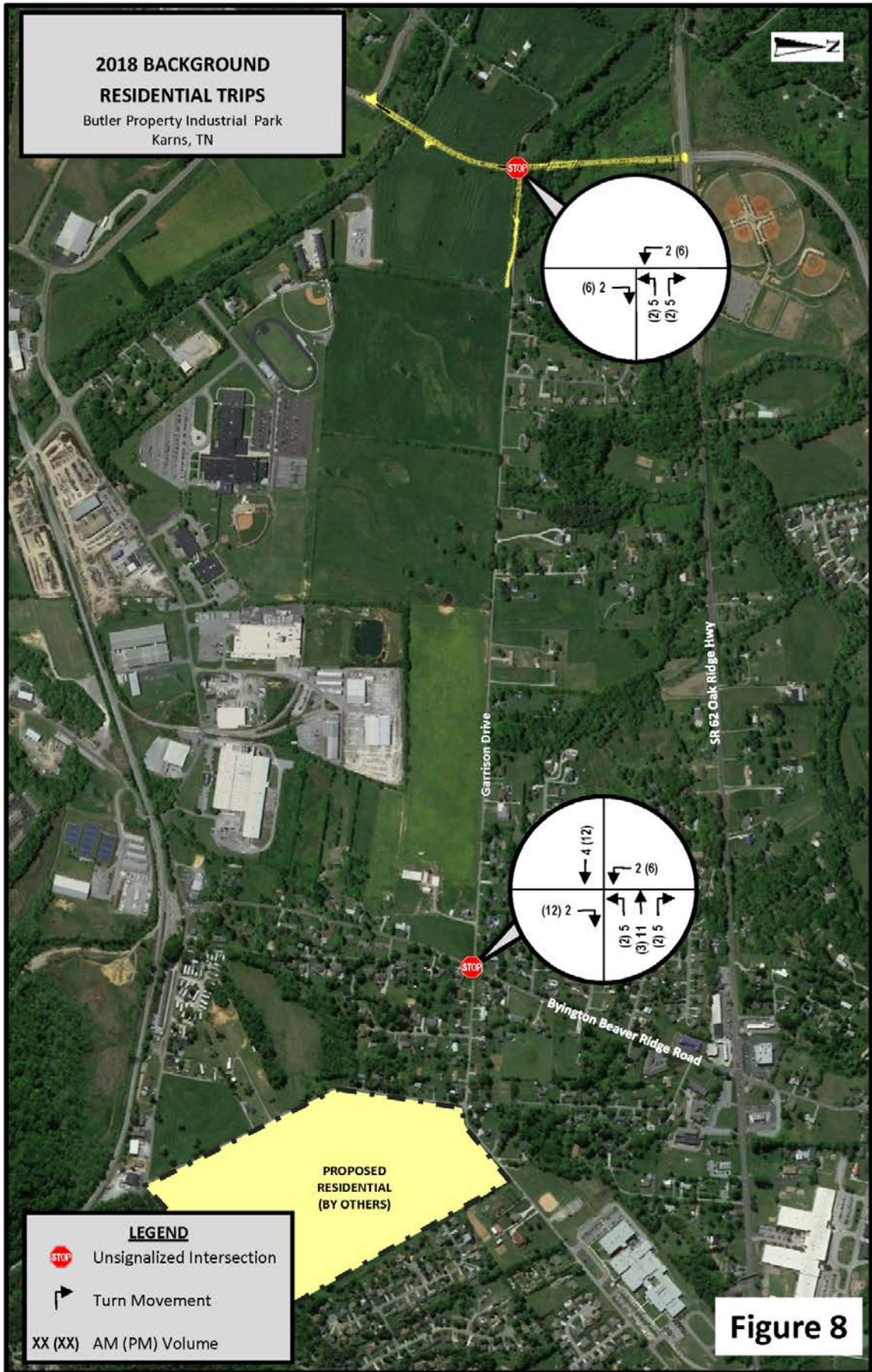
Traffic was developed for the Karns Valley Connector and Garrison Drive intersection using the MPO Travel Demand Model's 2014 traffic for the Karns Valley Connector and the intersection count conducted for Garrison Drive at Byington Beaver Ridge Road. The 2014 modeled average daily traffic for the Karns Valley Connector is 14,650 north of Garrison Drive and 14,350 to the south. Base 2018 traffic is illustrated in **Figure 7**.

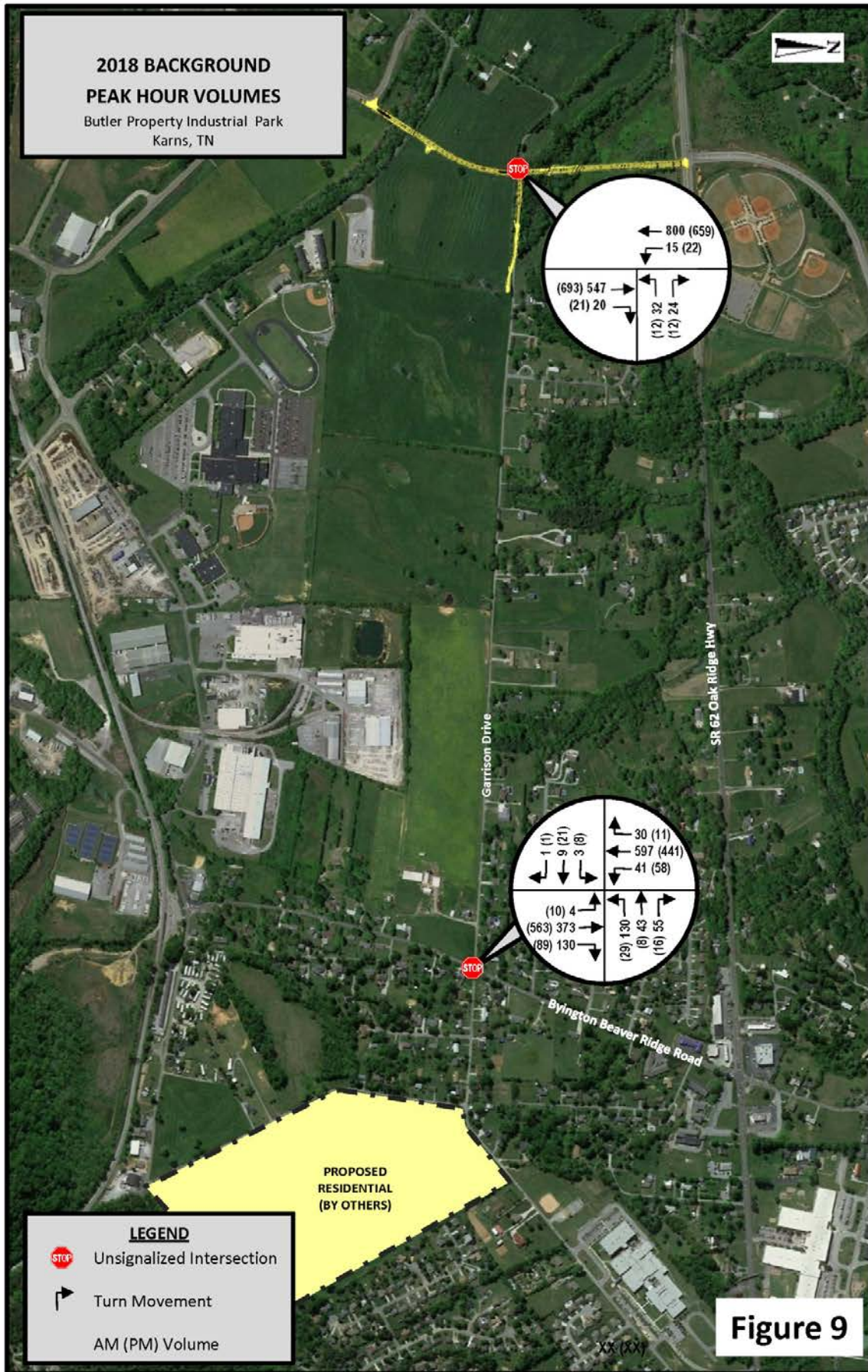
Also included in the background traffic is a planned 189 single-family unit residential subdivision, located in the southeast corner of Garrison Drive at Gray Hendrix Road. This planned development was identified by MPC for inclusion in the study. These trips were distributed to the study intersections and the site access. **Figure 8** illustrates the assignment of these trips.

Figures 9 illustrates the total 2018 background peak-hour traffic.









Background Capacity and Level of Service

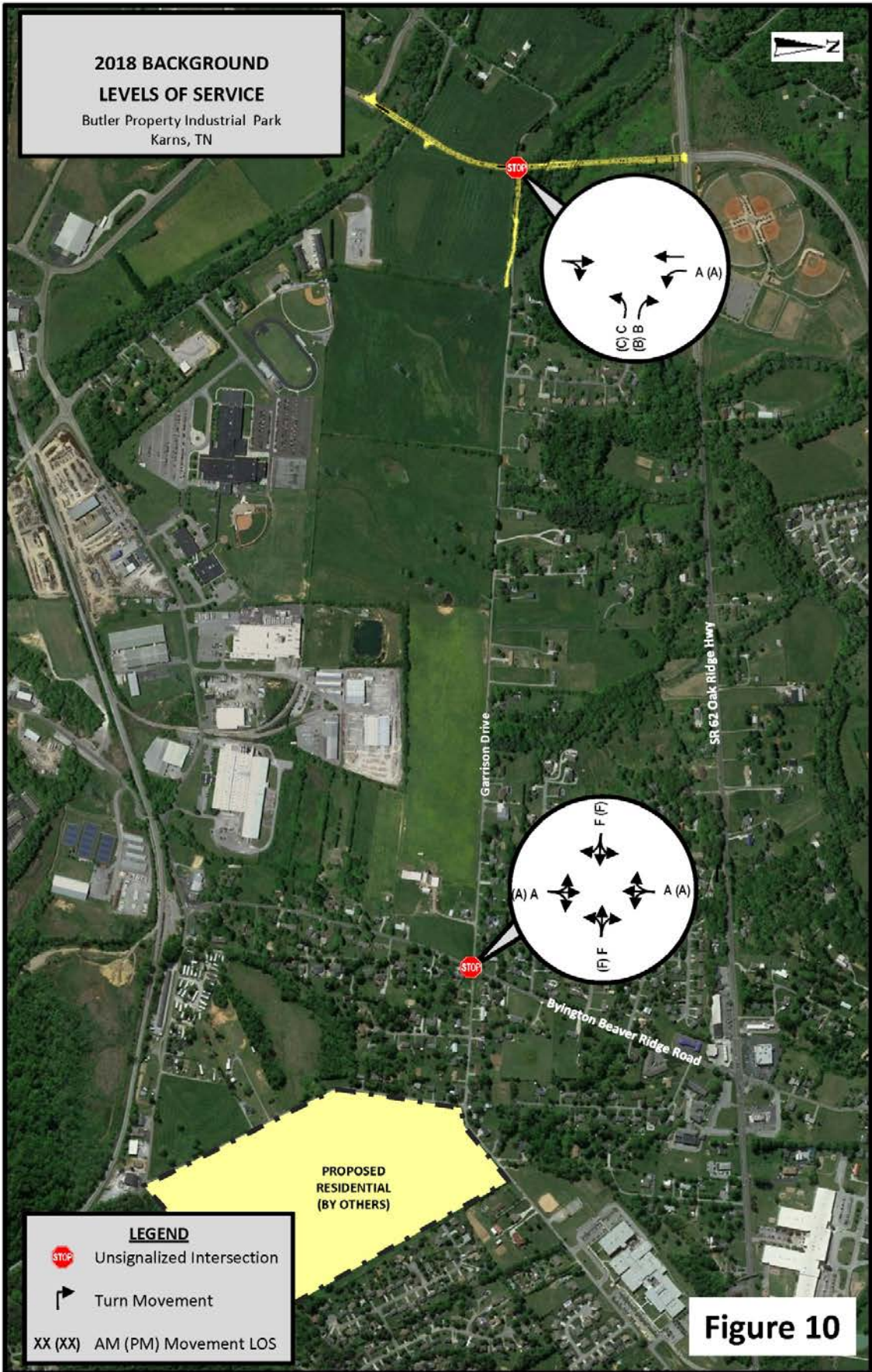
The study intersections of Garrison Drive at Byington Beaver Ridge Road and the planned Karns Valley Connector were analyzed and identified to operate at acceptable levels of service for the years of 2018. Analyses conducted for future projections use many defaults of the HCM due to the unknown conditions. **Table 4** presents the intersection 2018 background LOS summary. **Figure 10** illustrates the roadway geometrics, traffic control, and LOS by lane group for 2018.

Table 4
2018 Background
Capacity and Levels of Service

INTERSECTION	TRAFFIC CONTROL	PEAK PERIOD	V/C	DELAY	LOS
Karns Valley Connector at Garrison Drive	STOP	AM	0.05 / 0.03	19.3 / 14.2	C / B
	WBL/WBR	PM	0.13 / 0.05	20.6 / 12.6	C / B
Byington Beaver Ridge Road at Garrison Drive	STOP	AM	0.41 / 0.77	55.5 / 88.6	F / F
	EB/WB	PM	0.22 / 2.23	51.0 / >500.0	F / F
<i>INSTALL SIGNAL</i>	<i>SIGNAL</i>	<i>AM</i>	<i>0.76</i>	<i>14.8</i>	<i>B</i>
	<i>MITIGATION</i>	<i>PM</i>	<i>0.57</i>	<i>6.1</i>	<i>A</i>

Note: Average vehicle delay estimated in seconds. STOP control analyses conducted in HCM 2010 TWSC methodology are presented by total minor approaches.

The Karns Connector at Garrison Drive intersection should operate at a very good level of service. At the intersection of Byington Beaver Ridge Road and Garrison Drive, the eastbound and westbound approaches will operate at a LOS F. Signalization of Garrison Drive at Byington Beaver Ridge Road provides for a very good LOS.



PROJECT IMPACTS

Trip Generation

Project traffic was determined using the publication, **Trip Generation, 8th Edition**. This reference is published by the Institute of Transportation Engineers (ITE) and represents national data collected for many different land uses including industrial, residential, and commercial uses. **Trip Generation** is an essential tool in calculating the traffic, which may be generated by a proposed development.

The light industrial land use was assumed for this development, for it was found more conservative than other possible uses including an industrial park use. Trips generated for 855,000 square feet of light industrial space were found to be 3,290 daily trips, 680 AM peak hour trips, and 650 PM peak hour trips. The manufacturing land use (LUC 140) seems most appropriate with the employment goal of approximately 580 established for the site and is the use that best identifies the use described by the Development Corporation. **Table 5** presents the trip generation for the proposed site.

**Table 5
TRIP GENERATION**

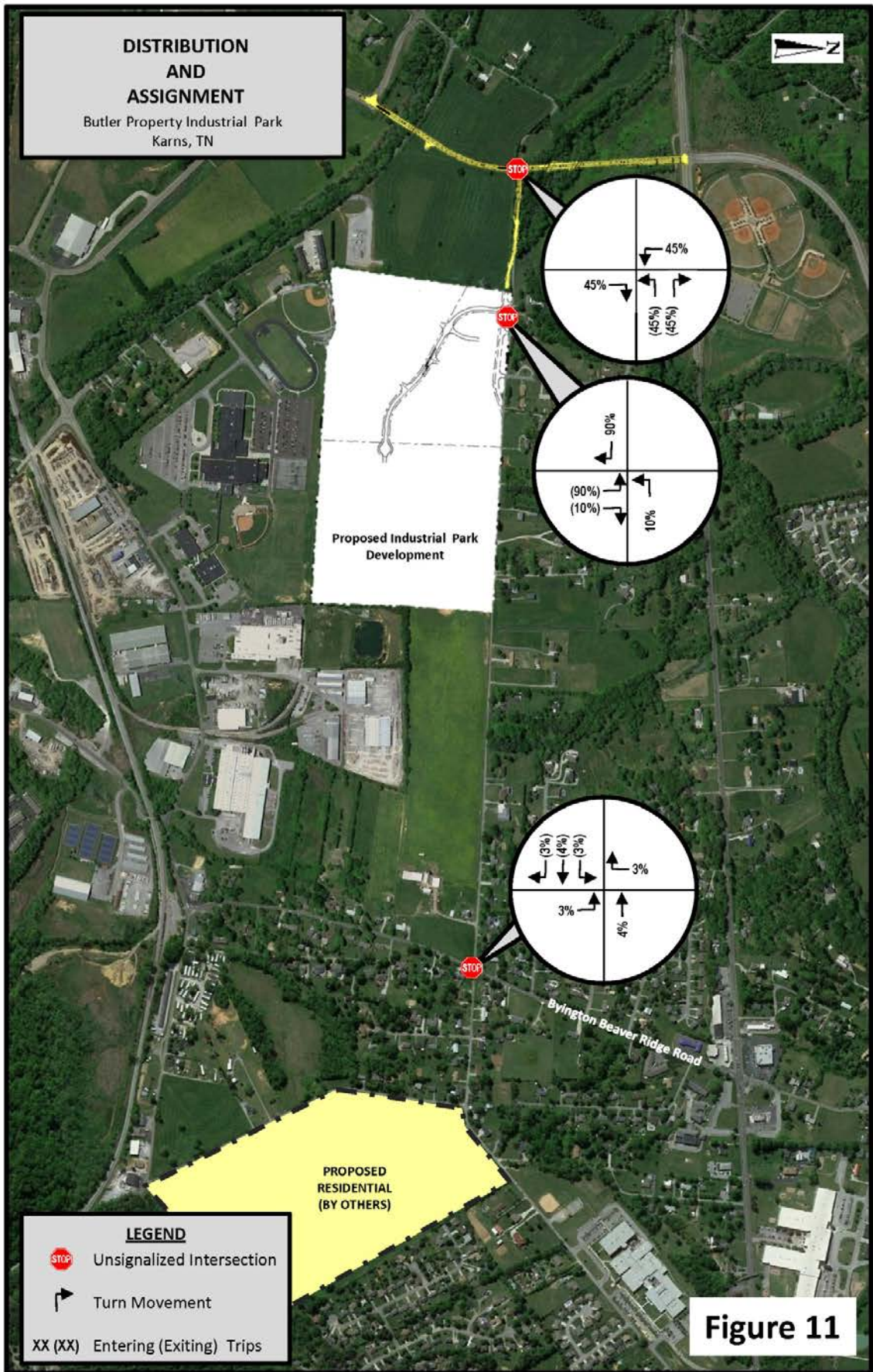
Land Use	Land-Use Code	Density	Daily Trips	AM Peak-Hour Trips		PM Peak-Hour Trips	
				Enter	Exit	Enter	Exit
Manufacturing	140	855,000 sqft	3,297	524	156	234	417

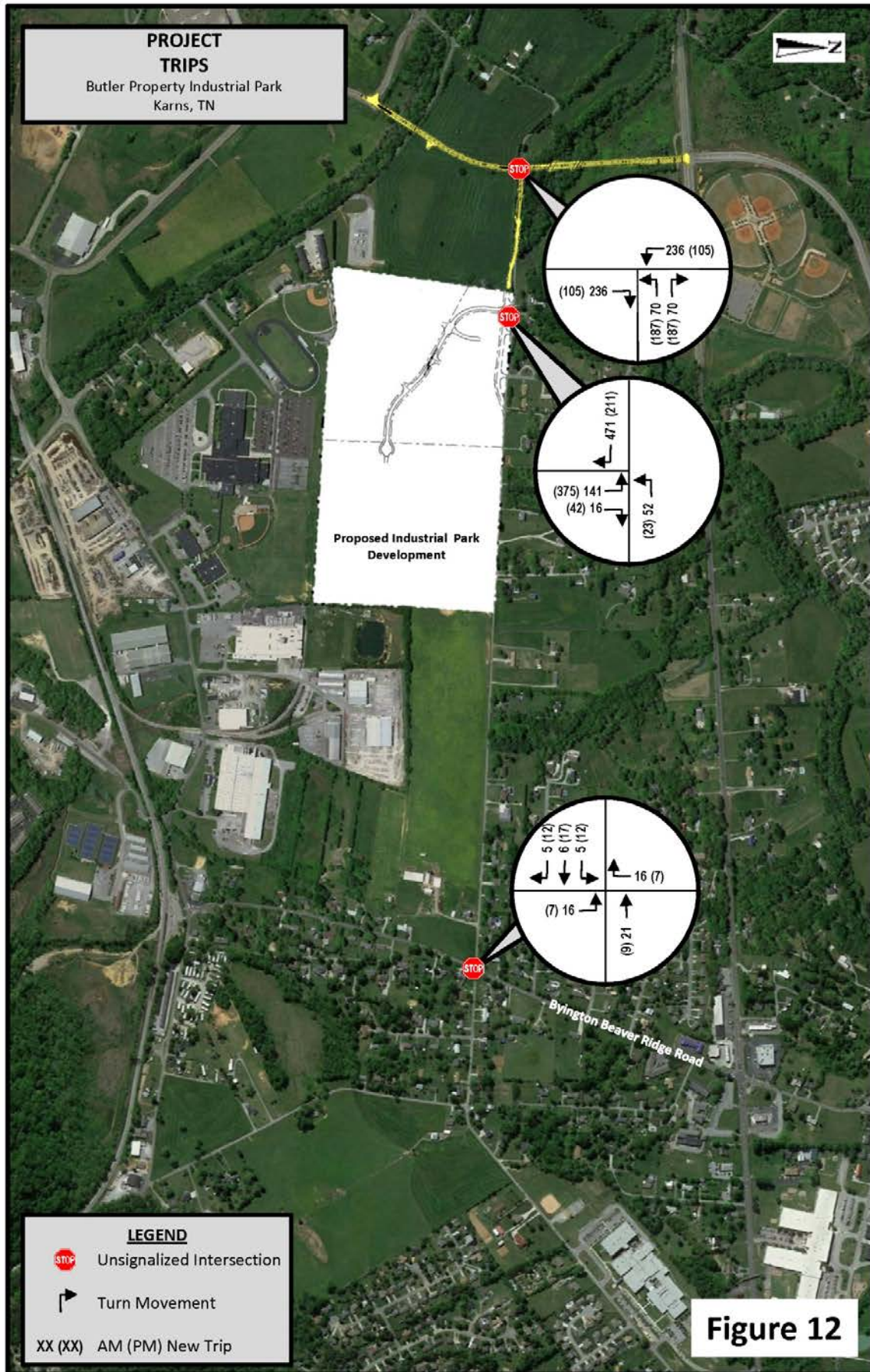
Reference: **Trip Generation, 8th Edition**

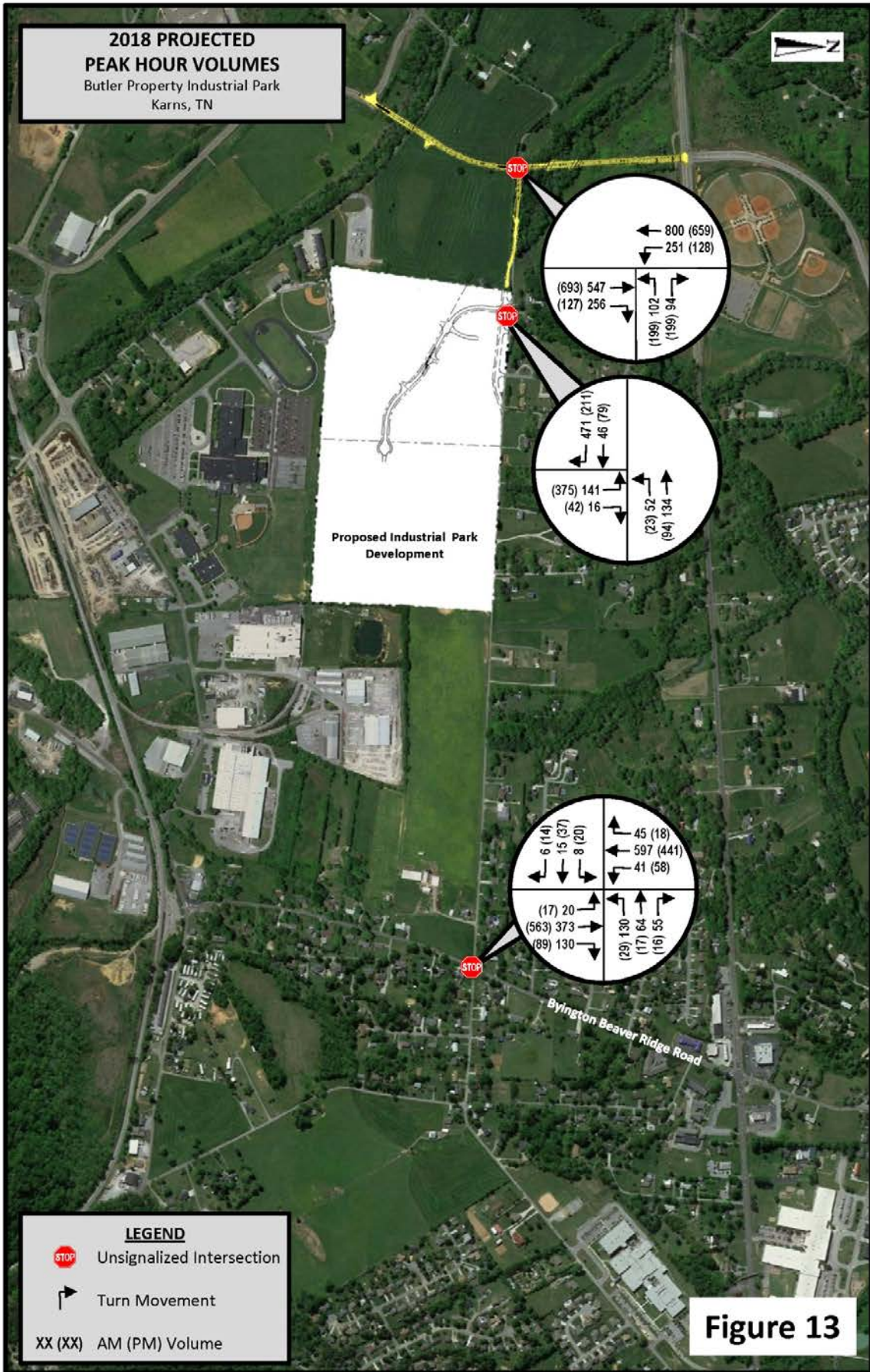
A low density residential use of this property as identified with the MPC land use plan would permit approximately 280 single-family units. The trip generation for a 280sfu residential development is approximately 2,680 daily and 265 peak-hour trips using **Trip Generation, 8th Edition** Land Use Code 210.

Trip Distribution And Assignment

The distribution and assignment was assumed based on the existing traffic patterns on the major street network. Traffic access to the site assumes a distribution of 90-percent to the west and the planned Karns Valley Connector and a conservative 10-percent to and from the east using Garrison Drive. **Figure 11** illustrates the trip distribution for the site. **Figure 12** illustrates the proposed project trips.







Projected Traffic Volumes

The addition of project trips to background traffic produces the total 2018 projected traffic volumes. **Figure 13** illustrates the projected traffic for study horizon year. The projected intersection volumes are used to analyze the intersection's future capacity and LOS. From the analyses and the projected traffic, measures are developed to facilitate the necessary access to and from the site. With a 2018 projected 1,111 daily traffic for Garrison Drive, the proposed development reflects approximately 30-percent of the traffic, representing a maximum impact given good access to Karns Valley Connector.

Traffic Signal Warrant Evaluation

For the Garrison Drive intersection with Karns Valley Connector, an evaluation for traffic control signals was conducted for the 2018 traffic conditions. Analyses were conducted for speeds below and excess of 40mph. With a posted speed limit of 40mph or prevailing speed not exceeding 40mph for the planned Karns Valley Connector, signal warrants were evaluated with the projected 2018 traffic. The intersection was found to nearly warrant signalization with the Interruption to Continuous Traffic Flow (Warrant 1B).

Assuming a prevailing speed exceeding 40mph, the signal warrant analysis found that the projected traffic for the intersection of Garrison Drive and the Karns Valley Connector would fully satisfy the Interruption (Warrant 1B), Four-Hour (Warrant 2), and the Peak-Hour (Warrant 3B) warrants.

The following table summarizes the warrant analysis. Warrants should be met at 50-percent of the projected build out of the site.

Karns Valley Connector & Garrison Drive

		<u>40mph</u>	<u>≥40mph</u>
Warrant 1A	Minimum Volume	2 (0) hours	4 (0) hours
Warrant 1B	Interruption to Continuous Traffic Flow	6 (3) hours	9 (1) hours
Warrant 1C	Combination of Parts A & B	2 hours	5 hours
Warrant 2	Four Hour	2 (0) hours	9 (0) hours
Warrant 3B	Peak-hour Volume	4 (0) hours	4 (0) hours

(X) denotes hours which the volume threshold is more than 90% satisfies

Projected Capacity and Level of Service

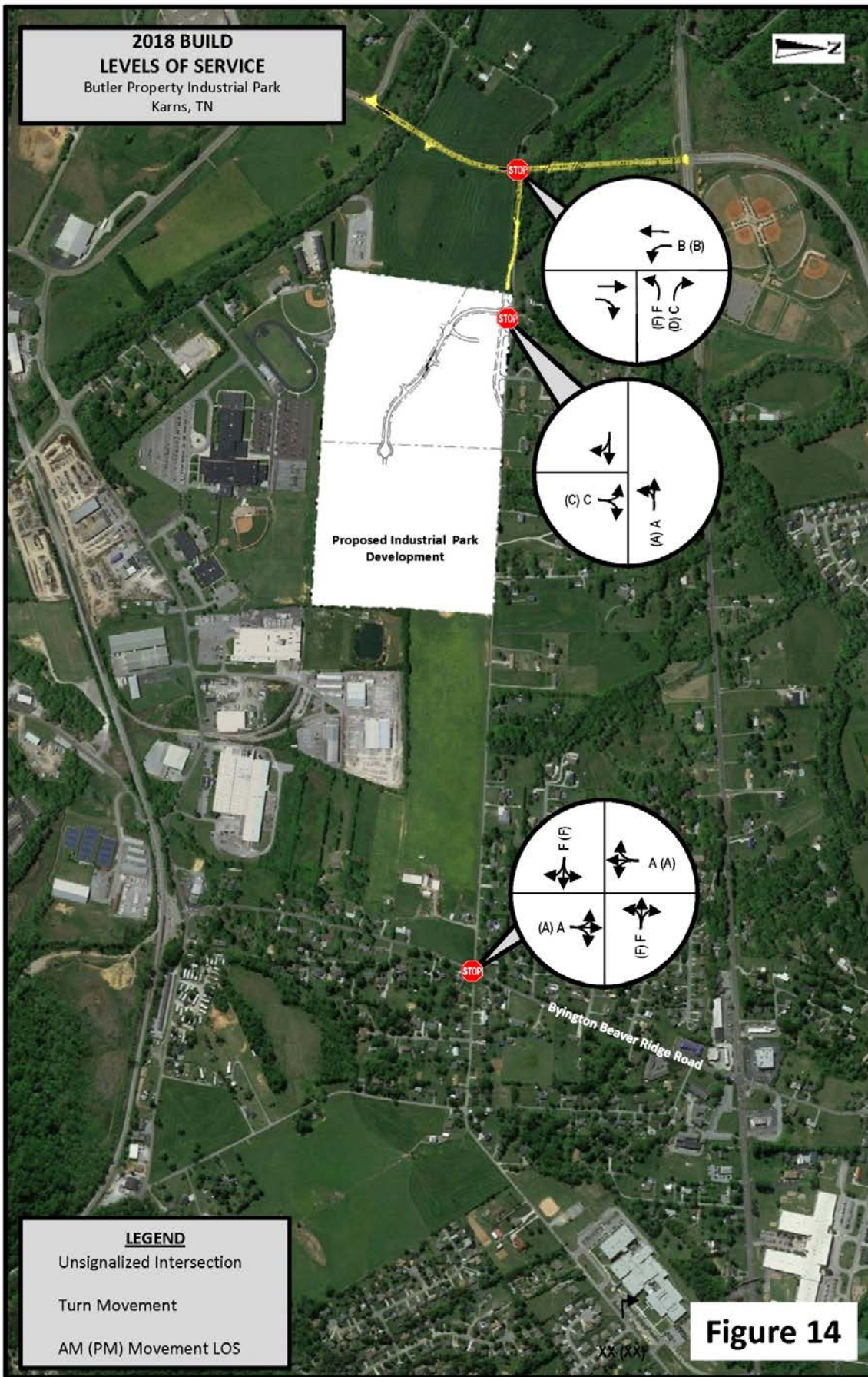
Traffic is analyzed with the proposed development to again determine the capacity and LOS for the site access. **Table 6** presents the results of these analyses. **Table 7** presents the summary of all the capacity and LOS analyses. **Figures 14 and 15** illustrate the intersection geometrics, traffic control, and the levels of service for the intersection lane groups for the 2018.

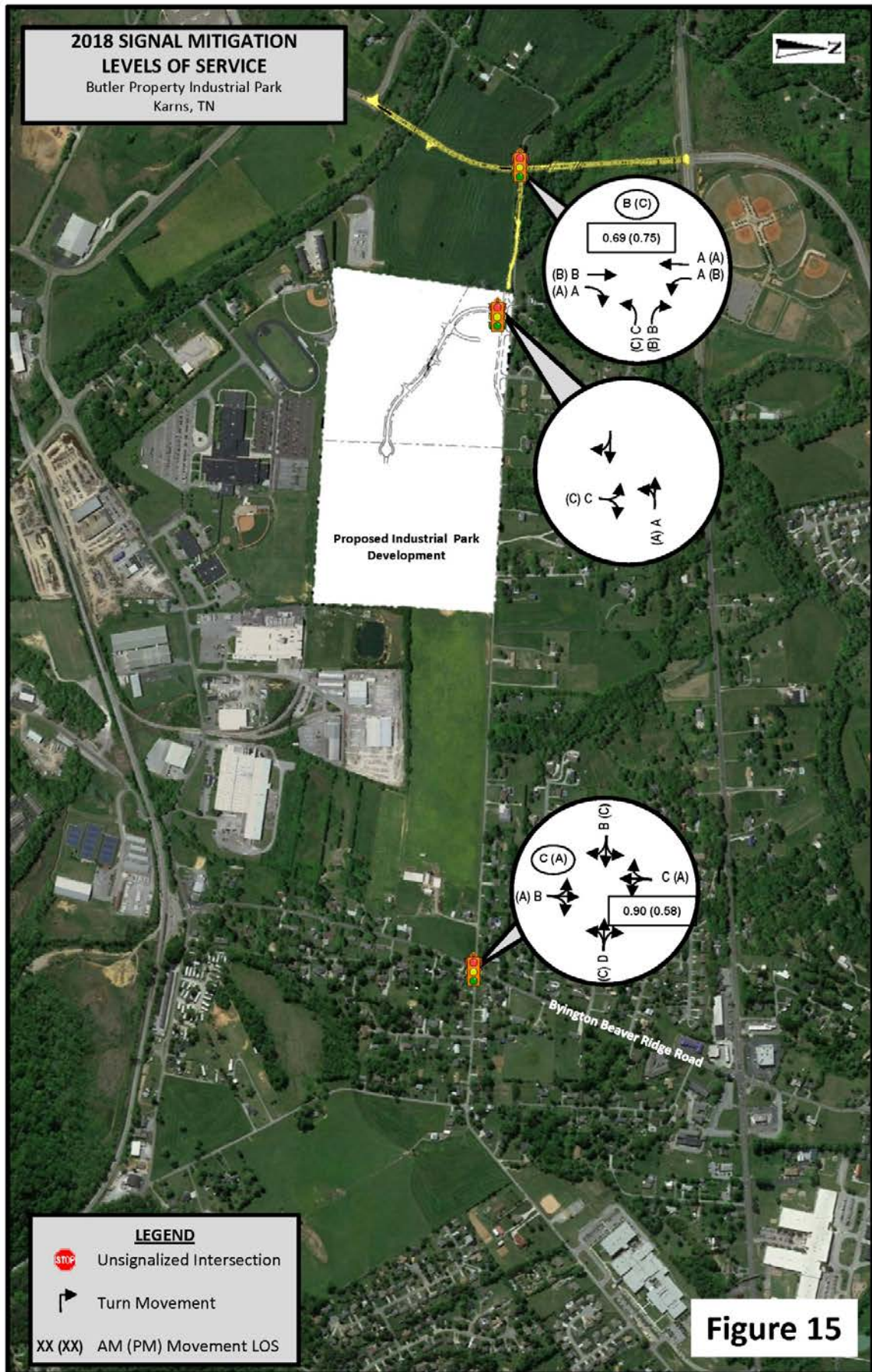
**Table 6
2018 Projected
Capacity and Levels of Service**

INTERSECTION	TRAFFIC CONTROL	PEAK PERIOD	V/C	DELAY	LOS
Karns Valley Connector at Garrison Drive	STOP	AM	0.99 / 0.24	154.6 / 16.3	F / C
	WBL/WBR	PM	1.14 / 0.58	158.4 / 27.0	F / D
<i>INSTALL SIGNAL</i>	<i>SIGNAL</i>	<i>AM</i>	<i>0.69</i>	<i>10.2</i>	<i>B</i>
	<i>MITIGATION</i>	<i>PM</i>	<i>0.75</i>	<i>13.8</i>	<i>B</i>
Byington Beaver Ridge Road at Garrison Drive	STOP	AM	0.00 / 3.80	0.0 / >500.0	- / F
	EB/WB	PM	1.06 / 1.29	158.6 / 269.3	F / F
<i>INSTALL SIGNAL</i>	<i>SIGNAL</i>	<i>AM</i>	<i>0.90</i>	<i>27.6</i>	<i>C</i>
	<i>MITIGATION</i>	<i>PM</i>	<i>0.58</i>	<i>8.8</i>	<i>A</i>
Site Access at Garrison Drive	STOP	AM	0.36	16.8	C
	NB	PM	0.70	22.5	C
<i>REALIGN GARRISON DR.</i>	<i>STOP</i>	<i>AM</i>	<i>0.33</i>	<i>13.6</i>	<i>B</i>
	<i>MITIGATION</i>	<i>PM</i>	<i>0.24</i>	<i>14.1</i>	<i>B</i>

Note: Average vehicle delay estimated in seconds. STOP control analyses conducted in HCM 2010 TWSC methodology are presented by total minor approaches.

With the manufacturing development build-out of the Butler property, the site access to Garrison Drive will operate at a LOS C with a STOP control intersection traffic control. The intersection of Karns Valley Connector and Garrison Drive, at buildout of the site will fail as an unsignalized intersection, STOP controlled for the Garrison Drive approach. Karns Valley Connector and Garrison Drive, and Byington Beaver Ridge Road and Garrison Drive, these intersections can operate at a minimum LOS C during the peak hours.





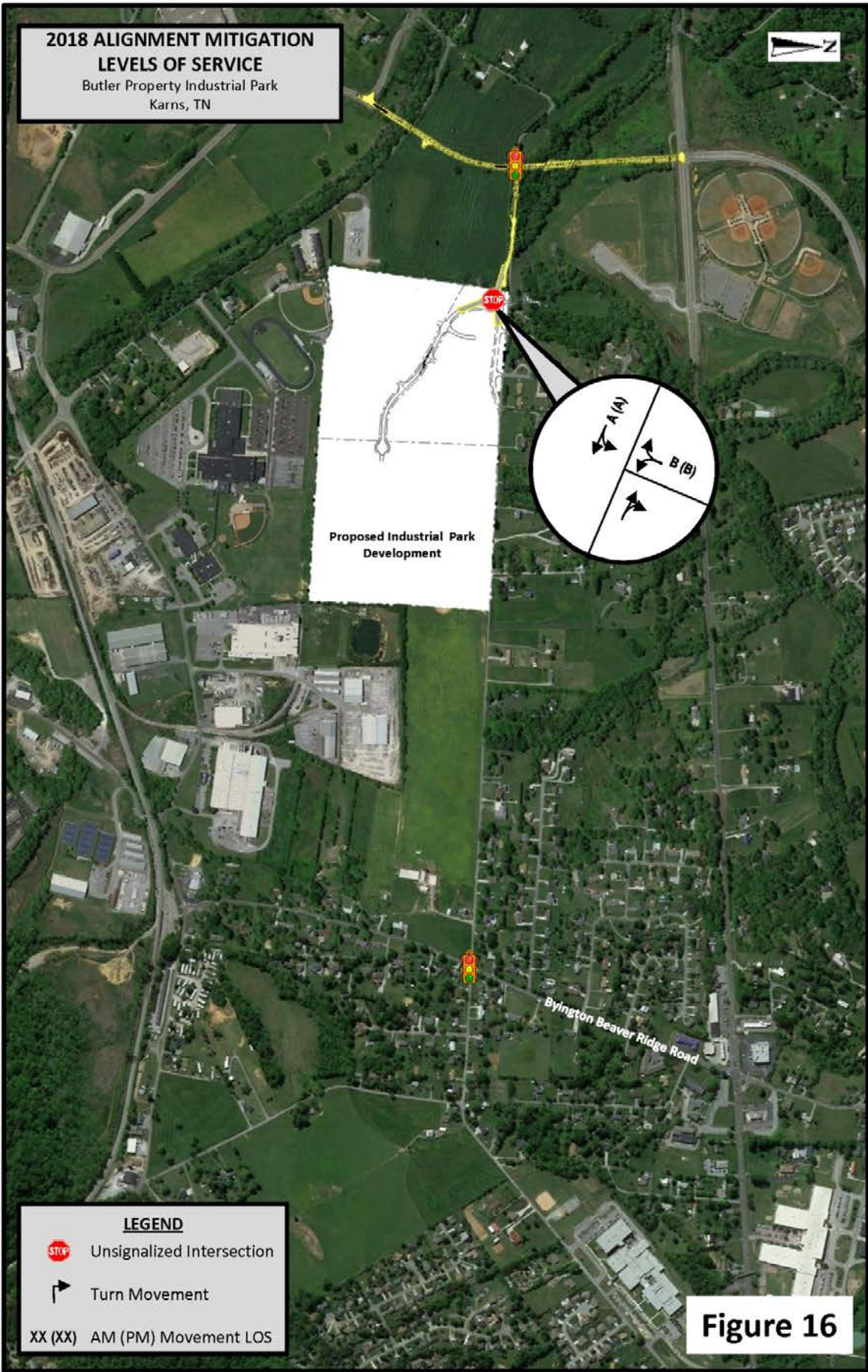
**Table 7
SUMMARY OF
CAPACITY AND LEVEL OF SERVICE**

INTERSECTION	TRAFFIC CONTROL	PEAK PERIOD	2013 TRAFFIC			2018 NO BUILD			2018 BUILD		
			V/C	DELAY	LOS	V/C	DELAY	LOS	V/C	DELAY	LOS
			Karns Valley Connector at Garrison Drive	STOP WBL/WBR	AM PM				0.05 / 0.03 0.13 / 0.05	19.3 / 14.2 20.6 / 12.6	C / B C / B
INSTALL SIGNAL	SIGNAL MITIGATION	AM PM				0.56 0.50	4.5 2.8	A A	0.69 0.75	10.2 13.8	B B
Byington Beaver Ridge Road at Garrison Drive	STOP EB/WB	AM PM	0.11 / 1.43 0.18 / 0.43	33.8 / 278.9 32.6 / 37.2	D / F D / E	0.41 / 0.77 0.22 / 2.23	55.5 / 88.6 51.0 / >500.0	F / F F / F	0.00 / 3.80 1.06 / 1.29	0.0 / >500.0 158.6 / 269.3	- / F F / F
INSTALL SIGNAL	SIGNAL MITIGATION	AM PM	0.69 0.49	10.3 5.6	B A	0.76 0.57	14.8 6.1	B A	0.90 0.58	27.6 8.8	C A
Site Access at Garrison Drive	STOP NB	AM PM							0.36 0.70	16.8 22.5	C C
REALIGN GARRISON DR	STOP MITIGATION	AM PM							0.33 0.24	13.6 14.1	B B

Note: Average vehicle delay estimated in seconds. STOP control analyses conducted in HCM 2010 TWSC methodology are presented by total minor approaches.

With the primary movement between Garrison Drive and the industrial access road, the intersection could be realigned with the through street being between Karns Valley Connector and the industrial access, this the Garrison Drive approach becomes the minor approach. This alignment would facilitate the primary traffic movement. The facilitation of the industrial traffic movement would be more efficient and should deter traffic from Garrison Drive. **Figure 16** illustrates the level of service with this intersection realignment.

The westbound approach to Karns Valley Connector may experience some queuing during the PM peak hour. To more effectively manage the queues for the westbound approach, separate left- and right-turn lanes should be provided of sufficient storage to efficiently distribute the turning traffic and permit right-turning access to the intersection.

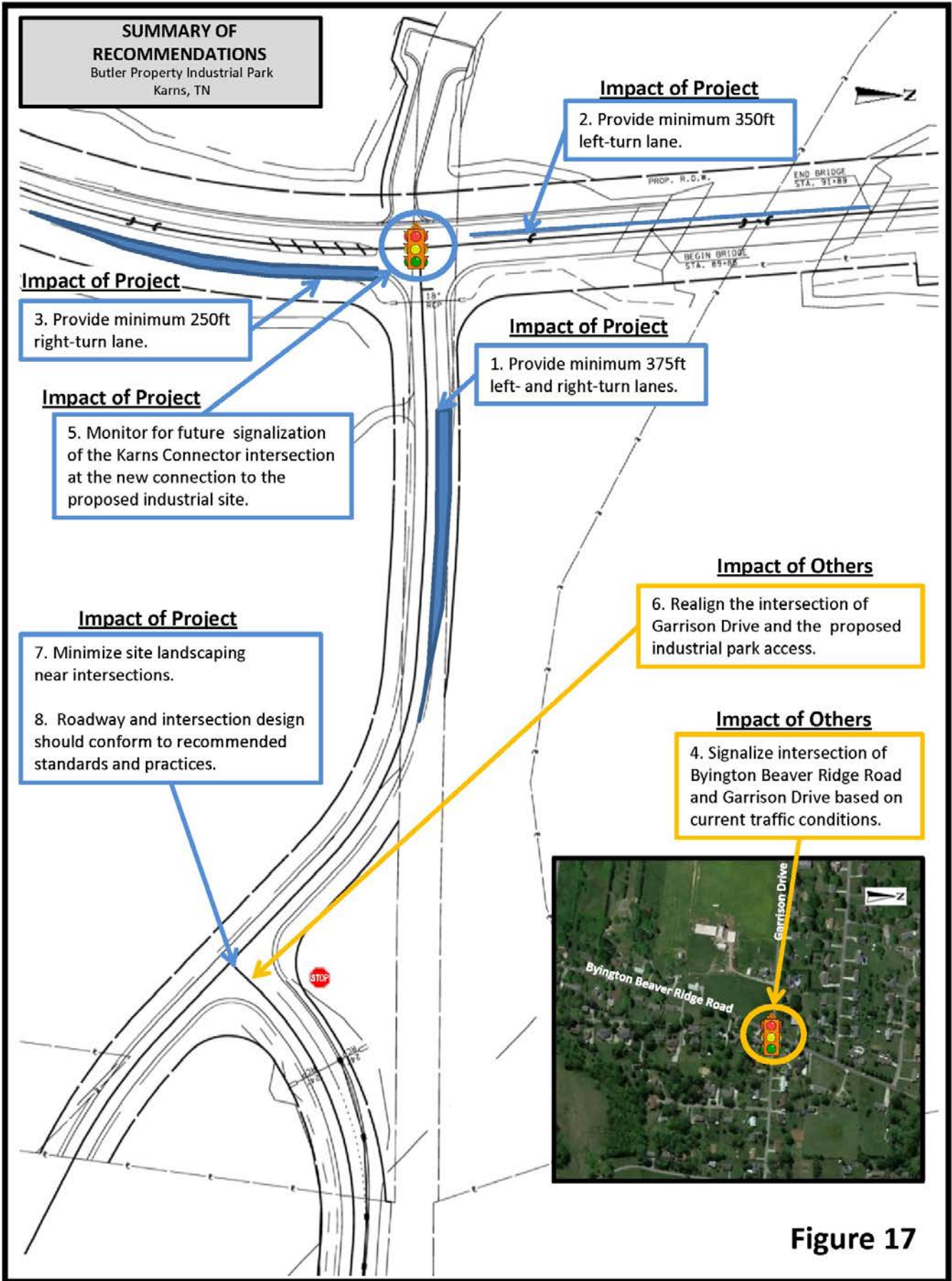


RECOMMENDED IMPROVEMENTS

The projected traffic volumes, analyses conducted, and Knox County policies identified the following improvements necessary for the maintenance of an acceptable LOS during the peak hours:

1. Provide minimum 375-foot left- and right-turn lanes for the westbound approach to the Karns Valley Connector.
2. Provide a minimum 350-foot left-turn lane for the southbound movement from Karns Valley Connector to Garrison Drive.
3. Provide a minimum 250-foot right-turn lane for the northbound movement from Karns Valley Connector to Garrison Drive.
4. Signalize the intersection of Byington Beaver Ridge Road and Garrison Drive for the mitigation of existing conditions.
5. Monitor the intersection of Karns Valley Connector and Garrison Drive for signalization. Install a traffic signal when warrant can be satisfied and provide for right-turn overlaps to and from Garrison Drive.
6. Realign the intersection of Garrison Drive and the industrial access assigning the right of way to the industrial development.
7. Minimize landscaping, using low growing vegetation and signing at the planned accesses to insure that safe sight-distance is maintained.
8. Roadway and intersection design should conform to the recommended standards and practices adopted by the Institute of Transportation Engineers (ITE), American Association of State and Highway Officials (AASHTO), and Knox County.

A summary of recommendations are provided in **Figure 17**. The blue color represents improvements identified due to the proposed project impacts and the orange color represents improvements identified due to impacts of others.



CONCLUSION

The proposed development is an industrial park development, located in Karns, northwest Knox County, Tennessee. The study included traffic counts and projected traffic with and without the proposed development. Background traffic, traffic that may be anticipated regardless of the proposed development, was determined using a 2-percent annual growth rate until the assumed build-out year of 2018.

The manufacturing land use was assumed for this development, for it more reflected the goal employment of 580 and the uses identified by the Development Corporation. Trips generated for 855,000 square feet of light industrial space were found to be 3,300 daily trips, 680 AM peak hour trips, and 650 PM peak hour trips. A conservative 10-percent of the trips were also assigned to Garrison Drive.

Using the identified turning movements for the existing and projected traffic conditions, with and without the proposed development, capacity and level of service analyses were conducted using the **2010 Highway Capacity Manual**. The site access and study intersections were found to operate at acceptable levels of service with the recommended improvements identified in this study.

The intersection of Byington Beaver Ridge Road and Garrison Drive currently meets the Peak Hour signal warrant mitigating a failing level of service. Signalization of the Byington Beaver Ridge Road and Garrison Drive intersection should provide a very good level of service. The industrial site, at build-out, should warrant signalization of the Karns Valley Connector and Garrison Road intersection. With the realignment of the industrial access road and Garrison Drive intersection providing for the primary traffic movement between the Karns Valley Connector and the industrial access would facilitate the primary traffic movement. Teeing Garrison Drive into the industrial access with a STOP control provides for an acceptable level of service and should deter traffic on Garrison Drive. Sufficient storage should be provided for the westbound approach to Karns Valley Connector to minimize delay and queues.

APPENDIX

Trip Generation

Traffic Signal Warrant Evaluations

Synchro Analysis Reports

Traffic Count & Delay Data

Land Use: 140

Manufacturing

Description

Manufacturing facilities are areas where the primary activity is the conversion of raw materials or parts into finished products. Size and type of activity may vary substantially from one facility to another. In addition to the actual production of goods, manufacturing facilities generally also have office, warehouse, research and associated functions. General light industrial (Land Use 110), general heavy industrial (Land Use 120) and industrial park (Land Use 130) are related uses.

Additional Data

Average weekday transit trip ends:

- 0.09 per employee
- 0.08 per 1,000 square feet gross floor area
- 1.25 per acre

Vehicle occupancy ranged from 1.2 to 1.3 persons per automobile on an average weekday.

The peak hour of the generator typically coincided with the peak hour of the adjacent street traffic.

Facilities with employees on shift work may peak at other hours.

The sites were surveyed in the late 1960s, the early 1970s, the mid-1980s, the 1990s and the 2000s throughout the United States.

Source Numbers

3, 7, 10, 15, 17, 74, 85, 88, 177, 184, 241, 357, 384, 418, 443, 583, 598, 611

Manufacturing (140)

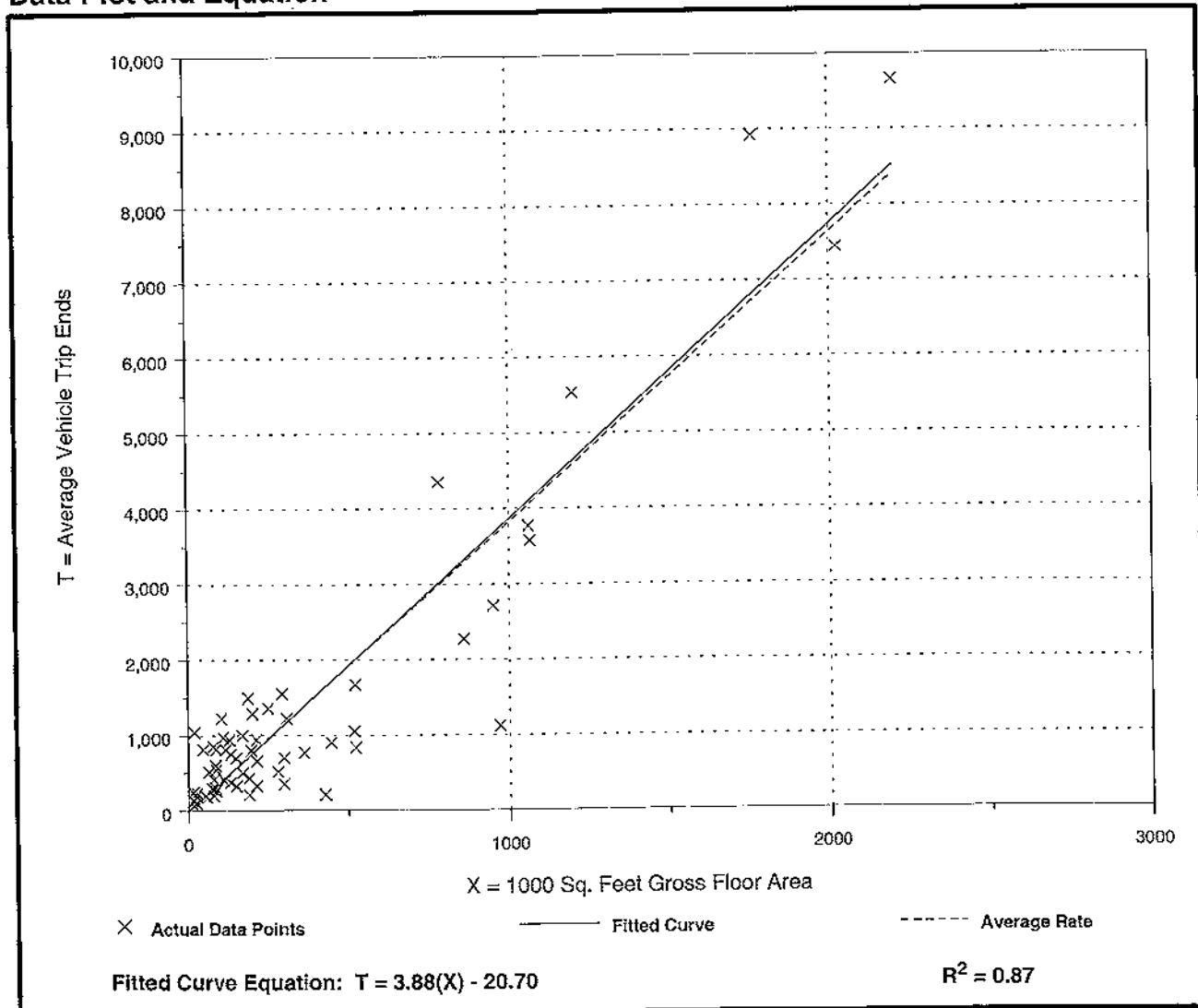
Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area
On a: Weekday

Number of Studies: 62
Average 1000 Sq. Feet GFA: 349
Directional Distribution: 50% entering, 50% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
3.82	0.50 - 52.05	3.07

Data Plot and Equation



Manufacturing (140)

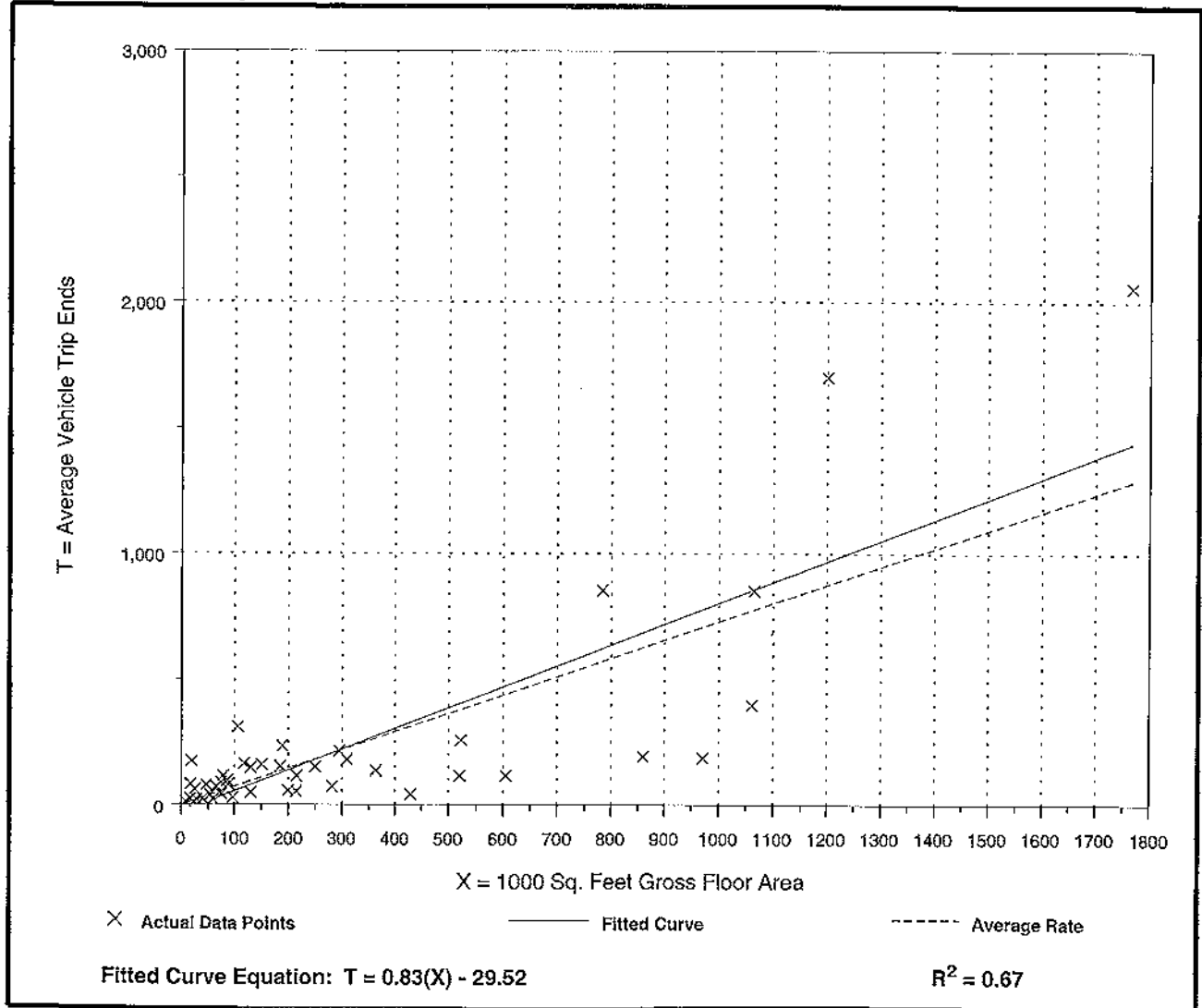
Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.

Number of Studies: 51
 Average 1000 Sq. Feet GFA: 293
 Directional Distribution: 78% entering, 22% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
0.73	0.10 - 8.75	1.04

Data Plot and Equation



Manufacturing (140)

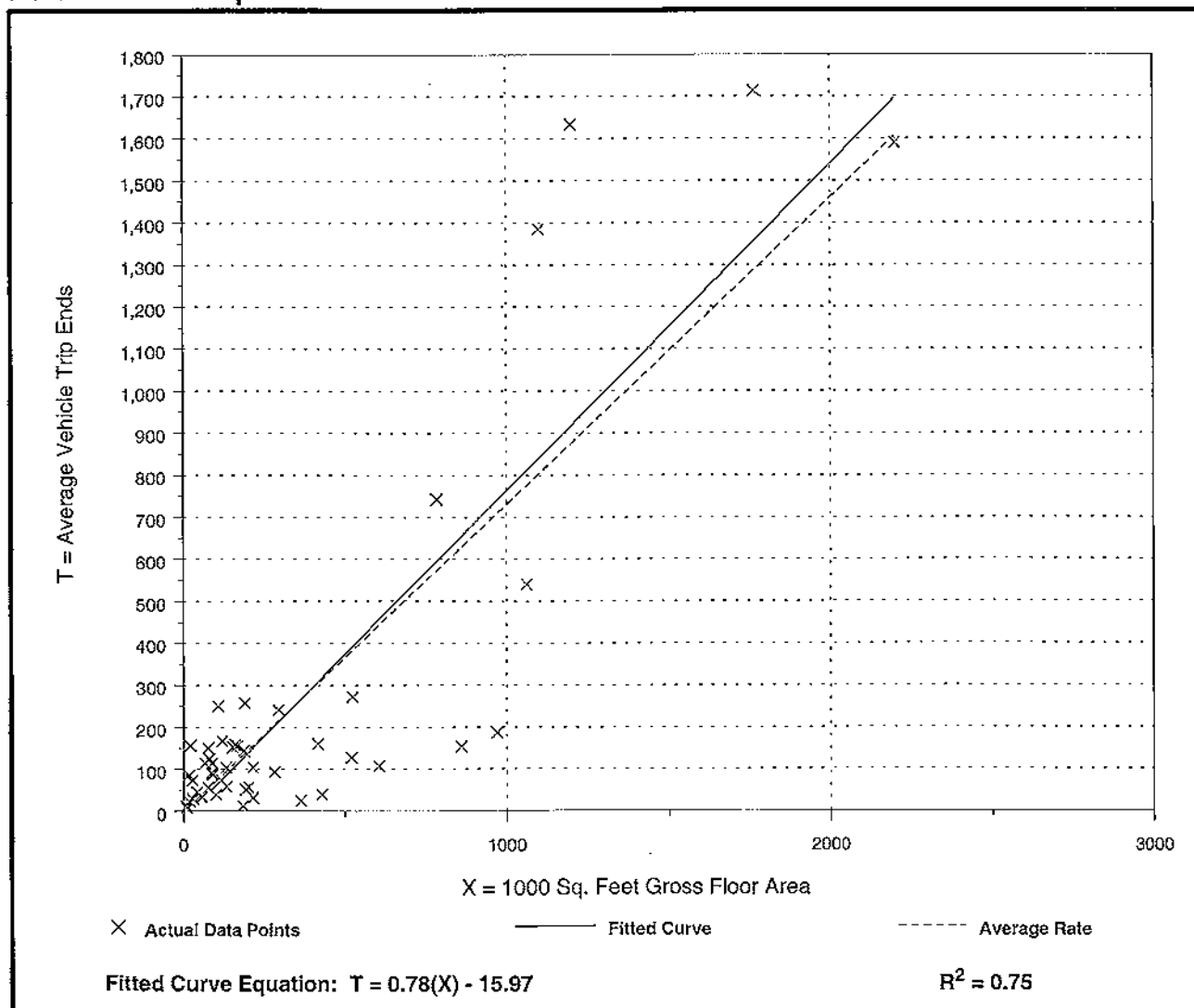
Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.

Number of Studies: 56
 Average 1000 Sq. Feet GFA: 318
 Directional Distribution: 36% entering, 64% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
0.73	0.07 - 7.85	1.01

Data Plot and Equation



MUTCD 2009-Signal Warrant Analysis

INTERSECTION: Byington Beaver Ridge Road (S.R. 131) & Garrison Drive (YR 2013)	
JOB NUMBER: 101299	
DATE: 11/08/2013	
85TH PERCENTILE SPEED: 41	PEDESTRIAN GAPS/HOUR : 28
POPULATION: 350,000	ESTABLISHED SCHOOL CROSSING, MINIMUM 20 Xing (YES/NO): NO
	NEAREST SIGNALIZED INTERSECTION: 1000
NUMBER OF APPROACHES: 4	IMPROVE PROGRESSION-PLATOONING (YES/NO): NO
LANES ON MAIN STREET: 1	MAJOR ROUTES (YES/NO): NO
MINOR STREET APPROACH LANES: 1	WARRANTS IN 5 YRS (YES/NO): NO
PEDESTRIANS: N/A	ALTERNATIVES TO A SIGNAL EXPLORED: YES
	NUMBER OF ACCIDENTS: 0
K-HOUR VOLUME (4-CONSECUTIVE 15MIN PERIODS): YES	PEAK HOUR DELAY (VEH-HR): 2.76
278% MAJOR: 1066	PROXIMITY OF RR ON MINOR APPROACH TO MAJOR STREET: 0
MINOR: 187	RAIL TRAFFIC FREQUENCY: 0
	MINOR APPROACH HIGH-OCCUPACY BUSES : 0.0%
EXISTING OR PROPOSED SIGNAL SYSTEM (YES/NO): N/A	TRACTOR-TRAILER PERCENTAGE 0.0%

HOUR	MAIN STREET			MINOR STREET				COMBINATION WARRANT A&B	4-HOUR WARRANT 2		PEAK HOUR WARRANT 3B		
	MAIN STREET VOLUME	PERCENT OF WARRANT 1A	PERCENT OF WARRANT 1B	MINOR STREET VOLUME	MINIMUM VOLUME WARRANT 1A	INTERRUPTION WARRANT 1B	WARRANT 1A		WARRANT 1B	WARRANT 2	WARRANT 3B		
		420	525		140	53							
24-1	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
1-2	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
2-3	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
3-4	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
4-5	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
5-6	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
6-7	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
7-8	976	232%	186%	171	122%	YES	325%	YES	YES	284%	YES	214%	YES
8-9	768	183%	146%	120	86%	NO	228%	YES	YES	216%	YES	95%	YES/NO
9-10	569	135%	108%	13	9%	NO	25%	NO	NO	13%	NO	7%	NO
10-11	595	142%	113%	15	10%	NO	28%	NO	NO	15%	NO	8%	NO
11-12	586	140%	112%	15	11%	NO	28%	NO	NO	15%	NO	8%	NO
12-13	537	128%	102%	16	12%	NO	31%	NO	NO	14%	NO	8%	NO
13-14	646	154%	123%	16	12%	NO	31%	NO	NO	20%	NO	10%	NO
14-15	656	156%	125%	31	22%	NO	59%	NO	NO	39%	NO	19%	NO
15-16	853	203%	162%	122	87%	NO	233%	YES	YES	204%	YES	117%	YES
16-17	599	143%	114%	28	20%	NO	54%	NO	NO	30%	NO	16%	NO
17-18	1,049	250%	200%	46	33%	NO	88%	NO	NO	77%	NO	67%	NO
18-19	0	0%	0%	19	14%	NO	37%	NO	NO	5%	NO	4%	NO
19-20	0	0%	0%	16	12%	NO	31%	NO	NO	4%	NO	3%	NO
20-21	0	0%	0%	13	10%	NO	25%	NO	NO	4%	NO	3%	NO
21-22	0	0%	0%	10	7%	NO	20%	NO	NO	3%	NO	2%	NO
22-23	0	0%	0%	7	5%	NO	14%	NO	NO	2%	NO	1%	NO
23-24	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO

S U M M A R Y	WARRANT	WARRANT DESCRIPTION	WARRANT OBTAINED?	HOURS	>=90% HOURS	PRIORITY POINTS
		1 A	MINIMUM VOLUME:	NO	1	0
	B	INTERRUPTION:	NO	3	0	30
	A & B	COMBINATION:	NO	3	N/A	27
	2	FOUR-HOUR:	NO	3	0	48
	3 A	PEAK HOUR DELAY:	NO	N/A	N/A	0
	B	PEAK HOUR VOLUME:	YES	2	1	112
	4	No data collected MINIMUM PED. VOLUMES:	N/A	N/A	N/A	N/A
	5	SCHOOL CROSSING:	NO	N/A	N/A	0
	6	CORD. SIGNAL SYSTEM:	NO	N/A	N/A	0
	7	ACCIDENT EXPERIENCE:	NO	4	N/A	0
	8	ROADWAY NETWORK:	NO	3	N/A	0
	9	INTERSECTION NEAR A GRADE CROSS	N/A	0	0	0
	PRIORITY VALUE					228

MUTCD 2009-Signal Warrant Analysis

INTERSECTION: Garrison Drive & Industrial Park Access (YR 2018)	
JOB NUMBER: 101299	
DATE: 11/08/2013	
85TH PERCENTILE SPEED: 30	PEDESTRIAN GAPS/HOUR : 28
POPULATION: 350,000	ESTABLISHED SCHOOL CROSSING, MINIMUM 20 Xing (YES/NO): NO
	NEAREST SIGNALIZED INTERSECTION: 1000
NUMBER OF APPROACHES: 3	IMPROVE PROGRESSION-PLATOONING (YES/NO): NO
LANES ON MAIN STREET: 1	MAJOR ROUTES (YES/NO): NO
MINOR STREET APPROACH LANES: 1	WARRANTS IN 5 YRS (YES/NO): NO
PEDESTRIANS: N/A	ALTERNATIVES TO A SIGNAL EXPLORED: YES
	NUMBER OF ACCIDENTS: 0
K-HOUR VOLUME (4-CONSECUTIVE 15MIN PERIODS) N/A	PEAK HOUR DELAY (VEH-HR): 0
MAJOR: 0	
MINOR: 0	PROXIMITY OF RR ON MINOR APPROACH TO MAJOR STREET: 0
	RAIL TRAFFIC FREQUENCY: 0 0
EXISTING OR PROPOSED SIGNAL SYSTEM (YES/NO): N/A	MINOR APPROACH HIGH-OCCUPACY BUSES : 0.0% 0
	TRACTOR-TRAILER PERCENTAGE 0.0% 0

HOUR	MAIN STREET			MINOR STREET				COMBINATION WARRANT A&B	4-HOUR WARRANT 2		PEAK HOUR WARRANT 3B		
	MAIN STREET VOLUME	PERCENT OF WARRANT 1A	PERCENT OF WARRANT 1B	MINOR STREET VOLUME	MINIMUM VOLUME WARRANT 1A		INTERRUPTION WARRANT 1B		WARRANT 1A	WARRANT 1B	WARRANT 2	WARRANT 3B	
		600	750		200	75							
24-1	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
1-2	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
2-3	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
3-4	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
4-5	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
5-6	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
6-7	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
7-8	110	18%	15%	536	268%	NO	714%	NO	NO	114%	YES	81%	NO
8-9	110	18%	15%	531	266%	NO	708%	NO	NO	113%	YES	80%	NO
9-10	204	34%	27%	223	111%	NO	297%	NO	NO	54%	NO	37%	NO
10-11	236	39%	31%	139	70%	NO	186%	NO	NO	35%	NO	24%	NO
11-12	314	52%	42%	298	149%	NO	397%	NO	NO	85%	NO	56%	NO
12-13	314	52%	42%	299	150%	NO	399%	NO	NO	85%	NO	57%	NO
13-14	189	31%	25%	200	100%	NO	267%	NO	NO	47%	NO	33%	NO
14-15	189	31%	25%	147	73%	NO	196%	NO	NO	35%	NO	24%	NO
15-16	251	42%	34%	154	77%	NO	205%	NO	NO	40%	NO	27%	NO
16-17	497	83%	66%	164	82%	NO	219%	NO	NO	62%	NO	39%	NO
17-18	497	83%	66%	162	81%	NO	216%	NO	NO	61%	NO	38%	NO
18-19	94	16%	13%	76	38%	NO	101%	NO	NO	16%	NO	11%	NO
19-20	63	10%	8%	73	36%	NO	97%	NO	NO	15%	NO	10%	NO
20-21	47	8%	6%	56	28%	NO	74%	NO	NO	11%	NO	8%	NO
21-22	16	3%	2%	24	12%	NO	33%	NO	NO	5%	NO	3%	NO
22-23	0	0%	0%	7	4%	NO	10%	NO	NO	1%	NO	1%	NO
23-24	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO

WARRANT	WARRANT DESCRIPTION	WARRANT OBTAINED?	HOURS	>=90% HOURS	PRIORITY POINTS
B	INTERRUPTION:	NO	0	0	0
A & B	COMBINATION:	NO	0	N/A	0
2	FOUR-HOUR:	NO	2	0	32
3 A	PEAK HOUR DELAY:	N/A	N/A	N/A	0
B	PEAK HOUR VOLUME:	NO	0	0	0
4	<small>No data collected</small> MINIMUM PED. VOLUMES:	N/A	N/A	N/A	N/A
5	SCHOOL CROSSING:	NO	N/A	N/A	0
6	CORD. SIGNAL SYSTEM:	NO	N/A	N/A	0
7	ACCIDENT EXPERIENCE:	NO	2	N/A	0
8	ROADWAY NETWORK:	NO	0	N/A	0
9	INTERSECTION NEAR A GRADE CROSS	N/A	0	0	0
PRIORITY VALUE					32

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MUTCD 2009-Signal Warrant Analysis

INTERSECTION: Karns Connector & Garrison Drive (YR 2018)	
JOB NUMBER: 101299	
DATE: 12/16/2013	
85TH PERCENTILE SPEED: 40	PEDESTRIAN GAPS/HOUR : 28
POPULATION: 350,000	ESTABLISHED SCHOOL CROSSING, MINIMUM 20 Xing (YES/NO): NO
	NEAREST SIGNALIZED INTERSECTION: 1000
NUMBER OF APPROACHES: 3	IMPROVE PROGRESSION-PLATOONING (YES/NO): NO
LANES ON MAIN STREET: 1	MAJOR ROUTES (YES/NO): NO
MINOR STREET APPROACH LANES: 2	WARRANTS IN 5 YRS (YES/NO): NO
PEDESTRIANS: N/A	ALTERNATIVES TO A SIGNAL EXPLORED: YES
	NUMBER OF ACCIDENTS: 0
K-HOUR VOLUME (4-CONSECUTIVE 15MIN PERIODS) N/A	PEAK HOUR DELAY (VEH-HR): 0
MAJOR: 0	
MINOR: 0	PROXIMITY OF RR ON MINOR APPROACH TO MAJOR STREET: 0
	RAIL TRAFFIC FREQUENCY: 0 0
EXISTING OR PROPOSED SIGNAL SYSTEM (YES/NO): N/A	MINOR APPROACH HIGH-OCCUPACY BUSES : 0.0% 0
	TRACTOR-TRAILER PERCENTAGE 0.0% 0

HOUR	MAIN STREET			MINOR STREET				COMBINATION WARRANT A&B	4-HOUR WARRANT 2		PEAK HOUR WARRANT 3B		
	MAIN STREET VOLUME	PERCENT OF WARRANT 1A	PERCENT OF WARRANT 1B	MINOR STREET VOLUME	MINIMUM VOLUME WARRANT 1A		INTERRUPTION WARRANT 1B		WARRANT 1	WARRANT 2	WARRANT 3A	WARRANT 3B	
		600	750		200	100							
24-1	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
1-2	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
2-3	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
3-4	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
4-5	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
5-6	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
6-7	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
7-8	1,265	211%	169%	67	34%	NO	67%	NO	NO	58%	NO	33%	NO
8-9	1,077	180%	144%	60	30%	NO	60%	NO	NO	49%	NO	23%	NO
9-10	956	159%	127%	95	47%	NO	95%	YES/NO	NO	61%	NO	31%	NO
10-11	918	153%	122%	109	55%	NO	109%	YES	NO	66%	NO	34%	NO
11-12	909	152%	121%	141	70%	NO	141%	YES	NO	84%	NO	43%	NO
12-13	909	152%	121%	142	71%	NO	142%	YES	NO	85%	NO	43%	NO
13-14	943	157%	126%	92	46%	NO	92%	YES/NO	NO	58%	NO	29%	NO
14-15	902	150%	120%	92	46%	NO	92%	YES/NO	NO	54%	NO	28%	NO
15-16	871	145%	116%	119	59%	NO	119%	YES	NO	66%	NO	35%	NO
16-17	1,125	188%	150%	218	109%	YES	218%	YES	YES	193%	YES	88%	NO
17-18	1,118	186%	149%	220	110%	YES	220%	YES	YES	192%	YES	88%	NO
18-19	965	161%	129%	57	29%	NO	57%	NO	NO	38%	NO	19%	NO
19-20	730	122%	97%	41	21%	NO	41%	NO	NO	18%	NO	10%	NO
20-21	645	108%	86%	32	16%	NO	32%	NO	NO	12%	NO	7%	NO
21-22	554	92%	74%	17	8%	NO	17%	NO	NO	5%	NO	3%	NO
22-23	392	65%	52%	7	4%	NO	7%	NO	NO	2%	NO	1%	NO
23-24	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO

S U M M A R Y	WARRANT	WARRANT DESCRIPTION	WARRANT OBTAINED?	HOURS	>=90% HOURS	PRIORITY POINTS
		1 A	MINIMUM VOLUME:	NO	2	0
	B	INTERRUPTION:	NO	6	3	60
	A & B	COMBINATION:	NO	2	N/A	18
	2	FOUR-HOUR:	NO	2	0	32
	3 A	PEAK HOUR DELAY:	N/A	N/A	N/A	0
	B	PEAK HOUR VOLUME:	NO	0	0	0
	4	No data collected MINIMUM PED. VOLUMES:	N/A	N/A	N/A	N/A
	5	SCHOOL CROSSING:	NO	N/A	N/A	0
	6	CORD. SIGNAL SYSTEM:	NO	N/A	N/A	0
	7	ACCIDENT EXPERIENCE:	NO	9	N/A	0
	8	ROADWAY NETWORK:	NO	10	N/A	0
	9	INTERSECTION NEAR A GRADE CROSS	N/A	0	0	0
	PRIORITY VALUE					132

MUTCD 2009-Signal Warrant Analysis

INTERSECTION: Karns Connector & Garrison Drive (YR 2018)	
JOB NUMBER: 101299	
DATE: 12/16/2013	
85TH PERCENTILE SPEED: 41	PEDESTRIAN GAPS/HOUR : 28
POPULATION: 350,000	ESTABLISHED SCHOOL CROSSING, MINIMUM 20 Xing (YES/NO): NO
	NEAREST SIGNALIZED INTERSECTION: 1000
NUMBER OF APPROACHES: 3	IMPROVE PROGRESSION-PLATOONING (YES/NO): NO
LANES ON MAIN STREET: 1	MAJOR ROUTES (YES/NO): NO
MINOR STREET APPROACH LANES: 2	WARRANTS IN 5 YRS (YES/NO): NO
PEDESTRIANS: N/A	ALTERNATIVES TO A SIGNAL EXPLORED: YES
	NUMBER OF ACCIDENTS: 0
K-HOUR VOLUME (4-CONSECUTIVE 15MIN PERIODS) N/A	PEAK HOUR DELAY (VEH-HR): 0
MAJOR: 0	
MINOR: 0	PROXIMITY OF RR ON MINOR APPROACH TO MAJOR STREET: 0
	RAIL TRAFFIC FREQUENCY: 0 0
EXISTING OR PROPOSED SIGNAL SYSTEM (YES/NO): N/A	MINOR APPROACH HIGH-OCCUPACY BUSES : 0.0% 0
	TRACTOR-TRAILER PERCENTAGE 0.0% 0

HOUR	MAIN STREET			MINOR STREET				COMBINATION WARRANT A&B	4-HOUR WARRANT 2		PEAK HOUR WARRANT 3B		
	MAIN STREET VOLUME	PERCENT OF WARRANT 1A	PERCENT OF WARRANT 1B	MINOR STREET VOLUME	MINIMUM VOLUME WARRANT 1A	INTERRUPTION WARRANT 1B							
		420	525		140	70							
24-1	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
1-2	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
2-3	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
3-4	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
4-5	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
5-6	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
6-7	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO
7-8	1,265	301%	241%	67	48%	NO	96%	YES/NO	NO	84%	NO	67%	NO
8-9	1,077	257%	205%	60	43%	NO	86%	NO	NO	75%	NO	64%	NO
9-10	956	228%	182%	95	68%	NO	136%	YES	NO	119%	YES	80%	NO
10-11	918	219%	175%	109	78%	NO	156%	YES	NO	136%	YES	85%	NO
11-12	909	216%	173%	141	101%	YES	201%	YES	YES	176%	YES	107%	YES
12-13	909	216%	173%	142	102%	YES	203%	YES	YES	178%	YES	109%	YES
13-14	943	225%	180%	92	66%	NO	131%	YES	NO	115%	YES	75%	NO
14-15	902	215%	172%	92	65%	NO	131%	YES	NO	114%	YES	69%	NO
15-16	871	207%	166%	119	85%	NO	169%	YES	YES	148%	YES	84%	NO
16-17	1,125	268%	214%	218	156%	YES	311%	YES	YES	273%	YES	254%	YES
17-18	1,118	266%	213%	220	157%	YES	315%	YES	YES	275%	YES	253%	YES
18-19	965	230%	184%	57	41%	NO	81%	NO	NO	71%	NO	49%	NO
19-20	730	174%	139%	41	30%	NO	59%	NO	NO	44%	NO	22%	NO
20-21	645	154%	123%	32	23%	NO	46%	NO	NO	27%	NO	15%	NO
21-22	554	132%	106%	17	12%	NO	24%	NO	NO	11%	NO	6%	NO
22-23	392	93%	75%	7	5%	NO	11%	NO	NO	3%	NO	2%	NO
23-24	0	0%	0%	0	0%	NO	0%	NO	NO	0%	NO	0%	NO

WARRANT	WARRANT DESCRIPTION	WARRANT OBTAINED?	HOURS	>=90% HOURS	PRIORITY POINTS
B	INTERRUPTION:	YES	9	1	90
A & B	COMBINATION:	NO	5	N/A	45
2	FOUR-HOUR:	YES	9	0	144
3 A	PEAK HOUR DELAY:	N/A	N/A	N/A	0
B	PEAK HOUR VOLUME:	YES	4	0	224
4	No data collected MINIMUM PED. VOLUMES:	N/A	N/A	N/A	N/A
5	SCHOOL CROSSING:	NO	N/A	N/A	0
6	CORD. SIGNAL SYSTEM:	NO	N/A	N/A	0
7	ACCIDENT EXPERIENCE:	NO	12	N/A	0
8	ROADWAY NETWORK:	NO	10	N/A	0
9	INTERSECTION NEAR A GRADE CROSS	N/A	0	0	0
PRIORITY VALUE					547

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Intersection

Intersection Delay, s/veh 40.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	3	5	1	113	29	45	4	339	117	36	543	27
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	63	25	88	66	87	33	79	79	75	89	36
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	8	4	128	44	52	12	429	148	48	610	75

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1319	1345	648	1277	1308	503	685	0	0	577	0	0
Stage 1	744	744	-	527	527	-	-	-	-	-	-	-
Stage 2	575	601	-	750	781	-	-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Capacity-1 Maneuver	134	151	470	143	159	569	908	-	-	996	-	-
Stage 1	407	421	-	535	528	-	-	-	-	-	-	-
Stage 2	503	489	-	403	405	-	-	-	-	-	-	-
Time blocked-Platoon, %								-	-	-	-	-
Mov Capacity-1 Maneuver	86	136	470	# 125	144	569	908	-	-	996	-	-
Mov Capacity-2 Maneuver	86	136	-	# 125	144	-	-	-	-	-	-	-
Stage 1	399	388	-	524	517	-	-	-	-	-	-	-
Stage 2	410	479	-	360	373	-	-	-	-	-	-	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	33.8			278.9			0.2			0.6		
HCM LOS	D			F								

Minor Lane / Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	908	-	-	141	157	996	-	-
HCM Lane V/C Ratio	0.013	-	-	0.113	1.427	0.048	-	-
HCM Control Delay (s)	9.018	0	-	33.8	278.9	8.797	0	-
HCM Lane LOS	A	A	-	D	F	A	A	-
HCM 95th %tile Q(veh)	0.041	-	-	0.373	14.272	0.152	-	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 3.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	7	8	1	25	4	13	9	512	70	47	401	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	58	67	25	57	50	41	56	91	80	73	94	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	12	12	4	44	8	32	16	563	88	64	427	12

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1219	1243	433	1208	1206	606	439	0	0	650	0	0
Stage 1	561	561	-	639	639	-	-	-	-	-	-	-
Stage 2	658	682	-	569	567	-	-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Capacity-1 Maneuver	157	174	623	160	184	497	1121	-	-	936	-	-
Stage 1	512	510	-	464	470	-	-	-	-	-	-	-
Stage 2	453	450	-	507	507	-	-	-	-	-	-	-
Time blocked-Platoon, %								-	-	-	-	-
Mov Capacity-1 Maneuver	129	155	623	137	163	497	1121	-	-	936	-	-
Mov Capacity-2 Maneuver	129	155	-	137	163	-	-	-	-	-	-	-
Stage 1	500	464	-	453	459	-	-	-	-	-	-	-
Stage 2	407	440	-	446	461	-	-	-	-	-	-	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	32.6			37.2			0.2			1.2		
HCM LOS	D			E								

Minor Lane / Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1121	-	-	158	193	936	-	-
HCM Lane V/C Ratio	0.014	-	-	0.177	0.433	0.069	-	-
HCM Control Delay (s)	8.258	0	-	32.6	37.2	9.13	0	-
HCM Lane LOS	A	A	-	D	E	A	A	-
HCM 95th %tile Q(veh)	0.044	-	-	0.623	1.999	0.221	-	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2013 Existing AM Peak w Signal Mitigations
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	3	5	1	113	29	45	4	339	117	36	543	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	8	8	8	8	8	8	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.97			0.97			0.97			0.99	
Flt Protected		0.99			0.97			1.00			1.00	
Satd. Flow (prot)		1541			1520			1798			1831	
Flt Permitted		0.93			0.81			0.99			0.94	
Satd. Flow (perm)		1450			1273			1774			1734	
Peak-hour factor, PHF	0.75	0.63	0.25	0.88	0.66	0.87	0.33	0.79	0.79	0.75	0.89	0.36
Adj. Flow (vph)	4	8	4	128	44	52	12	429	148	48	610	75
RTOR Reduction (vph)	0	3	0	0	19	0	0	20	0	0	7	0
Lane Group Flow (vph)	0	13	0	0	205	0	0	569	0	0	726	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		12.5			12.5			28.9			28.9	
Effective Green, g (s)		12.5			12.5			28.9			28.9	
Actuated g/C Ratio		0.25			0.25			0.59			0.59	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		366			322			1037			1014	
v/s Ratio Prot												
v/s Ratio Perm		0.01			0.16			0.32			0.42	
v/c Ratio		0.04			0.64			0.55			0.72	
Uniform Delay, d1		13.9			16.4			6.3			7.3	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.0			4.1			0.6			2.4	
Delay (s)		13.9			20.5			6.9			9.8	
Level of Service		B			C			A			A	
Approach Delay (s)		13.9			20.5			6.9			9.8	
Approach LOS		B			C			A			A	

Intersection Summary

HCM 2000 Control Delay	10.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.69		
Actuated Cycle Length (s)	49.4	Sum of lost time (s)	8.0
Intersection Capacity Utilization	77.9%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2013 Existing PM Peak w Signal Mitigations
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	7	8	1	25	4	13	9	512	70	47	401	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	8	8	8	8	8	8	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.98			0.95			0.98			1.00	
Flt Protected		0.98			0.97			1.00			0.99	
Satd. Flow (prot)		1550			1492			1827			1845	
Flt Permitted		0.89			0.82			0.99			0.88	
Satd. Flow (perm)		1415			1257			1808			1638	
Peak-hour factor, PHF	0.58	0.67	0.25	0.57	0.50	0.41	0.56	0.91	0.80	0.73	0.94	0.83
Adj. Flow (vph)	12	12	4	44	8	32	16	563	88	64	427	12
RTOR Reduction (vph)	0	4	0	0	29	0	0	6	0	0	1	0
Lane Group Flow (vph)	0	24	0	0	55	0	0	661	0	0	502	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		5.4			5.4			36.6			36.6	
Effective Green, g (s)		5.4			5.4			36.6			36.6	
Actuated g/C Ratio		0.11			0.11			0.73			0.73	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		152			135			1323			1199	
v/s Ratio Prot												
v/s Ratio Perm		0.02			c0.04			c0.37			0.31	
v/c Ratio		0.16			0.41			0.50			0.42	
Uniform Delay, d1		20.2			20.8			2.8			2.6	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.5			2.0			1.3			1.1	
Delay (s)		20.7			22.8			4.2			3.7	
Level of Service		C			C			A			A	
Approach Delay (s)		20.7			22.8			4.2			3.7	
Approach LOS		C			C			A			A	

Intersection Summary

HCM 2000 Control Delay	5.6	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.49		
Actuated Cycle Length (s)	50.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	59.8%	ICU Level of Service	B
Analysis Period (min)	15		

c Critical Lane Group

Intersection

Intersection Delay, s/veh 0.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Vol, veh/h	32	24	547	20	15	800
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	35	26	595	22	16	870

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	1507	605	0
Stage 1	605	-	-
Stage 2	902	-	-
Follow-up Headway	3.518	3.318	-
Pot Capacity-1 Maneuver	133	498	-
Stage 1	545	-	-
Stage 2	396	-	-
Time blocked-Platoon, %			
Mov Capacity-1 Maneuver	131	498	-
Mov Capacity-2 Maneuver	265	-	-
Stage 1	545	-	-
Stage 2	389	-	-

Approach	WB	NB	SB
HCM Control Delay, s	17.2	0	0.2
HCM LOS	C		

Minor Lane / Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	265	498	964	-
HCM Lane V/C Ratio	-	-	0.131	0.052	0.017	-
HCM Control Delay (s)	-	-	20.6	12.6	8.799	-
HCM Lane LOS			C	B	A	
HCM 95th %tile Q(veh)	-	-	0.446	0.165	0.052	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 100.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	3	9	1	130	42	55	4	373	130	41	597	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	63	25	88	66	87	33	79	79	75	89	36
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	4	14	4	148	64	63	12	472	165	55	671	83

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1464	1483	712	1410	1442	554	754	0	0	637	0	0
Stage 1	822	822	-	579	579	-	-	-	-	-	-	-
Stage 2	642	661	-	831	863	-	-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Capacity-1 Maneuver	106	125	432	# 116	132	532	856	-	-	947	-	-
Stage 1	368	388	-	501	501	-	-	-	-	-	-	-
Stage 2	463	460	-	364	372	-	-	-	-	-	-	-
Time blocked-Platoon, %								-	-	-	-	-
Mov Capacity-1 Maneuver	48	110	432	# 94	116	532	856	-	-	947	-	-
Mov Capacity-2 Maneuver	48	110	-	# 94	116	-	-	-	-	-	-	-
Stage 1	360	348	-	490	490	-	-	-	-	-	-	-
Stage 2	347	450	-	311	334	-	-	-	-	-	-	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	51			\$ 637.6			0.2			0.6		
HCM LOS	F			F								

Minor Lane / Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	856	-	-	100	123	947	-	-
HCM Lane V/C Ratio	0.014	-	-	0.223	2.232	0.058	-	-
HCM Control Delay (s)	9.266	0	-	51	\$ 637.6	9.034	0	-
HCM Lane LOS	A	A	-	F	F	A	A	-
HCM 95th %tile Q(veh)	0.043	-	-	0.795	23.356	0.183	-	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	32	24	547	20	15	800
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	12	12	12	12
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	1.00		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1593	1425	1854		1770	1863
Flt Permitted	0.95	1.00	1.00		0.41	1.00
Satd. Flow (perm)	1593	1425	1854		761	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	35	26	595	22	16	870
RTOR Reduction (vph)	0	24	1	0	0	0
Lane Group Flow (vph)	35	2	616	0	16	870
Turn Type	NA	Perm	NA		Perm	NA
Protected Phases	8		2			6
Permitted Phases		8			6	
Actuated Green, G (s)	4.5	4.5	49.1		49.1	49.1
Effective Green, g (s)	4.5	4.5	49.1		49.1	49.1
Actuated g/C Ratio	0.07	0.07	0.80		0.80	0.80
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	116	104	1477		606	1484
v/s Ratio Prot	c0.02		0.33			c0.47
v/s Ratio Perm		0.00			0.02	
v/c Ratio	0.30	0.02	0.42		0.03	0.59
Uniform Delay, d1	27.1	26.5	1.9		1.3	2.4
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	1.5	0.1	0.9		0.1	1.7
Delay (s)	28.5	26.6	2.8		1.4	4.1
Level of Service	C	C	A		A	A
Approach Delay (s)	27.7		2.8			4.0
Approach LOS	C		A			A

Intersection Summary

HCM 2000 Control Delay	4.5	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.56		
Actuated Cycle Length (s)	61.6	Sum of lost time (s)	8.0
Intersection Capacity Utilization	52.1%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2018 No Build AM Peak w Signal Mitigations
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Volume (vph)	3	9	1	130	42	55	4	373	130	41	597	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	9	9	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.98			0.97			0.97			0.99	
Flt Protected		0.99			0.97			1.00			1.00	
Satd. Flow (prot)		1621			1582			1797			1831	
Flt Permitted		0.94			0.82			0.99			0.93	
Satd. Flow (perm)		1545			1332			1774			1713	
Peak-hour factor, PHF	0.75	0.63	0.25	0.88	0.66	0.87	0.33	0.79	0.79	0.75	0.89	0.36
Adj. Flow (vph)	4	14	4	148	64	63	12	472	165	55	671	83
RTOR Reduction (vph)	0	3	0	0	18	0	0	19	0	0	6	0
Lane Group Flow (vph)	0	19	0	0	257	0	0	630	0	0	803	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		14.6			14.6			38.2			38.2	
Effective Green, g (s)		14.6			14.6			38.2			38.2	
Actuated g/C Ratio		0.24			0.24			0.63			0.63	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		371			319			1114			1076	
v/s Ratio Prot												
v/s Ratio Perm		0.01			0.19			0.36			0.47	
v/c Ratio		0.05			0.80			0.57			0.75	
Uniform Delay, d1		17.8			21.8			6.5			7.9	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.1			13.7			2.1			4.7	
Delay (s)		17.8			35.5			8.6			12.6	
Level of Service		B			D			A			B	
Approach Delay (s)		17.8			35.5			8.6			12.6	
Approach LOS		B			D			A			B	

Intersection Summary

HCM 2000 Control Delay	14.8	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.76		
Actuated Cycle Length (s)	60.8	Sum of lost time (s)	8.0
Intersection Capacity Utilization	87.0%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

Intersection

Intersection Delay, s/veh 0.4

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Vol, veh/h	12	12	693	21	22	659
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	13	753	23	24	716

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	1529	765	0
Stage 1	765	-	-
Stage 2	764	-	-
Follow-up Headway	3.518	3.318	-
Pot Capacity-1 Maneuver	129	403	-
Stage 1	459	-	-
Stage 2	460	-	-
Time blocked-Platoon, %			-
Mov Capacity-1 Maneuver	125	403	-
Mov Capacity-2 Maneuver	264	-	-
Stage 1	459	-	-
Stage 2	447	-	-

Approach	WB	NB	SB
HCM Control Delay, s	16.8	0	0.3
HCM LOS	C		

Minor Lane / Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	264	403	840	-
HCM Lane V/C Ratio	-	-	0.049	0.032	0.028	-
HCM Control Delay (s)	-	-	19.3	14.2	9.411	-
HCM Lane LOS			C	B	A	
HCM 95th %tile Q(veh)	-	-	0.155	0.1	0.088	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 8.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	8	20	1	29	8	16	10	563	89	57	441	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	58	67	25	57	50	41	56	91	80	73	94	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	14	30	4	51	16	39	18	619	111	78	469	13

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1370	1398	476	1359	1349	674	482	0	0	730	0	0
Stage 1	632	632	-	710	710	-	-	-	-	-	-	-
Stage 2	738	766	-	649	639	-	-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Capacity-1 Maneuver	124	141	589	126	151	455	1081	-	-	874	-	-
Stage 1	468	474	-	424	437	-	-	-	-	-	-	-
Stage 2	410	412	-	458	470	-	-	-	-	-	-	-
Time blocked-Platoon, %								-	-	-	-	-
Mov Capacity-1 Maneuver	91	120	589	90	129	455	1081	-	-	874	-	-
Mov Capacity-2 Maneuver	91	120	-	90	129	-	-	-	-	-	-	-
Stage 1	454	416	-	412	424	-	-	-	-	-	-	-
Stage 2	350	400	-	371	413	-	-	-	-	-	-	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	55.5			88.6			0.2			1.3		
HCM LOS	F			F								

Minor Lane / Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1081	-	-	117	137	874	-	-
HCM Lane V/C Ratio	0.017	-	-	0.407	0.773	0.089	-	-
HCM Control Delay (s)	8.386	0	-	55.5	88.6	9.523	0	-
HCM Lane LOS	A	A	-	F	F	A	A	-
HCM 95th %tile Q(veh)	0.05	-	-	1.72	4.651	0.293	-	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

HCM Signalized Intersection Capacity Analysis 2018 No Build PM Peak w Signal Mitigations
 2: Karns Valley Connector & Garrison Drive Butler Property Industrial Park TIS



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	12	12	693	21	22	659
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	12	12	12	12
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	1.00		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1593	1425	1855		1770	1863
Flt Permitted	0.95	1.00	1.00		0.35	1.00
Satd. Flow (perm)	1593	1425	1855		648	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	13	13	753	23	24	716
RTOR Reduction (vph)	0	13	1	0	0	0
Lane Group Flow (vph)	13	0	775	0	24	716
Turn Type	NA	Perm	NA		Perm	NA
Protected Phases	8		2			6
Permitted Phases		8			6	
Actuated Green, G (s)	1.2	1.2	46.3		46.3	46.3
Effective Green, g (s)	1.2	1.2	46.3		46.3	46.3
Actuated g/C Ratio	0.02	0.02	0.83		0.83	0.83
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	34	30	1547		540	1554
v/s Ratio Prot	c0.01		c0.42			0.38
v/s Ratio Perm		0.00			0.04	
v/c Ratio	0.38	0.01	0.50		0.04	0.46
Uniform Delay, d1	26.8	26.6	1.3		0.8	1.2
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	7.0	0.1	1.2		0.2	1.0
Delay (s)	33.8	26.7	2.5		0.9	2.2
Level of Service	C	C	A		A	A
Approach Delay (s)	30.3		2.5			2.2
Approach LOS	C		A			A

Intersection Summary			
HCM 2000 Control Delay	2.8	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.50		
Actuated Cycle Length (s)	55.5	Sum of lost time (s)	8.0
Intersection Capacity Utilization	47.7%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis 2018 No Build PM Peak w Signal Mitigations
 3: Byington Beaver Ridge Road & Garrison Drive Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	8	20	1	29	8	16	10	563	89	57	441	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	9	9	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.99			0.95			0.98			1.00	
Flt Protected		0.99			0.98			1.00			0.99	
Satd. Flow (prot)		1634			1556			1823			1844	
Flt Permitted		0.89			0.82			0.99			0.85	
Satd. Flow (perm)		1471			1311			1800			1581	
Peak-hour factor, PHF	0.58	0.67	0.25	0.57	0.50	0.41	0.56	0.91	0.80	0.73	0.94	0.83
Adj. Flow (vph)	14	30	4	51	16	39	18	619	111	78	469	13
RTOR Reduction (vph)	0	3	0	0	33	0	0	9	0	0	1	0
Lane Group Flow (vph)	0	45	0	0	73	0	0	739	0	0	559	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		6.4			6.4			28.0			28.0	
Effective Green, g (s)		6.4			6.4			28.0			28.0	
Actuated g/C Ratio		0.15			0.15			0.66			0.66	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		222			197			1188			1044	
v/s Ratio Prot												
v/s Ratio Perm		0.03			c0.06			c0.41			0.35	
v/c Ratio		0.20			0.37			0.62			0.54	
Uniform Delay, d1		15.8			16.2			4.2			3.8	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.4			1.2			1.0			0.5	
Delay (s)		16.2			17.4			5.2			4.3	
Level of Service		B			B			A			A	
Approach Delay (s)		16.2			17.4			5.2			4.3	
Approach LOS		B			B			A			A	

Intersection Summary		
HCM 2000 Control Delay	6.1	HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio	0.57	A
Actuated Cycle Length (s)	42.4	Sum of lost time (s)
Intersection Capacity Utilization	70.3%	8.0
Analysis Period (min)	15	ICU Level of Service
		C

c Critical Lane Group

Intersection

Intersection Delay, s/veh 20.6

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Vol, veh/h	81	74	547	385	379	800
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	88	80	595	418	412	870

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	2497	804	0
Stage 1	804	-	-
Stage 2	1693	-	-
Follow-up Headway	3.518	3.318	-
Pot Capacity-1 Maneuver	# 32	383	-
Stage 1	440	-	-
Stage 2	164	-	-
Time blocked-Platoon, %			
Mov Capacity-1 Maneuver	# 13	383	-
Mov Capacity-2 Maneuver	# 54	-	-
Stage 1	440	-	-
Stage 2	# 65	-	-

Approach	WB	NB	SB
HCM Control Delay, s	257	0	5.8
HCM LOS	F		

Minor Lane / Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	54	383	684	-
HCM Lane V/C Ratio	-	-	1.63	0.21	0.602	-
HCM Control Delay (s)	-	-	\$ 476.3	16.9	17.907	-
HCM Lane LOS			F	C	C	
HCM 95th %tile Q(veh)	-	-	8.255	0.781	4.059	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 399.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	7	13	4	130	75	55	29	373	130	41	597	54
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	58	67	25	57	50	41	56	91	80	73	94	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	12	19	16	228	150	134	52	410	162	56	635	65

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	1517	1456	668	1393	1407	491	700	0	0	572	0	0
Stage 1	780	780	-	595	595	-	-	-	-	-	-	-
Stage 2	737	676	-	798	812	-	-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Capacity-1 Maneuver	98	130	458	# 119	# 139	578	897	-	-	1001	-	-
Stage 1	388	406	-	491	492	-	-	-	-	-	-	-
Stage 2	410	453	-	380	392	-	-	-	-	-	-	-
Time blocked-Platoon, %								-	-	-	-	-
Mov Capacity-1 Maneuver	-	108	458	# 86	# 115	578	897	-	-	1001	-	-
Mov Capacity-2 Maneuver	-	108	-	# 86	# 115	-	-	-	-	-	-	-
Stage 1	354	368	-	448	449	-	-	-	-	-	-	-
Stage 2	191	414	-	315	356	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	+	\$ 1511.6	0.8	0.7
HCM LOS	-	F		

Minor Lane / Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	897	-	-	+	122	1001	-	-
HCM Lane V/C Ratio	0.058	-	-	+	4.198	0.056	-	-
HCM Control Delay (s)	9.259	0	-	+	\$ 1511.6	8.81	0	-
HCM Lane LOS	A	A	-	+	F	A	A	-
HCM 95th %tile Q(veh)	0.183	-	-	+	52.44	0.178	-	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 2.9

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Vol, veh/h	46	728	81	134	99	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	50	791	88	146	108	12

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	841
Stage 1	-	-	-
Stage 2	-	-	-
Follow-up Headway	-	-	2.218
Pot Capacity-1 Maneuver	-	-	794
Stage 1	-	-	-
Stage 2	-	-	-
Time blocked-Platoon, %	-	-	-
Mov Capacity-1 Maneuver	-	-	794
Mov Capacity-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	3.8	21.2
HCM LOS			C

Minor Lane / Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	341	-	-	794	-
HCM Lane V/C Ratio	0.351	-	-	0.111	-
HCM Control Delay (s)	21.2	-	-	10.099	0
HCM Lane LOS	C			B	A
HCM 95th %tile Q(veh)	1.535	-	-	0.373	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	81	74	547	385	379	800
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	12	12	12	12
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1593	1425	1863	1583	1770	1863
Flt Permitted	0.95	1.00	1.00	1.00	0.20	1.00
Satd. Flow (perm)	1593	1425	1863	1583	371	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	88	80	595	418	412	870
RTOR Reduction (vph)	0	52	0	64	0	0
Lane Group Flow (vph)	88	28	595	354	412	870
Turn Type	NA	pm+ov	NA	pm+ov	pm+pt	NA
Protected Phases	8	1	2	8	1	6
Permitted Phases		8		2	6	
Actuated Green, G (s)	8.7	20.5	25.3	34.0	41.1	41.1
Effective Green, g (s)	8.7	20.5	25.3	34.0	41.1	41.1
Actuated g/C Ratio	0.15	0.35	0.44	0.59	0.71	0.71
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	239	604	815	1040	549	1324
v/s Ratio Prot	0.06	0.01	0.32	c0.05	c0.15	0.47
v/s Ratio Perm		0.01		0.17	c0.38	
v/c Ratio	0.37	0.05	0.73	0.34	0.75	0.66
Uniform Delay, d1	22.1	12.2	13.4	6.1	8.7	4.5
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.0	0.0	5.7	0.2	5.7	2.6
Delay (s)	23.0	12.3	19.1	6.3	14.4	7.1
Level of Service	C	B	B	A	B	A
Approach Delay (s)	17.9		13.8			9.4
Approach LOS	B		B			A

Intersection Summary

HCM 2000 Control Delay	11.8	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.74		
Actuated Cycle Length (s)	57.8	Sum of lost time (s)	12.0
Intersection Capacity Utilization	64.3%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2018 Build AM Peak w Signal Mitigations
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	7	13	4	130	75	55	29	373	130	41	597	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	9	9	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.95			0.96			0.96			0.99	
Flt Protected		0.99			0.98			1.00			1.00	
Satd. Flow (prot)		1579			1582			1790			1834	
Flt Permitted		0.89			0.83			0.92			0.93	
Satd. Flow (perm)		1417			1349			1645			1714	
Peak-hour factor, PHF	0.58	0.67	0.25	0.57	0.50	0.41	0.56	0.91	0.80	0.73	0.94	0.83
Adj. Flow (vph)	12	19	16	228	150	134	52	410	162	56	635	65
RTOR Reduction (vph)	0	10	0	0	22	0	0	21	0	0	6	0
Lane Group Flow (vph)	0	37	0	0	490	0	0	603	0	0	751	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		22.0			22.0			30.0			30.0	
Effective Green, g (s)		22.0			22.0			30.0			30.0	
Actuated g/C Ratio		0.37			0.37			0.50			0.50	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		519			494			822			857	
v/s Ratio Prot												
v/s Ratio Perm		0.03			c0.36			0.37			c0.44	
v/c Ratio		0.07			0.99			0.73			0.88	
Uniform Delay, d1		12.4			18.9			11.8			13.3	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.1			38.4			5.8			12.2	
Delay (s)		12.4			57.4			17.6			25.5	
Level of Service		B			E			B			C	
Approach Delay (s)		12.4			57.4			17.6			25.5	
Approach LOS		B			E			B			C	

Intersection Summary

HCM 2000 Control Delay	31.1	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.92		
Actuated Cycle Length (s)	60.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	75.9%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↻			↻	↻	
Volume (vph)	46	728	81	134	99	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	12	12
Total Lost time (s)	4.0			4.0	4.0	
Lane Util. Factor	1.00			1.00	1.00	
Frt	0.87			1.00	0.99	
Flt Protected	1.00			0.98	0.96	
Satd. Flow (prot)	1464			1646	1758	
Flt Permitted	1.00			0.31	0.96	
Satd. Flow (perm)	1464			526	1758	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	50	791	88	146	108	12
RTOR Reduction (vph)	422	0	0	0	8	0
Lane Group Flow (vph)	419	0	0	234	112	0
Turn Type	NA		Perm	NA	NA	
Protected Phases	4			8	2	
Permitted Phases			8			
Actuated Green, G (s)	21.0			21.0	16.0	
Effective Green, g (s)	21.0			21.0	16.0	
Actuated g/C Ratio	0.47			0.47	0.36	
Clearance Time (s)	4.0			4.0	4.0	
Vehicle Extension (s)	3.0			3.0	3.0	
Lane Grp Cap (vph)	683			245	625	
v/s Ratio Prot	0.29				c0.06	
v/s Ratio Perm				c0.44		
v/c Ratio	0.61			0.96	0.18	
Uniform Delay, d1	9.0			11.5	10.0	
Progression Factor	1.00			1.00	1.00	
Incremental Delay, d2	1.6			44.7	0.6	
Delay (s)	10.6			56.3	10.6	
Level of Service	B			E	B	
Approach Delay (s)	10.6			56.3	10.6	
Approach LOS	B			E	B	

Intersection Summary

HCM 2000 Control Delay	19.6	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.62		
Actuated Cycle Length (s)	45.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	75.1%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	81	74	547	385	379	800
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	12	12	12	12
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.94		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1593	1425	1759		1770	1863
Flt Permitted	0.95	1.00	1.00		0.06	1.00
Satd. Flow (perm)	1593	1425	1759		114	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	88	80	595	418	412	870
RTOR Reduction (vph)	0	72	22	0	0	0
Lane Group Flow (vph)	88	8	991	0	412	870
Turn Type	NA	Perm	NA		pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Actuated Green, G (s)	11.1	11.1	61.1		86.1	86.1
Effective Green, g (s)	11.1	11.1	61.1		86.1	86.1
Actuated g/C Ratio	0.11	0.11	0.58		0.82	0.82
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	168	150	1021		423	1524
v/s Ratio Prot	c0.06		0.56		c0.19	0.47
v/s Ratio Perm		0.01			c0.60	
v/c Ratio	0.52	0.06	0.97		0.97	0.57
Uniform Delay, d1	44.5	42.3	21.2		36.0	3.3
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	2.9	0.2	22.0		36.7	1.6
Delay (s)	47.5	42.5	43.1		72.7	4.8
Level of Service	D	D	D		E	A
Approach Delay (s)	45.1		43.1			26.6
Approach LOS	D		D			C

Intersection Summary

HCM 2000 Control Delay	34.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.95		
Actuated Cycle Length (s)	105.2	Sum of lost time (s)	12.0
Intersection Capacity Utilization	87.8%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2018 Build AM Peak w Mitigations to "T" Garrison Dr.
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	7	13	4	130	75	55	29	373	130	41	597	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	9	9	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.95			0.96			0.96			0.99	
Flt Protected		0.99			0.98			1.00			1.00	
Satd. Flow (prot)		1579			1582			1790			1834	
Flt Permitted		0.89			0.83			0.92			0.93	
Satd. Flow (perm)		1417			1349			1645			1714	
Peak-hour factor, PHF	0.58	0.67	0.25	0.57	0.50	0.41	0.56	0.91	0.80	0.73	0.94	0.83
Adj. Flow (vph)	12	19	16	228	150	134	52	410	162	56	635	65
RTOR Reduction (vph)	0	10	0	0	22	0	0	21	0	0	6	0
Lane Group Flow (vph)	0	37	0	0	490	0	0	603	0	0	751	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		22.0			22.0			30.0			30.0	
Effective Green, g (s)		22.0			22.0			30.0			30.0	
Actuated g/C Ratio		0.37			0.37			0.50			0.50	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		519			494			822			857	
v/s Ratio Prot												
v/s Ratio Perm		0.03			c0.36			0.37			c0.44	
v/c Ratio		0.07			0.99			0.73			0.88	
Uniform Delay, d1		12.4			18.9			11.8			13.3	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		0.1			38.4			5.8			12.2	
Delay (s)		12.4			57.4			17.6			25.5	
Level of Service		B			E			B			C	
Approach Delay (s)		12.4			57.4			17.6			25.5	
Approach LOS		B			E			B			C	

Intersection Summary

HCM 2000 Control Delay	31.1	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.92		
Actuated Cycle Length (s)	60.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	75.9%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

Intersection

Intersection Delay, s/veh 4.3

Movement	NBT	NBR	SBL	SBT	SWL	SWR
Vol, veh/h	99	11	46	728	81	134
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	108	12	50	791	88	146

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	120
Stage 1	-	-	-
Stage 2	-	-	-
Follow-up Headway	-	-	2.218
Pot Capacity-1 Maneuver	-	-	1468
Stage 1	-	-	-
Stage 2	-	-	-
Time blocked-Platoon, %	-	-	-
Mov Capacity-1 Maneuver	-	-	1468
Mov Capacity-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	NB	SB	SW
HCM Control Delay, s	0	0.4	20.4
HCM LOS			C

Minor Lane / Major Mvmt	NBT	NBR	SBL	SBT	SWLn1
Capacity (veh/h)	-	-	1468	-	463
HCM Lane V/C Ratio	-	-	0.034	-	0.505
HCM Control Delay (s)	-	-	7.539	0	20.4
HCM Lane LOS			A	A	C
HCM 95th %tile Q(veh)	-	-	0.106	-	2.787

Notes
 ~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 128.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Vol, veh/h	433	433	693	79	80	659
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	100	-	-	100	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	471	471	753	86	87	716

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	1686	796	0
Stage 1	796	-	-
Stage 2	890	-	-
Follow-up Headway	3.518	3.318	-
Pot Capacity-1 Maneuver	# 103	# 387	-
Stage 1	# 444	-	-
Stage 2	# 401	-	-
Time blocked-Platoon, %			
Mov Capacity-1 Maneuver	# 92	# 387	-
Mov Capacity-2 Maneuver	# 222	-	-
Stage 1	# 444	-	-
Stage 2	# 357	-	-

Approach	WB	NB	SB
HCM Control Delay, s	\$ 351.8	0	1.1
HCM LOS	F		

Minor Lane / Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	222	387	796	-
HCM Lane V/C Ratio	-	-	2.12	1.216	0.109	-
HCM Control Delay (s)	-	-	\$ 554.3	149.3	10.076	-
HCM Lane LOS			F	F	B	
HCM 95th %tile Q(veh)	-	-	35.986	19.505	0.366	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 104.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	36	58	29	29	13	16	14	563	89	57	441	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	58	67	25	57	50	41	56	91	80	73	94	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	62	87	116	51	26	39	25	619	111	78	469	18

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1391	1414	478	1460	1367	674	487	0	0	730	0	0
Stage 1	634	634	-	724	724	-	-	-	-	-	-	-
Stage 2	757	780	-	736	643	-	-	-	-	-	-	-
Follow-up Headway	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Capacity-1 Maneuver	120	138	587	107	147	455	1076	-	-	874	-	-
Stage 1	467	473	-	417	430	-	-	-	-	-	-	-
Stage 2	400	406	-	411	468	-	-	-	-	-	-	-
Time blocked-Platoon, %								-	-	-	-	-
Mov Capacity-1 Maneuver	81	116	587	# 30	124	455	1076	-	-	874	-	-
Mov Capacity-2 Maneuver	81	116	-	# 30	124	-	-	-	-	-	-	-
Stage 1	448	415	-	400	413	-	-	-	-	-	-	-
Stage 2	329	390	-	229	410	-	-	-	-	-	-	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	\$ 395.2			\$ 617.7			0.3			1.3		
HCM LOS	F			F								

Minor Lane / Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1076	-	-	155	58	874	-	-
HCM Lane V/C Ratio	0.023	-	-	1.707	1.998	0.089	-	-
HCM Control Delay (s)	8.425	0	-	\$ 395.2	\$ 617.7	9.523	0	-
HCM Lane LOS	A	A	-	F	F	A	A	-
HCM 95th %tile Q(veh)	0.071	-	-	18.943	11.139	0.293	-	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Intersection

Intersection Delay, s/veh 161.3

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Vol, veh/h	79	115	13	94	844	94
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	86	125	14	102	917	102

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	211
Stage 1	-	-	-
Stage 2	-	-	-
Follow-up Headway	-	-	2.218
Pot Capacity-1 Maneuver	-	-	1360
Stage 1	-	-	-
Stage 2	-	-	-
Time blocked-Platoon, %	-	-	-
Mov Capacity-1 Maneuver	-	-	1360
Mov Capacity-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.9	212.9
HCM LOS			F

Minor Lane / Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	720	-	-	1360	-
HCM Lane V/C Ratio	1.416	-	-	0.01	-
HCM Control Delay (s)	212.9	-	-	7.675	0
HCM Lane LOS	F			A	A
HCM 95th %tile Q(veh)	45.795	-	-	0.031	-

Notes

~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	433	433	693	79	80	659
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	12	12	12	12
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1593	1425	1863	1583	1770	1863
Flt Permitted	0.95	1.00	1.00	1.00	0.13	1.00
Satd. Flow (perm)	1593	1425	1863	1583	233	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	471	471	753	86	87	716
RTOR Reduction (vph)	0	74	0	21	0	0
Lane Group Flow (vph)	471	397	753	65	87	716
Turn Type	NA	pm+ov	NA	pm+ov	pm+pt	NA
Protected Phases	8	1	2	8	1	6
Permitted Phases		8		2	6	
Actuated Green, G (s)	23.0	29.0	34.0	57.0	44.0	44.0
Effective Green, g (s)	23.0	29.0	34.0	57.0	44.0	44.0
Actuated g/C Ratio	0.31	0.39	0.45	0.76	0.59	0.59
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	488	627	844	1287	259	1092
v/s Ratio Prot	c0.30	0.05	c0.40	0.02	0.03	c0.38
v/s Ratio Perm		0.23		0.03	0.17	
v/c Ratio	0.97	0.63	0.89	0.05	0.34	0.66
Uniform Delay, d1	25.6	18.7	18.8	2.2	24.3	10.4
Progression Factor	0.86	0.70	1.00	1.00	1.00	1.00
Incremental Delay, d2	24.5	1.4	13.8	0.0	0.8	3.1
Delay (s)	46.6	14.5	32.6	2.3	25.0	13.5
Level of Service	D	B	C	A	C	B
Approach Delay (s)	30.6		29.5			14.7
Approach LOS	C		C			B

Intersection Summary

HCM 2000 Control Delay	25.3	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.92		
Actuated Cycle Length (s)	75.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	74.9%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2018 Build PM Peak w Signal Mitigations
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	36	58	29	29	13	16	14	563	89	57	441	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	9	9	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.94			0.95			0.98			1.00	
Flt Protected		0.99			0.98			1.00			0.99	
Satd. Flow (prot)		1559			1566			1823			1842	
Flt Permitted		0.91			0.73			0.98			0.84	
Satd. Flow (perm)		1442			1170			1785			1565	
Peak-hour factor, PHF	0.58	0.67	0.25	0.57	0.50	0.41	0.56	0.91	0.80	0.73	0.94	0.83
Adj. Flow (vph)	62	87	116	51	26	39	25	619	111	78	469	18
RTOR Reduction (vph)	0	50	0	0	31	0	0	9	0	0	2	0
Lane Group Flow (vph)	0	215	0	0	85	0	0	746	0	0	563	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		12.7			12.7			38.1			38.1	
Effective Green, g (s)		12.7			12.7			38.1			38.1	
Actuated g/C Ratio		0.22			0.22			0.65			0.65	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		311			252			1156			1014	
v/s Ratio Prot												
v/s Ratio Perm		c0.15			0.07			c0.42			0.36	
v/c Ratio		0.69			0.34			0.65			0.56	
Uniform Delay, d1		21.2			19.5			6.3			5.7	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		6.5			0.8			2.8			2.2	
Delay (s)		27.7			20.3			9.0			7.9	
Level of Service		C			C			A			A	
Approach Delay (s)		27.7			20.3			9.0			7.9	
Approach LOS		C			C			A			A	

Intersection Summary

HCM 2000 Control Delay	12.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.66		
Actuated Cycle Length (s)	58.8	Sum of lost time (s)	8.0
Intersection Capacity Utilization	68.9%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↻			↻	↻	
Volume (vph)	79	115	13	94	844	94
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	12	12
Total Lost time (s)	4.0			4.0	4.0	
Lane Util. Factor	1.00			1.00	1.00	
Frt	0.92			1.00	0.99	
Flt Protected	1.00			0.99	0.96	
Satd. Flow (prot)	1542			1666	1758	
Flt Permitted	1.00			0.88	0.96	
Satd. Flow (perm)	1542			1481	1758	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	86	125	14	102	917	102
RTOR Reduction (vph)	75	0	0	0	4	0
Lane Group Flow (vph)	136	0	0	116	1015	0
Turn Type	NA		Perm	NA	NA	
Protected Phases	4			8	2	
Permitted Phases			8			
Actuated Green, G (s)	11.4			11.4	55.6	
Effective Green, g (s)	11.4			11.4	55.6	
Actuated g/C Ratio	0.15			0.15	0.74	
Clearance Time (s)	4.0			4.0	4.0	
Vehicle Extension (s)	3.0			3.0	3.0	
Lane Grp Cap (vph)	234			225	1303	
v/s Ratio Prot	c0.09				c0.58	
v/s Ratio Perm				0.08		
v/c Ratio	0.58			0.52	0.78	
Uniform Delay, d1	29.6			29.3	5.9	
Progression Factor	0.74			1.00	1.00	
Incremental Delay, d2	3.4			2.0	4.6	
Delay (s)	25.3			31.3	10.6	
Level of Service	C			C	B	
Approach Delay (s)	25.3			31.3	10.6	
Approach LOS	C			C	B	

Intersection Summary

HCM 2000 Control Delay	14.7	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.74		
Actuated Cycle Length (s)	75.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	75.0%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↰	↰	↰		↰	↰
Volume (vph)	433	433	693	79	80	659
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width	9	9	12	12	12	12
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1593	1425	1837		1770	1863
Flt Permitted	0.95	1.00	1.00		0.10	1.00
Satd. Flow (perm)	1593	1425	1837		184	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	471	471	753	86	87	716
RTOR Reduction (vph)	0	165	6	0	0	0
Lane Group Flow (vph)	471	306	833	0	87	716
Turn Type	NA	Perm	NA		pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8			6	
Actuated Green, G (s)	22.0	22.0	36.6		43.8	43.8
Effective Green, g (s)	22.0	22.0	36.6		43.8	43.8
Actuated g/C Ratio	0.30	0.30	0.50		0.59	0.59
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	474	424	911		177	1105
v/s Ratio Prot	c0.30		c0.45		0.02	c0.38
v/s Ratio Perm		0.21			0.27	
v/c Ratio	0.99	0.72	0.91		0.49	0.65
Uniform Delay, d1	25.8	23.2	17.2		14.3	9.9
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	39.5	6.0	15.2		2.1	2.9
Delay (s)	65.3	29.1	32.4		16.5	12.9
Level of Service	E	C	C		B	B
Approach Delay (s)	47.2		32.4			13.2
Approach LOS	D		C			B

Intersection Summary

HCM 2000 Control Delay	31.8	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.94		
Actuated Cycle Length (s)	73.8	Sum of lost time (s)	12.0
Intersection Capacity Utilization	79.7%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Byington Beaver Ridge Road & Garrison Drive

2018 Build PM Peak w Mitigations to "T" Garrison Dr.
 Butler Property Industrial Park TIS



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Volume (vph)	36	58	29	29	13	16	14	563	89	57	441	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	9	9	9	9	9	9	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0			4.0	
Lane Util. Factor		1.00			1.00			1.00			1.00	
Frt		0.94			0.95			0.98			1.00	
Flt Protected		0.99			0.98			1.00			0.99	
Satd. Flow (prot)		1559			1566			1823			1842	
Flt Permitted		0.91			0.73			0.98			0.84	
Satd. Flow (perm)		1442			1170			1785			1565	
Peak-hour factor, PHF	0.58	0.67	0.25	0.57	0.50	0.41	0.56	0.91	0.80	0.73	0.94	0.83
Adj. Flow (vph)	62	87	116	51	26	39	25	619	111	78	469	18
RTOR Reduction (vph)	0	50	0	0	31	0	0	9	0	0	2	0
Lane Group Flow (vph)	0	215	0	0	85	0	0	746	0	0	563	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		
Actuated Green, G (s)		12.7			12.7			38.1			38.1	
Effective Green, g (s)		12.7			12.7			38.1			38.1	
Actuated g/C Ratio		0.22			0.22			0.65			0.65	
Clearance Time (s)		4.0			4.0			4.0			4.0	
Vehicle Extension (s)		3.0			3.0			3.0			3.0	
Lane Grp Cap (vph)		311			252			1156			1014	
v/s Ratio Prot												
v/s Ratio Perm		c0.15			0.07			c0.42			0.36	
v/c Ratio		0.69			0.34			0.65			0.56	
Uniform Delay, d1		21.2			19.5			6.3			5.7	
Progression Factor		1.00			1.00			1.00			1.00	
Incremental Delay, d2		6.5			0.8			2.8			2.2	
Delay (s)		27.7			20.3			9.0			7.9	
Level of Service		C			C			A			A	
Approach Delay (s)		27.7			20.3			9.0			7.9	
Approach LOS		C			C			A			A	

Intersection Summary

HCM 2000 Control Delay	12.3	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.66		
Actuated Cycle Length (s)	58.8	Sum of lost time (s)	8.0
Intersection Capacity Utilization	68.9%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

Intersection						
Intersection Delay, s/veh	3					
Movement						
	WBL	WBR	NBT	NBR	SBL	SBT
Vol, veh/h	13	94	844	94	79	115
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	14	102	917	102	86	125
Major/Minor						
	Minor1		Major1		Major2	
Conflicting Flow All	1265	968	0	0	1020	0
Stage 1	968	-	-	-	-	-
Stage 2	297	-	-	-	-	-
Follow-up Headway	3.518	3.318	-	-	2.218	-
Pot Capacity-1 Maneuver	187	308	-	-	680	-
Stage 1	368	-	-	-	-	-
Stage 2	754	-	-	-	-	-
Time blocked-Platoon, %			-	-		-
Mov Capacity-1 Maneuver	162	308	-	-	680	-
Mov Capacity-2 Maneuver	162	-	-	-	-	-
Stage 1	368	-	-	-	-	-
Stage 2	651	-	-	-	-	-
Approach						
	WB		NB		SB	
HCM Control Delay, s	27		0		4.5	
HCM LOS	D					
Minor Lane / Major Mvmt						
		NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	278	680	-
HCM Lane V/C Ratio		-	-	0.418	0.126	-
HCM Control Delay (s)		-	-	27	11.058	0
HCM Lane LOS				D	B	A
HCM 95th %tile Q(veh)		-	-	1.967	0.431	-
Notes						
~ : Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined						

CDM SMITH Inc.
 1100 Marion Street, Suite 200
 Knoxville, TN 37921
 (865) 963-4300

Counted by: Allyson Foster

File Name : byington at garrison total
 Site Code : 101
 Start Date : 10/29/2013
 Page No : 1

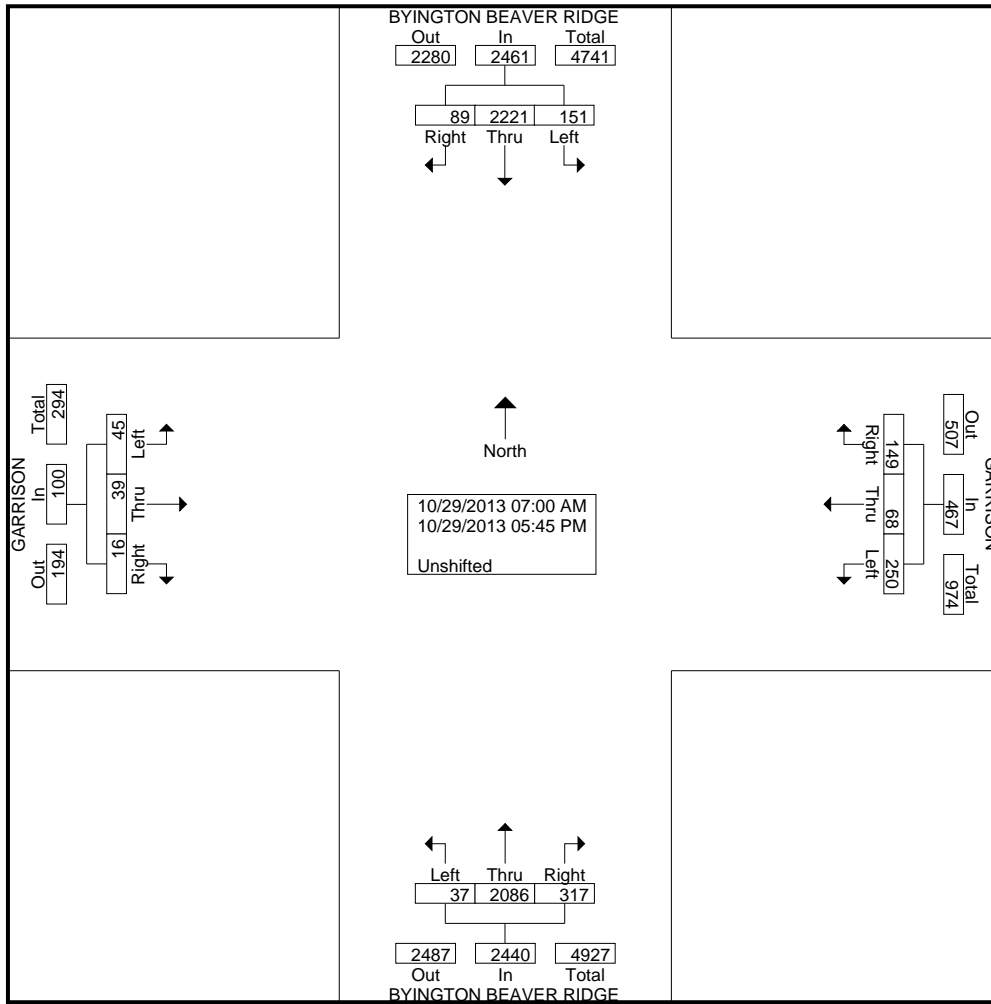
Groups Printed- Unshifted

Start Time	BYINGTON BEAVER RIDGE Southbound				GARRISON Westbound				BYINGTON BEAVER RIDGE Northbound				GARRISON Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	59	9	68	2	17	0	19	3	95	1	99	7	8	8	23	209
07:15 AM	9	145	3	157	24	0	13	37	0	107	19	126	1	0	0	1	321
07:30 AM	7	126	2	135	26	7	12	45	1	97	30	128	1	2	1	4	312
07:45 AM	12	152	3	167	31	11	12	54	0	65	31	96	1	1	0	2	319
Total	28	482	17	527	83	35	37	155	4	364	81	449	10	11	9	30	1161
08:00 AM	8	120	19	147	32	11	8	51	3	70	37	110	0	2	0	2	310
08:15 AM	5	107	11	123	20	7	13	40	1	69	27	97	0	1	0	1	261
08:30 AM	2	110	2	114	14	1	0	15	0	36	4	40	3	2	1	6	175
08:45 AM	1	93	1	95	1	0	2	3	0	39	3	42	0	0	0	0	140
Total	16	430	33	479	67	19	23	109	4	214	71	289	3	5	1	9	886
*** BREAK ***																	
02:00 PM	5	72	1	78	1	0	2	3	0	79	2	81	6	0	0	6	168
02:15 PM	4	62	1	67	4	3	1	8	0	80	6	86	0	0	1	1	162
02:30 PM	3	65	1	69	4	2	3	9	3	82	10	95	1	0	0	1	174
02:45 PM	6	100	1	107	4	0	4	8	1	64	8	73	3	2	1	6	194
Total	18	299	4	321	13	5	10	28	4	305	26	335	10	2	2	14	698
03:00 PM	5	97	3	105	3	0	0	3	0	75	11	86	2	3	0	5	199
03:15 PM	10	84	5	99	6	1	7	14	0	78	13	91	1	0	0	1	205
03:30 PM	3	103	3	109	32	2	36	70	2	125	18	145	4	3	0	7	331
03:45 PM	5	84	5	94	11	1	12	24	2	112	10	124	5	5	0	10	252
Total	23	368	16	407	52	4	55	111	4	390	52	446	12	11	0	23	987
04:00 PM	6	78	5	89	2	1	3	6	10	85	2	97	2	0	2	4	196
04:15 PM	0	40	1	41	1	0	6	7	1	59	1	61	1	1	0	2	111
04:30 PM	4	42	0	46	5	0	0	5	0	55	0	55	0	0	0	0	106
04:45 PM	9	81	3	93	2	0	2	4	1	102	14	117	0	1	1	2	216
Total	19	241	9	269	10	1	11	22	12	301	17	330	3	2	3	8	629
05:00 PM	10	99	3	112	11	2	8	21	1	133	12	146	1	3	0	4	283
05:15 PM	16	96	3	115	2	1	1	4	4	140	22	166	1	1	0	2	287
05:30 PM	9	107	3	119	4	1	4	9	1	135	18	154	3	2	1	6	288
05:45 PM	12	99	1	112	8	0	0	8	3	104	18	125	2	2	0	4	249
Total	47	401	10	458	25	4	13	42	9	512	70	591	7	8	1	16	1107
Grand Total	151	2221	89	2461	250	68	149	467	37	2086	317	2440	45	39	16	100	5468
Apprch %	6.1	90.2	3.6		53.5	14.6	31.9		1.5	85.5	13		45	39	16		
Total %	2.8	40.6	1.6	45	4.6	1.2	2.7	8.5	0.7	38.1	5.8	44.6	0.8	0.7	0.3	1.8	

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 Site Code : 101
 Start Date : 10/29/2013
 Page No : 2



Start Time	BYINGTON BEAVER RIDGE Southbound				GARRISON Westbound				BYINGTON BEAVER RIDGE Northbound				GARRISON Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 07:00 AM to 11:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	9	145	3	157	24	0	13	37	0	107	19	126	1	0	0	1	321
07:30 AM	7	126	2	135	26	7	12	45	1	97	30	128	1	2	1	4	312
07:45 AM	12	152	3	167	31	11	12	54	0	65	31	96	1	1	0	2	319
08:00 AM	8	120	19	147	32	11	8	51	3	70	37	110	0	2	0	2	310
Total Volume	36	543	27	606	113	29	45	187	4	339	117	460	3	5	1	9	1262
% App. Total	5.9	89.6	4.5		60.4	15.5	24.1		0.9	73.7	25.4		33.3	55.6	11.1		
PHF	.750	.893	.355	.907	.883	.659	.865	.866	.333	.792	.791	.898	.750	.625	.250	.563	.983

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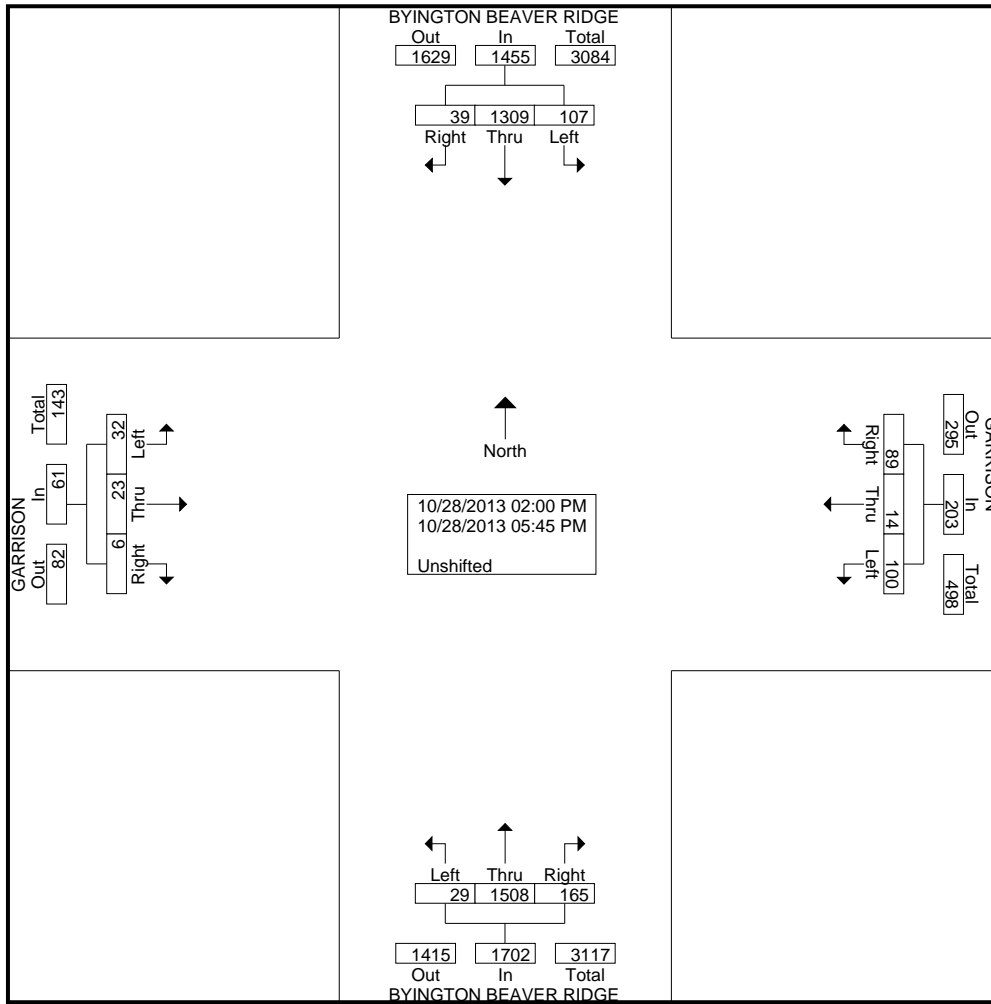
File Name : byington at garrison total
 Site Code : 101
 Start Date : 10/29/2013
 Page No : 3

Start Time	BYINGTON BEAVER RIDGE Southbound				GARRISON Westbound				BYINGTON BEAVER RIDGE Northbound				GARRISON Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 12:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	10	99	3	112	11	2	8	21	1	133	12	146	1	3	0	4	283
05:15 PM	16	96	3	115	2	1	1	4	4	140	22	166	1	1	0	2	287
05:30 PM	9	107	3	119	4	1	4	9	1	135	18	154	3	2	1	6	288
05:45 PM	12	99	1	112	8	0	0	8	3	104	18	125	2	2	0	4	249
Total Volume	47	401	10	458	25	4	13	42	9	512	70	591	7	8	1	16	1107
% App. Total	10.3	87.6	2.2		59.5	9.5	31		1.5	86.6	11.8		43.8	50	6.2		
PHF	.734	.937	.833	.962	.568	.500	.406	.500	.563	.914	.795	.890	.583	.667	.250	.667	.961

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Counted by: Allyson Foster

File Name : Byington at Garrison PM
 Site Code : 101
 Start Date : 10/28/2013
 Page No : 2



CDM SMITH Inc.
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Counted by: Allyson Foster

File Name : Garrison Stop Delay

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/29/2013

Lane 2- WB Garrison Drive

Page No : 1

L n.	No.	Joined Queue	Released From Queue	Delay
1	1	6:56:52 AM	6:56:54 AM	2
1	2	6:57:00 AM	6:57:05 AM	5
1	3	7:07:19 AM	7:07:27 AM	8
1	4	7:10:05 AM	7:10:33 AM	28
1	5	7:11:36 AM	7:12:12 AM	36
1	6	7:19:07 AM	7:19:26 AM	19
1	7	7:21:50 AM	7:23:08 AM	78
1	8	7:27:21 AM	7:27:56 AM	35
1	9	7:27:41 AM	7:28:20 AM	39
1	10	7:27:42 AM	7:28:42 AM	60
1	11	7:33:00 AM	7:33:09 AM	9
1	12	7:34:26 AM	7:34:52 AM	26
1	13	7:35:04 AM	7:35:31 AM	27
1	14	7:39:28 AM	7:39:46 AM	18
1	15	7:41:49 AM	7:42:05 AM	16
1	16	7:46:50 AM	7:47:13 AM	23
1	17	7:52:00 AM	7:52:21 AM	21
1	18	8:09:28 AM	8:09:46 AM	18
1	19	8:11:55 AM	8:12:59 AM	64
1	20	8:12:54 AM	8:13:04 AM	10
1	21	8:21:06 AM	8:21:34 AM	28
1	22	8:23:42 AM	8:24:08 AM	26
1	23	8:25:09 AM	8:25:58 AM	49
1	24	8:26:30 AM	8:26:49 AM	19
1	25	8:27:08 AM	8:27:15 AM	7
1	26	8:29:04 AM	8:29:07 AM	3
1	27	8:30:09 AM	8:30:45 AM	36
1	28	8:30:10 AM	8:30:47 AM	37
1	29	8:31:48 AM	8:31:53 AM	5
1	30	8:32:52 AM	8:32:58 AM	6
1	31	8:37:07 AM	8:37:23 AM	16
1	32	8:37:57 AM	8:38:32 AM	35
1	33	8:54:10 AM	8:54:18 AM	8
2	1	6:56:00 AM	6:56:04 AM	4
2	2	7:01:43 AM	7:01:45 AM	2
2	3	7:03:18 AM	7:03:23 AM	5
2	4	7:05:50 AM	7:06:15 AM	25
2	5	7:06:36 AM	7:06:49 AM	13
2	6	7:08:16 AM	7:08:20 AM	4
2	7	7:08:18 AM	7:08:37 AM	19
2	8	7:10:09 AM	7:10:13 AM	4
2	9	7:10:17 AM	7:10:22 AM	5
2	10	7:12:17 AM	7:12:23 AM	6
2	11	7:13:31 AM	7:13:34 AM	3
2	12	7:14:02 AM	7:14:03 AM	1
2	13	7:16:50 AM	7:17:11 AM	21
2	14	7:18:11 AM	7:18:21 AM	10
2	15	7:18:46 AM	7:18:52 AM	6
2	16	7:18:59 AM	7:19:25 AM	26
2	17	7:19:32 AM	7:19:44 AM	12
2	18	7:19:49 AM	7:19:51 AM	2
2	19	7:20:12 AM	7:20:14 AM	2
2	20	7:20:26 AM	7:20:28 AM	2
2	21	7:20:58 AM	7:21:03 AM	5
2	22	7:21:10 AM	7:21:14 AM	4
2	23	7:21:18 AM	7:21:21 AM	3
2	24	7:21:36 AM	7:22:41 AM	65
2	25	7:22:18 AM	7:22:45 AM	27
2	26	7:22:19 AM	7:23:15 AM	56
2	27	7:23:10 AM	7:23:33 AM	23
2	28	7:23:12 AM	7:23:37 AM	25

CDM SMITH Inc.
 1100 Marion Street, Suite 200
 Knoxville, TN 37921
 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/29/2013

Lane 2- WB Garrison Drive

Page No : 2

L n.	No.	Joined Queue	Released From Queue	Delay
2	29	7:23:25 AM	7:23:59 AM	34
2	30	7:24:23 AM	7:24:25 AM	2
2	31	7:25:02 AM	7:25:05 AM	3
2	32	7:25:09 AM	7:25:20 AM	11
2	33	7:25:12 AM	7:25:27 AM	15
2	34	7:25:39 AM	7:25:49 AM	10
2	35	7:25:44 AM	7:26:13 AM	29
2	36	7:26:06 AM	7:26:22 AM	16
2	37	7:26:27 AM	7:26:32 AM	5
2	38	7:26:35 AM	7:26:43 AM	8
2	39	7:26:38 AM	7:26:48 AM	10
2	40	7:27:00 AM	7:27:56 AM	56
2	41	7:27:25 AM	7:28:00 AM	35
2	42	7:27:27 AM	7:28:03 AM	36
2	43	7:27:34 AM	7:28:07 AM	33
2	44	7:27:48 AM	7:28:18 AM	30
2	45	7:27:50 AM	7:28:43 AM	53
2	46	7:28:06 AM	7:29:01 AM	55
2	47	7:28:13 AM	7:29:06 AM	53
2	48	7:29:13 AM	7:29:17 AM	4
2	49	7:29:37 AM	7:30:07 AM	30
2	50	7:29:46 AM	7:30:23 AM	37
2	51	7:31:07 AM	7:31:20 AM	13
2	52	7:31:08 AM	7:31:26 AM	18
2	53	7:31:50 AM	7:31:54 AM	4
2	54	7:32:01 AM	7:32:08 AM	7
2	55	7:33:08 AM	7:33:25 AM	17
2	56	7:33:17 AM	7:33:34 AM	17
2	57	7:33:20 AM	7:34:09 AM	49
2	58	7:33:52 AM	7:34:17 AM	25
2	59	7:33:53 AM	7:34:23 AM	30
2	60	7:34:12 AM	7:34:27 AM	15
2	61	7:34:13 AM	7:34:33 AM	20
2	62	7:34:29 AM	7:34:53 AM	24
2	63	7:34:51 AM	7:34:56 AM	5
2	64	7:35:40 AM	7:35:46 AM	6
2	65	7:35:45 AM	7:35:58 AM	13
2	66	7:35:53 AM	7:36:06 AM	13
2	67	7:35:55 AM	7:36:32 AM	37
2	68	7:36:08 AM	7:36:37 AM	29
2	69	7:36:24 AM	7:36:47 AM	23
2	70	7:36:49 AM	7:37:23 AM	34
2	71	7:36:57 AM	7:37:34 AM	37
2	72	7:37:02 AM	7:37:45 AM	43
2	73	7:37:04 AM	7:37:49 AM	45
2	74	7:37:07 AM	7:37:52 AM	45
2	75	7:37:17 AM	7:38:02 AM	45
2	76	7:37:22 AM	7:38:47 AM	85
2	77	7:38:22 AM	7:38:49 AM	27
2	78	7:38:41 AM	7:38:56 AM	15
2	79	7:39:23 AM	7:39:30 AM	7
2	80	7:39:27 AM	7:39:48 AM	21
2	81	7:39:43 AM	7:39:53 AM	10
2	82	7:40:20 AM	7:40:21 AM	1
2	83	7:41:41 AM	7:41:55 AM	14
2	84	7:42:09 AM	7:42:35 AM	26
2	85	7:42:10 AM	7:43:41 AM	91
2	86	7:42:36 AM	7:43:53 AM	77
2	87	7:42:38 AM	7:43:59 AM	81
2	88	7:42:51 AM	7:44:07 AM	76
2	89	7:42:55 AM	7:44:12 AM	77

CDM SMITH Inc.
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 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/29/2013

Lane 2- WB Garrison Drive

Page No : 3

L n.	No.	Joined Queue	Released From Queue	Delay
2	90	7:43:06 AM	7:44:16 AM	70
2	91	7:43:20 AM	7:44:21 AM	61
2	92	7:43:43 AM	7:44:25 AM	42
2	93	7:43:46 AM	7:44:30 AM	44
2	94	7:44:11 AM	7:44:39 AM	28
2	95	7:44:18 AM	7:45:08 AM	50
2	96	7:44:57 AM	7:45:17 AM	20
2	97	7:45:23 AM	7:46:03 AM	40
2	98	7:45:38 AM	7:46:08 AM	30
2	99	7:45:47 AM	7:46:15 AM	28
2	100	7:45:58 AM	7:47:40 AM	102
2	101	7:45:59 AM	7:47:56 AM	117
2	102	7:46:04 AM	7:48:05 AM	121
2	103	7:46:05 AM	7:48:19 AM	134
2	104	7:46:16 AM	7:48:41 AM	145
2	105	7:46:46 AM	7:48:58 AM	132
2	106	7:47:00 AM	7:49:04 AM	124
2	107	7:47:03 AM	7:49:07 AM	124
2	108	7:47:08 AM	7:49:10 AM	122
2	109	7:47:18 AM	7:49:14 AM	116
2	110	7:47:54 AM	7:49:17 AM	83
2	111	7:48:22 AM	7:49:20 AM	58
2	112	7:48:24 AM	7:49:25 AM	61
2	113	7:48:32 AM	7:49:47 AM	75
2	114	7:48:48 AM	7:49:57 AM	69
2	115	7:48:56 AM	7:50:23 AM	87
2	116	7:49:12 AM	7:50:30 AM	78
2	117	7:49:45 AM	7:50:36 AM	51
2	118	7:49:49 AM	7:51:05 AM	76
2	119	7:50:05 AM	7:51:52 AM	107
2	120	7:50:09 AM	7:51:57 AM	108
2	121	7:50:27 AM	7:52:16 AM	109
2	122	7:50:27 AM	7:52:33 AM	126
2	123	7:50:40 AM	7:52:44 AM	124
2	124	7:50:58 AM	7:53:04 AM	126
2	125	7:50:58 AM	7:53:08 AM	130
2	126	7:51:12 AM	7:53:12 AM	120
2	127	7:51:21 AM	7:53:16 AM	115
2	128	7:51:39 AM	7:53:21 AM	102
2	129	7:51:45 AM	7:53:32 AM	107
2	130	7:51:55 AM	7:54:19 AM	144
2	131	7:52:46 AM	7:54:22 AM	96
2	132	7:52:52 AM	7:54:27 AM	95
2	133	7:53:23 AM	7:54:32 AM	69
2	134	7:53:26 AM	7:54:43 AM	77
2	135	7:53:39 AM	7:54:53 AM	74
2	136	7:53:39 AM	7:55:06 AM	87
2	137	7:53:40 AM	7:55:11 AM	91
2	138	7:53:55 AM	7:55:44 AM	109
2	139	7:54:10 AM	7:56:02 AM	112
2	140	7:54:15 AM	7:56:29 AM	134
2	141	7:54:21 AM	7:56:32 AM	131
2	142	7:54:34 AM	7:56:53 AM	139
2	143	7:54:38 AM	7:57:09 AM	151
2	144	7:54:51 AM	7:57:25 AM	154
2	145	7:55:30 AM	7:57:28 AM	118
2	146	7:55:30 AM	7:57:34 AM	124
2	147	7:55:31 AM	7:58:04 AM	153
2	148	7:55:45 AM	7:58:07 AM	142
2	149	7:56:10 AM	7:58:10 AM	120
2	150	7:56:16 AM	7:58:13 AM	117

CDM SMITH Inc.
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 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/29/2013

Lane 2- WB Garrison Drive

Page No : 4

L n.	No.	Joined Queue	Released From Queue	Delay
2	151	7:56:20 AM	7:58:23 AM	123
2	152	7:57:11 AM	7:58:30 AM	79
2	153	7:57:37 AM	7:58:45 AM	68
2	154	7:57:42 AM	7:58:55 AM	73
2	155	7:57:45 AM	7:59:56 AM	131
2	156	7:57:58 AM	8:00:03 AM	125
2	157	7:58:10 AM	8:00:11 AM	121
2	158	7:58:25 AM	8:00:55 AM	150
2	159	7:59:02 AM	8:01:11 AM	129
2	160	7:59:33 AM	8:01:19 AM	106
2	161	7:59:35 AM	8:01:27 AM	112
2	162	7:59:44 AM	8:01:40 AM	116
2	163	7:59:44 AM	8:02:00 AM	136
2	164	7:59:54 AM	8:02:29 AM	155
2	165	8:00:10 AM	8:02:37 AM	147
2	166	8:00:47 AM	8:03:03 AM	136
2	167	8:00:52 AM	8:03:28 AM	156
2	168	8:01:08 AM	8:03:44 AM	156
2	169	8:01:14 AM	8:03:49 AM	155
2	170	8:01:49 AM	8:03:51 AM	122
2	171	8:02:34 AM	8:03:56 AM	82
2	172	8:02:59 AM	8:04:00 AM	61
2	173	8:03:00 AM	8:04:20 AM	80
2	174	8:03:00 AM	8:04:23 AM	83
2	175	8:03:05 AM	8:05:07 AM	122
2	176	8:03:18 AM	8:05:13 AM	115
2	177	8:03:19 AM	8:06:07 AM	168
2	178	8:03:20 AM	8:06:17 AM	177
2	179	8:04:18 AM	8:06:21 AM	123
2	180	8:04:28 AM	8:06:25 AM	117
2	181	8:04:41 AM	8:06:44 AM	123
2	182	8:04:43 AM	8:06:47 AM	124
2	183	8:04:47 AM	8:06:49 AM	122
2	184	8:05:31 AM	8:07:08 AM	97
2	185	8:05:31 AM	8:07:26 AM	115
2	186	8:05:32 AM	8:07:44 AM	132
2	187	8:06:09 AM	8:07:57 AM	108
2	188	8:06:31 AM	8:08:04 AM	93
2	189	8:06:54 AM	8:08:26 AM	92
2	190	8:06:55 AM	8:08:53 AM	118
2	191	8:06:55 AM	8:08:59 AM	124
2	192	8:07:01 AM	8:09:09 AM	128
2	193	8:07:50 AM	8:09:12 AM	82
2	194	8:07:55 AM	8:09:22 AM	87
2	195	8:08:14 AM	8:09:25 AM	71
2	196	8:08:15 AM	8:09:32 AM	77
2	197	8:08:21 AM	8:09:47 AM	86
2	198	8:08:22 AM	8:09:50 AM	88
2	199	8:08:52 AM	8:09:53 AM	61
2	200	8:09:04 AM	8:10:17 AM	73
2	201	8:09:11 AM	8:10:47 AM	96
2	202	8:09:11 AM	8:10:52 AM	101
2	203	8:09:29 AM	8:11:10 AM	101
2	204	8:10:11 AM	8:11:14 AM	63
2	205	8:10:12 AM	8:11:40 AM	88
2	206	8:10:16 AM	8:12:53 AM	157
2	207	8:10:20 AM	8:12:58 AM	158
2	208	8:10:31 AM	8:13:01 AM	150
2	209	8:10:32 AM	8:13:14 AM	162
2	210	8:10:36 AM	8:13:52 AM	196
2	211	8:11:17 AM	8:13:56 AM	159

CDM SMITH Inc.
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 Knoxville, TN 37921
 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/29/2013

Lane 2- WB Garrison Drive

Page No : 5

L n.	No.	Joined Queue	Released From Queue	Delay
2	212	8:11:24 AM	8:14:39 AM	195
2	213	8:11:28 AM	8:14:55 AM	207
2	214	8:12:05 AM	8:14:57 AM	172
2	215	8:13:10 AM	8:15:04 AM	114
2	216	8:13:11 AM	8:15:09 AM	118
2	217	8:13:12 AM	8:15:26 AM	134
2	218	8:13:19 AM	8:15:29 AM	130
2	219	8:13:20 AM	8:15:52 AM	152
2	220	8:13:23 AM	8:16:47 AM	204
2	221	8:14:25 AM	8:16:50 AM	145
2	222	8:14:25 AM	8:16:54 AM	149
2	223	8:14:26 AM	8:16:57 AM	151
2	224	8:14:26 AM	8:17:21 AM	175
2	225	8:14:27 AM	8:17:42 AM	195
2	226	8:15:20 AM	8:17:45 AM	145
2	227	8:15:41 AM	8:17:51 AM	130
2	228	8:15:42 AM	8:17:56 AM	134
2	229	8:16:29 AM	8:18:02 AM	93
2	230	8:16:30 AM	8:18:04 AM	94
2	231	8:16:42 AM	8:18:07 AM	85
2	232	8:16:56 AM	8:18:09 AM	73
2	233	8:17:26 AM	8:18:16 AM	50
2	234	8:17:27 AM	8:18:19 AM	52
2	235	8:17:42 AM	8:18:27 AM	45
2	236	8:17:42 AM	8:19:07 AM	85
2	237	8:17:58 AM	8:19:15 AM	77
2	238	8:18:14 AM	8:19:21 AM	67
2	239	8:18:49 AM	8:19:58 AM	69
2	240	8:19:25 AM	8:20:03 AM	38
2	241	8:19:26 AM	8:20:06 AM	40
2	242	8:20:46 AM	8:21:10 AM	24
2	243	8:20:51 AM	8:21:23 AM	32
2	244	8:21:16 AM	8:21:28 AM	12
2	245	8:22:34 AM	8:22:40 AM	6
2	246	8:23:03 AM	8:23:14 AM	11
2	247	8:23:18 AM	8:23:23 AM	5
2	248	8:23:35 AM	8:23:44 AM	9
2	249	8:24:13 AM	8:24:18 AM	5
2	250	8:24:29 AM	8:24:34 AM	5
2	251	8:24:59 AM	8:25:16 AM	17
2	252	8:25:13 AM	8:25:31 AM	18
2	253	8:25:17 AM	8:25:55 AM	38
2	254	8:25:32 AM	8:26:51 AM	79
2	255	8:25:33 AM	8:27:03 AM	90
2	256	8:25:49 AM	8:27:07 AM	78
2	257	8:26:00 AM	8:27:41 AM	101
2	258	8:26:39 AM	8:27:44 AM	65
2	259	8:27:34 AM	8:28:17 AM	43
2	260	8:28:25 AM	8:28:29 AM	4
2	261	8:28:50 AM	8:28:58 AM	8
2	262	8:29:47 AM	8:30:02 AM	15
2	263	8:29:48 AM	8:30:07 AM	19
2	264	8:30:00 AM	8:30:08 AM	8
2	265	8:31:11 AM	8:31:18 AM	7
2	266	8:31:39 AM	8:31:43 AM	4
2	267	8:33:37 AM	8:33:53 AM	16
2	268	8:33:39 AM	8:34:04 AM	25
2	269	8:33:41 AM	8:34:10 AM	29
2	270	8:33:45 AM	8:34:12 AM	27
2	271	8:34:08 AM	8:34:16 AM	8
2	272	8:35:32 AM	8:35:34 AM	2

CDM SMITH Inc.
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 (865) 963-4300

Counted by: Allyson Foster

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

File Name : Garrison Stop Delay
 Site Code : 101
 Start Date : 10/29/2013
 Page No : 6

L n.	No.	Joined Queue	Released From Queue	Delay
2	273	8:36:16 AM	8:36:19 AM	3
2	274	8:37:20 AM	8:37:25 AM	5
2	275	8:37:54 AM	8:37:59 AM	5
2	276	8:39:13 AM	8:39:44 AM	31
2	277	8:39:38 AM	8:39:55 AM	17
2	278	8:40:03 AM	8:40:06 AM	3
2	279	8:42:37 AM	8:42:47 AM	10
2	280	8:42:51 AM	8:43:02 AM	11
2	281	8:45:47 AM	8:46:06 AM	19
2	282	8:47:10 AM	8:47:13 AM	3
2	283	8:47:40 AM	8:47:42 AM	2
2	284	8:48:40 AM	8:48:41 AM	1
2	285	8:54:36 AM	8:54:39 AM	3

Summary Information:

6:56:00 AM - 8:55:00 AM	Lane 1	Lane 2
Total Vehicle Count:	33	285
Delayed Vehicle Count:	33	285
Through Vehicle Count:	0	0
Average Stopped Time:	24.76	68.993
Maximum Stopped Time:	78	207
Min. Secs. for Delay:	0	0
Average Queue:	0.12	2.762
Queue Density:	1.13	5.307
Maximum Queue:	3	14
Delay in Vehicle Hour:	0.12	2.76
Total Delay:	817	19663

Counted by: Allyson Foster

File Name : Garrison Stop Delay AM
 Site Code : 101
 Start Date : 10/29/2013
 Page No : 1

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

L n.	No.	Joined Queue	Released From Queue	Delay
1	6	7:19:07 AM	7:19:26 AM	19
1	7	7:21:50 AM	7:23:08 AM	78
1	8	7:27:21 AM	7:27:56 AM	35
1	9	7:27:41 AM	7:28:20 AM	39
1	10	7:27:42 AM	7:28:42 AM	60
1	11	7:33:00 AM	7:33:09 AM	9
1	12	7:34:26 AM	7:34:52 AM	26
1	13	7:35:04 AM	7:35:31 AM	27
1	14	7:39:28 AM	7:39:46 AM	18
1	15	7:41:49 AM	7:42:05 AM	16
1	16	7:46:50 AM	7:47:13 AM	23
1	17	7:52:00 AM	7:52:21 AM	21
1	18	8:09:28 AM	8:09:46 AM	18
1	19	8:11:55 AM	8:12:59 AM	64
1	20	8:12:54 AM	8:13:04 AM	10
2	13	7:16:50 AM	7:17:11 AM	21
2	14	7:18:11 AM	7:18:21 AM	10
2	15	7:18:46 AM	7:18:52 AM	6
2	16	7:18:59 AM	7:19:25 AM	26
2	17	7:19:32 AM	7:19:44 AM	12
2	18	7:19:49 AM	7:19:51 AM	2
2	19	7:20:12 AM	7:20:14 AM	2
2	20	7:20:26 AM	7:20:28 AM	2
2	21	7:20:58 AM	7:21:03 AM	5
2	22	7:21:10 AM	7:21:14 AM	4
2	23	7:21:18 AM	7:21:21 AM	3
2	24	7:21:36 AM	7:22:41 AM	65
2	25	7:22:18 AM	7:22:45 AM	27
2	26	7:22:19 AM	7:23:15 AM	56
2	27	7:23:10 AM	7:23:33 AM	23
2	28	7:23:12 AM	7:23:37 AM	25
2	29	7:23:25 AM	7:23:59 AM	34
2	30	7:24:23 AM	7:24:25 AM	2
2	31	7:25:02 AM	7:25:05 AM	3
2	32	7:25:09 AM	7:25:20 AM	11
2	33	7:25:12 AM	7:25:27 AM	15
2	34	7:25:39 AM	7:25:49 AM	10
2	35	7:25:44 AM	7:26:13 AM	29
2	36	7:26:06 AM	7:26:22 AM	16
2	37	7:26:27 AM	7:26:32 AM	5
2	38	7:26:35 AM	7:26:43 AM	8
2	39	7:26:38 AM	7:26:48 AM	10
2	40	7:27:00 AM	7:27:56 AM	56
2	41	7:27:25 AM	7:28:00 AM	35
2	42	7:27:27 AM	7:28:03 AM	36
2	43	7:27:34 AM	7:28:07 AM	33
2	44	7:27:48 AM	7:28:18 AM	30
2	45	7:27:50 AM	7:28:43 AM	53
2	46	7:28:06 AM	7:29:01 AM	55
2	47	7:28:13 AM	7:29:06 AM	53
2	48	7:29:13 AM	7:29:17 AM	4
2	49	7:29:37 AM	7:30:07 AM	30
2	50	7:29:46 AM	7:30:23 AM	37
2	51	7:31:07 AM	7:31:20 AM	13
2	52	7:31:08 AM	7:31:26 AM	18
2	53	7:31:50 AM	7:31:54 AM	4
2	54	7:32:01 AM	7:32:08 AM	7
2	55	7:33:08 AM	7:33:25 AM	17
2	56	7:33:17 AM	7:33:34 AM	17
2	57	7:33:20 AM	7:34:09 AM	49
2	58	7:33:52 AM	7:34:17 AM	25
2	59	7:33:53 AM	7:34:23 AM	30
2	60	7:34:12 AM	7:34:27 AM	15

Counted by: Allyson Foster

File Name : Garrison Stop Delay AM

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

Site Code : 101

Start Date : 10/29/2013

Page No : 2

L n.	No.	Joined Queue	Released From Queue	Delay
2	61	7:34:13 AM	7:34:33 AM	20
2	62	7:34:29 AM	7:34:53 AM	24
2	63	7:34:51 AM	7:34:56 AM	5
2	64	7:35:40 AM	7:35:46 AM	6
2	65	7:35:45 AM	7:35:58 AM	13
2	66	7:35:53 AM	7:36:06 AM	13
2	67	7:35:55 AM	7:36:32 AM	37
2	68	7:36:08 AM	7:36:37 AM	29
2	69	7:36:24 AM	7:36:47 AM	23
2	70	7:36:49 AM	7:37:23 AM	34
2	71	7:36:57 AM	7:37:34 AM	37
2	72	7:37:02 AM	7:37:45 AM	43
2	73	7:37:04 AM	7:37:49 AM	45
2	74	7:37:07 AM	7:37:52 AM	45
2	75	7:37:17 AM	7:38:02 AM	45
2	76	7:37:22 AM	7:38:47 AM	85
2	77	7:38:22 AM	7:38:49 AM	27
2	78	7:38:41 AM	7:38:56 AM	15
2	79	7:39:23 AM	7:39:30 AM	7
2	80	7:39:27 AM	7:39:48 AM	21
2	81	7:39:43 AM	7:39:53 AM	10
2	82	7:40:20 AM	7:40:21 AM	1
2	83	7:41:41 AM	7:41:55 AM	14
2	84	7:42:09 AM	7:42:35 AM	26
2	85	7:42:10 AM	7:43:41 AM	91
2	86	7:42:36 AM	7:43:53 AM	77
2	87	7:42:38 AM	7:43:59 AM	81
2	88	7:42:51 AM	7:44:07 AM	76
2	89	7:42:55 AM	7:44:12 AM	77
2	90	7:43:06 AM	7:44:16 AM	70
2	91	7:43:20 AM	7:44:21 AM	61
2	92	7:43:43 AM	7:44:25 AM	42
2	93	7:43:46 AM	7:44:30 AM	44
2	94	7:44:11 AM	7:44:39 AM	28
2	95	7:44:18 AM	7:45:08 AM	50
2	96	7:44:57 AM	7:45:17 AM	20
2	97	7:45:23 AM	7:46:03 AM	40
2	98	7:45:38 AM	7:46:08 AM	30
2	99	7:45:47 AM	7:46:15 AM	28
2	100	7:45:58 AM	7:47:40 AM	102
2	101	7:45:59 AM	7:47:56 AM	117
2	102	7:46:04 AM	7:48:05 AM	121
2	103	7:46:05 AM	7:48:19 AM	134
2	104	7:46:16 AM	7:48:41 AM	145
2	105	7:46:46 AM	7:48:58 AM	132
2	106	7:47:00 AM	7:49:04 AM	124
2	107	7:47:03 AM	7:49:07 AM	124
2	108	7:47:08 AM	7:49:10 AM	122
2	109	7:47:18 AM	7:49:14 AM	116
2	110	7:47:54 AM	7:49:17 AM	83
2	111	7:48:22 AM	7:49:20 AM	58
2	112	7:48:24 AM	7:49:25 AM	61
2	113	7:48:32 AM	7:49:47 AM	75
2	114	7:48:48 AM	7:49:57 AM	69
2	115	7:48:56 AM	7:50:23 AM	87
2	116	7:49:12 AM	7:50:30 AM	78
2	117	7:49:45 AM	7:50:36 AM	51
2	118	7:49:49 AM	7:51:05 AM	76
2	119	7:50:05 AM	7:51:52 AM	107
2	120	7:50:09 AM	7:51:57 AM	108
2	121	7:50:27 AM	7:52:16 AM	109
2	122	7:50:27 AM	7:52:33 AM	126
2	123	7:50:40 AM	7:52:44 AM	124
2	124	7:50:58 AM	7:53:04 AM	126

Counted by: Allyson Foster

File Name : Garrison Stop Delay AM

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

Site Code : 101

Start Date : 10/29/2013

Page No : 3

L n.	No.	Joined Queue	Released From Queue	Delay
2	125	7:50:58 AM	7:53:08 AM	130
2	126	7:51:12 AM	7:53:12 AM	120
2	127	7:51:21 AM	7:53:16 AM	115
2	128	7:51:39 AM	7:53:21 AM	102
2	129	7:51:45 AM	7:53:32 AM	107
2	130	7:51:55 AM	7:54:19 AM	144
2	131	7:52:46 AM	7:54:22 AM	96
2	132	7:52:52 AM	7:54:27 AM	95
2	133	7:53:23 AM	7:54:32 AM	69
2	134	7:53:26 AM	7:54:43 AM	77
2	135	7:53:39 AM	7:54:53 AM	74
2	136	7:53:39 AM	7:55:06 AM	87
2	137	7:53:40 AM	7:55:11 AM	91
2	138	7:53:55 AM	7:55:44 AM	109
2	139	7:54:10 AM	7:56:02 AM	112
2	140	7:54:15 AM	7:56:29 AM	134
2	141	7:54:21 AM	7:56:32 AM	131
2	142	7:54:34 AM	7:56:53 AM	139
2	143	7:54:38 AM	7:57:09 AM	151
2	144	7:54:51 AM	7:57:25 AM	154
2	145	7:55:30 AM	7:57:28 AM	118
2	146	7:55:30 AM	7:57:34 AM	124
2	147	7:55:31 AM	7:58:04 AM	153
2	148	7:55:45 AM	7:58:07 AM	142
2	149	7:56:10 AM	7:58:10 AM	120
2	150	7:56:16 AM	7:58:13 AM	117
2	151	7:56:20 AM	7:58:23 AM	123
2	152	7:57:11 AM	7:58:30 AM	79
2	153	7:57:37 AM	7:58:45 AM	68
2	154	7:57:42 AM	7:58:55 AM	73
2	155	7:57:45 AM	7:59:56 AM	131
2	156	7:57:58 AM	8:00:03 AM	125
2	157	7:58:10 AM	8:00:11 AM	121
2	158	7:58:25 AM	8:00:55 AM	150
2	159	7:59:02 AM	8:01:11 AM	129
2	160	7:59:33 AM	8:01:19 AM	106
2	161	7:59:35 AM	8:01:27 AM	112
2	162	7:59:44 AM	8:01:40 AM	116
2	163	7:59:44 AM	8:02:00 AM	136
2	164	7:59:54 AM	8:02:29 AM	155
2	165	8:00:10 AM	8:02:37 AM	147
2	166	8:00:47 AM	8:03:03 AM	136
2	167	8:00:52 AM	8:03:28 AM	156
2	168	8:01:08 AM	8:03:44 AM	156
2	169	8:01:14 AM	8:03:49 AM	155
2	170	8:01:49 AM	8:03:51 AM	122
2	171	8:02:34 AM	8:03:56 AM	82
2	172	8:02:59 AM	8:04:00 AM	61
2	173	8:03:00 AM	8:04:20 AM	80
2	174	8:03:00 AM	8:04:23 AM	83
2	175	8:03:05 AM	8:05:07 AM	122
2	176	8:03:18 AM	8:05:13 AM	115
2	177	8:03:19 AM	8:06:07 AM	168
2	178	8:03:20 AM	8:06:17 AM	177
2	179	8:04:18 AM	8:06:21 AM	123
2	180	8:04:28 AM	8:06:25 AM	117
2	181	8:04:41 AM	8:06:44 AM	123
2	182	8:04:43 AM	8:06:47 AM	124
2	183	8:04:47 AM	8:06:49 AM	122
2	184	8:05:31 AM	8:07:08 AM	97
2	185	8:05:31 AM	8:07:26 AM	115
2	186	8:05:32 AM	8:07:44 AM	132
2	187	8:06:09 AM	8:07:57 AM	108
2	188	8:06:31 AM	8:08:04 AM	93

Counted by: Allyson Foster

File Name : Garrison Stop Delay AM
 Site Code : 101
 Start Date : 10/29/2013
 Page No : 4

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

L n.	No.	Joined Queue	Released From Queue	Delay
2	189	8:06:54 AM	8:08:26 AM	92
2	190	8:06:55 AM	8:08:53 AM	118
2	191	8:06:55 AM	8:08:59 AM	124
2	192	8:07:01 AM	8:09:09 AM	128
2	193	8:07:50 AM	8:09:12 AM	82
2	194	8:07:55 AM	8:09:22 AM	87
2	195	8:08:14 AM	8:09:25 AM	71
2	196	8:08:15 AM	8:09:32 AM	77
2	197	8:08:21 AM	8:09:47 AM	86
2	198	8:08:22 AM	8:09:50 AM	88
2	199	8:08:52 AM	8:09:53 AM	61
2	200	8:09:04 AM	8:10:17 AM	73
2	201	8:09:11 AM	8:10:47 AM	96
2	202	8:09:11 AM	8:10:52 AM	101
2	203	8:09:29 AM	8:11:10 AM	101
2	204	8:10:11 AM	8:11:14 AM	63
2	205	8:10:12 AM	8:11:40 AM	88
2	206	8:10:16 AM	8:12:53 AM	157
2	207	8:10:20 AM	8:12:58 AM	158
2	208	8:10:31 AM	8:13:01 AM	150
2	209	8:10:32 AM	8:13:14 AM	162
2	210	8:10:36 AM	8:13:52 AM	196
2	211	8:11:17 AM	8:13:56 AM	159
2	212	8:11:24 AM	8:14:39 AM	195
2	213	8:11:28 AM	8:14:55 AM	207
2	214	8:12:05 AM	8:14:57 AM	172

Summary Information:

7:15:00 AM - 8:15:00 AM	Lane 1	Lane 2
Total Vehicle Count:	15	213
Delayed Vehicle Count:	15	213
Through Vehicle Count:	0	0
Average Stopped Time:	30.87	81.559
Maximum Stopped Time:	78	207
Min. Secs. for Delay:	0	0
Average Queue:	0.14	4.757
Queue Density:	1.14	5.792
Maximum Queue:	3	14
Delay in Vehicle Hour:	0.14	4.756845
Total Delay:	463	17372

CDM SMITH Inc.
 1100 Marion Street, Suite 200
 Knoxville, TN 37921
 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/28/2013

Lane 2- WB Garrison Drive

Page No : 1

L n.	No.	Joined Queue	Released From Queue	Delay
1	3	2:01:12 PM	2:01:14 PM	2
1	4	2:01:37 PM	2:01:40 PM	3
1	5	2:04:51 PM	2:05:01 PM	10
1	6	2:05:10 PM	2:05:13 PM	3
1	7	2:05:37 PM	2:05:51 PM	14
1	8	2:09:01 PM	2:09:17 PM	16
1	9	2:10:58 PM	2:10:59 PM	1
1	10	2:13:06 PM	2:13:12 PM	6
1	11	2:30:32 PM	2:30:33 PM	1
1	12	2:44:40 PM	2:44:47 PM	7
1	13	2:46:48 PM	2:47:50 PM	62
1	14	2:47:42 PM	2:48:56 PM	74
1	15	2:48:31 PM	2:49:05 PM	34
1	16	2:49:40 PM	2:50:13 PM	33
1	17	2:50:37 PM	2:50:51 PM	14
1	18	2:53:59 PM	2:54:01 PM	2
1	19	2:56:33 PM	2:56:36 PM	3
1	20	3:05:18 PM	3:05:21 PM	3
1	21	3:06:31 PM	3:06:39 PM	8
1	22	3:07:27 PM	3:07:51 PM	24
1	23	3:07:27 PM	3:08:03 PM	36
1	24	3:11:20 PM	3:12:02 PM	42
1	25	3:11:21 PM	3:12:05 PM	44
1	26	3:13:32 PM	3:13:45 PM	13
1	27	3:24:35 PM	3:24:44 PM	9
1	28	3:26:27 PM	3:26:38 PM	11
1	29	3:35:27 PM	3:36:26 PM	59
1	30	3:37:46 PM	3:37:58 PM	12
1	31	3:39:13 PM	3:39:25 PM	12
1	32	3:39:14 PM	3:39:34 PM	20
1	33	3:40:01 PM	3:40:08 PM	7
1	34	3:40:31 PM	3:40:40 PM	9
1	35	3:42:21 PM	3:42:41 PM	20
1	36	3:42:42 PM	3:42:47 PM	5
1	37	3:42:53 PM	3:43:18 PM	25
1	38	3:50:10 PM	3:50:16 PM	6
1	39	3:50:23 PM	3:50:50 PM	27
1	40	3:54:12 PM	3:54:19 PM	7
1	41	3:54:40 PM	3:55:30 PM	50
1	42	3:55:12 PM	3:55:46 PM	34
1	43	4:05:58 PM	4:06:06 PM	8
1	44	4:06:01 PM	4:06:12 PM	11
1	45	4:10:38 PM	4:11:13 PM	35
1	46	4:12:02 PM	4:12:13 PM	11
1	47	4:12:18 PM	4:12:21 PM	3
1	48	4:21:42 PM	4:22:16 PM	34
1	49	4:30:59 PM	4:31:31 PM	32
1	50	4:35:04 PM	4:35:15 PM	11
1	51	4:35:28 PM	4:35:35 PM	7
1	52	4:37:53 PM	4:37:56 PM	3
1	53	5:03:57 PM	5:04:21 PM	24
1	54	5:04:36 PM	5:05:15 PM	39
1	55	5:12:43 PM	5:13:53 PM	70
1	56	5:15:46 PM	5:15:55 PM	9
1	57	5:25:26 PM	5:25:31 PM	5
1	58	5:29:50 PM	5:30:01 PM	11
1	59	5:32:13 PM	5:32:53 PM	40
1	60	5:35:22 PM	5:35:35 PM	13
1	61	5:38:17 PM	5:38:19 PM	2
1	62	5:42:52 PM	5:42:58 PM	6
1	63	5:44:02 PM	5:44:39 PM	37

CDM SMITH Inc.
 1100 Marion Street, Suite 200
 Knoxville, TN 37921
 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/28/2013

Lane 2- WB Garrison Drive

Page No : 2

L n.	No.	Joined Queue	Released From Queue	Delay
1	64	5:44:55 PM	5:44:58 PM	3
1	65	5:46:16 PM	5:46:26 PM	10
1	66	5:48:23 PM	5:48:26 PM	3
1	67	5:48:32 PM	5:48:33 PM	1
1	68	5:53:08 PM	5:54:06 PM	58
2	2	2:00:39 PM	2:00:45 PM	6
2	3	2:01:48 PM	2:02:12 PM	24
2	4	2:15:00 PM	2:15:10 PM	10
2	5	2:15:48 PM	2:15:57 PM	9
2	6	2:18:05 PM	2:18:09 PM	4
2	7	2:18:06 PM	2:18:13 PM	7
2	8	2:18:50 PM	2:19:15 PM	25
2	9	2:19:11 PM	2:19:22 PM	11
2	10	2:22:13 PM	2:22:18 PM	5
2	11	2:28:22 PM	2:28:34 PM	12
2	12	2:31:49 PM	2:31:55 PM	6
2	13	2:34:51 PM	2:34:52 PM	1
2	14	2:35:13 PM	2:35:21 PM	8
2	15	2:35:16 PM	2:35:24 PM	8
2	16	2:35:38 PM	2:35:59 PM	21
2	17	2:37:10 PM	2:37:32 PM	22
2	18	2:40:33 PM	2:40:40 PM	7
2	19	2:41:16 PM	2:41:19 PM	3
2	20	2:42:44 PM	2:42:58 PM	14
2	21	2:42:47 PM	2:43:05 PM	18
2	22	2:43:31 PM	2:43:32 PM	1
2	23	2:46:40 PM	2:46:42 PM	2
2	24	2:49:23 PM	2:49:24 PM	1
2	25	2:51:02 PM	2:55:31 PM	269
2	26	2:55:00 PM	2:56:32 PM	92
2	27	2:56:29 PM	2:57:06 PM	37
2	28	2:56:48 PM	2:57:08 PM	20
2	29	3:01:22 PM	3:01:24 PM	2
2	30	3:09:46 PM	3:09:54 PM	8
2	31	3:13:17 PM	3:13:18 PM	1
2	32	3:15:00 PM	3:15:05 PM	5
2	33	3:17:07 PM	3:17:19 PM	12
2	34	3:17:54 PM	3:17:57 PM	3
2	35	3:20:13 PM	3:20:16 PM	3
2	36	3:22:12 PM	3:22:18 PM	6
2	37	3:23:19 PM	3:23:30 PM	11
2	38	3:23:51 PM	3:23:54 PM	3
2	39	3:24:06 PM	3:24:16 PM	10
2	40	3:24:37 PM	3:24:40 PM	3
2	41	3:25:52 PM	3:25:55 PM	3
2	42	3:26:10 PM	3:26:13 PM	3
2	43	3:27:17 PM	3:27:24 PM	7
2	44	3:28:44 PM	3:28:49 PM	5
2	45	3:28:48 PM	3:28:53 PM	5
2	46	3:28:55 PM	3:28:59 PM	4
2	47	3:28:57 PM	3:29:11 PM	14
2	48	3:29:12 PM	3:29:14 PM	2
2	49	3:29:15 PM	3:29:25 PM	10
2	50	3:29:16 PM	3:29:37 PM	21
2	51	3:29:34 PM	3:29:44 PM	10
2	52	3:29:35 PM	3:29:50 PM	15
2	53	3:29:48 PM	3:29:55 PM	7
2	54	3:30:56 PM	3:31:00 PM	4
2	55	3:30:57 PM	3:31:07 PM	10
2	56	3:30:59 PM	3:31:16 PM	17
2	57	3:31:00 PM	3:31:29 PM	29

CDM SMITH Inc.
 1100 Marion Street, Suite 200
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 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/28/2013

Lane 2- WB Garrison Drive

Page No : 3

L n.	No.	Joined Queue	Released From Queue	Delay
2	58	3:31:27 PM	3:31:35 PM	8
2	59	3:31:36 PM	3:31:47 PM	11
2	60	3:31:37 PM	3:31:56 PM	19
2	61	3:32:08 PM	3:32:14 PM	6
2	62	3:32:11 PM	3:32:28 PM	17
2	63	3:32:15 PM	3:32:39 PM	24
2	64	3:32:28 PM	3:32:53 PM	25
2	65	3:32:32 PM	3:33:02 PM	30
2	66	3:32:34 PM	3:33:10 PM	36
2	67	3:32:43 PM	3:33:35 PM	52
2	68	3:32:49 PM	3:33:39 PM	50
2	69	3:32:52 PM	3:34:00 PM	68
2	70	3:33:04 PM	3:34:13 PM	69
2	71	3:33:19 PM	3:34:16 PM	57
2	72	3:33:22 PM	3:34:23 PM	61
2	73	3:33:26 PM	3:34:27 PM	61
2	74	3:33:40 PM	3:34:34 PM	54
2	75	3:33:50 PM	3:34:38 PM	48
2	76	3:34:20 PM	3:34:54 PM	34
2	77	3:34:26 PM	3:34:57 PM	31
2	78	3:34:28 PM	3:35:08 PM	40
2	79	3:34:31 PM	3:35:21 PM	50
2	80	3:34:42 PM	3:35:25 PM	43
2	81	3:34:46 PM	3:35:28 PM	42
2	82	3:34:49 PM	3:35:57 PM	68
2	83	3:35:05 PM	3:36:29 PM	84
2	84	3:35:12 PM	3:36:36 PM	84
2	85	3:35:33 PM	3:36:40 PM	67
2	86	3:35:54 PM	3:37:07 PM	73
2	87	3:35:56 PM	3:37:11 PM	75
2	88	3:36:41 PM	3:37:14 PM	33
2	89	3:36:53 PM	3:37:19 PM	26
2	90	3:36:55 PM	3:37:22 PM	27
2	91	3:37:03 PM	3:37:41 PM	38
2	92	3:37:09 PM	3:37:48 PM	39
2	93	3:37:16 PM	3:37:52 PM	36
2	94	3:37:37 PM	3:37:59 PM	22
2	95	3:37:38 PM	3:38:02 PM	24
2	96	3:37:39 PM	3:38:23 PM	44
2	97	3:38:00 PM	3:38:53 PM	53
2	98	3:38:07 PM	3:39:02 PM	55
2	99	3:38:11 PM	3:39:07 PM	56
2	100	3:38:14 PM	3:39:26 PM	72
2	101	3:38:18 PM	3:39:34 PM	76
2	102	3:38:23 PM	3:39:56 PM	93
2	103	3:38:25 PM	3:40:01 PM	96
2	104	3:39:06 PM	3:40:08 PM	62
2	105	3:39:08 PM	3:40:15 PM	67
2	106	3:39:08 PM	3:40:22 PM	74
2	107	3:39:16 PM	3:40:26 PM	70
2	108	3:39:23 PM	3:40:33 PM	70
2	109	3:39:23 PM	3:40:38 PM	75
2	110	3:39:23 PM	3:41:00 PM	97
2	111	3:39:33 PM	3:41:03 PM	90
2	112	3:40:09 PM	3:41:06 PM	57
2	113	3:40:13 PM	3:41:09 PM	56
2	114	3:40:28 PM	3:41:22 PM	54
2	115	3:40:31 PM	3:41:27 PM	56
2	116	3:40:59 PM	3:42:01 PM	62
2	117	3:41:00 PM	3:42:43 PM	103
2	118	3:41:19 PM	3:43:01 PM	102

CDM SMITH Inc.
 1100 Marion Street, Suite 200
 Knoxville, TN 37921
 (865) 963-4300

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM

Site Code : 101

Lane 1- EB Garrison Drive

Start Date : 10/28/2013

Lane 2- WB Garrison Drive

Page No : 4

L n.	No.	Joined Queue	Released From Queue	Delay
2	119	3:41:33 PM	3:43:19 PM	106
2	120	3:42:20 PM	3:43:23 PM	63
2	121	3:42:26 PM	3:43:27 PM	61
2	122	3:43:05 PM	3:43:31 PM	26
2	123	3:43:24 PM	3:44:28 PM	64
2	124	3:43:26 PM	3:44:46 PM	80
2	125	3:43:29 PM	3:44:52 PM	83
2	126	3:43:50 PM	3:45:17 PM	87
2	127	3:44:02 PM	3:45:35 PM	93
2	128	3:44:14 PM	3:45:38 PM	84
2	129	3:44:39 PM	3:45:54 PM	75
2	130	3:44:53 PM	3:46:08 PM	75
2	131	3:44:55 PM	3:46:19 PM	84
2	132	3:45:35 PM	3:46:21 PM	46
2	133	3:45:56 PM	3:46:26 PM	30
2	134	3:46:15 PM	3:46:51 PM	36
2	135	3:46:15 PM	3:47:00 PM	45
2	136	3:46:36 PM	3:47:06 PM	30
2	137	3:46:48 PM	3:47:26 PM	38
2	138	3:47:09 PM	3:47:32 PM	23
2	139	3:47:21 PM	3:47:38 PM	17
2	140	3:47:26 PM	3:47:43 PM	17
2	141	3:47:36 PM	3:47:48 PM	12
2	142	3:48:22 PM	3:48:25 PM	3
2	143	3:48:23 PM	3:48:29 PM	6
2	144	3:48:24 PM	3:48:41 PM	17
2	145	3:48:39 PM	3:48:45 PM	6
2	146	3:49:24 PM	3:49:30 PM	6
2	147	3:49:44 PM	3:50:07 PM	23
2	148	3:50:28 PM	3:50:59 PM	31
2	149	3:50:42 PM	3:51:07 PM	25
2	150	3:51:38 PM	3:51:46 PM	8
2	151	3:51:59 PM	3:52:11 PM	12
2	152	3:52:09 PM	3:52:21 PM	12
2	153	3:52:51 PM	3:52:54 PM	3
2	154	3:53:53 PM	3:54:20 PM	27
2	155	3:54:05 PM	3:54:25 PM	20
2	156	3:54:26 PM	3:54:36 PM	10
2	157	3:54:27 PM	3:55:16 PM	49
2	158	3:55:34 PM	3:55:46 PM	12
2	159	3:57:36 PM	3:57:39 PM	3
2	160	3:58:04 PM	3:58:06 PM	2
2	161	3:59:06 PM	3:59:27 PM	21
2	162	4:00:22 PM	4:00:32 PM	10
2	163	4:00:45 PM	4:01:17 PM	32
2	164	4:02:15 PM	4:02:19 PM	4
2	165	4:03:36 PM	4:04:16 PM	40
2	166	4:07:47 PM	4:08:01 PM	14
2	167	4:10:38 PM	4:11:07 PM	29
2	168	4:13:11 PM	4:13:27 PM	16
2	169	4:13:54 PM	4:13:56 PM	2
2	170	4:14:48 PM	4:15:21 PM	33
2	171	4:15:21 PM	4:15:24 PM	3
2	172	4:18:23 PM	4:18:35 PM	12
2	173	4:18:44 PM	4:18:51 PM	7
2	174	4:20:34 PM	4:20:56 PM	22
2	175	4:20:57 PM	4:21:05 PM	8
2	176	4:21:13 PM	4:21:40 PM	27
2	177	4:21:35 PM	4:21:47 PM	12
2	178	4:24:48 PM	4:24:54 PM	6
2	179	4:25:51 PM	4:25:53 PM	2

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Counted by: Allyson Foster

File Name : Garrison Stop Delay PM

Lane 1- EB Garrison Drive

Site Code : 101

Lane 2- WB Garrison Drive

Start Date : 10/28/2013

Page No : 5

L n.	No.	Joined Queue	Released From Queue	Delay
2	180	4:26:03 PM	4:26:06 PM	3
2	181	4:30:43 PM	4:30:57 PM	14
2	182	4:36:20 PM	4:36:30 PM	10
2	183	4:36:41 PM	4:36:43 PM	2
2	184	4:41:28 PM	4:41:40 PM	12
2	185	4:42:59 PM	4:43:00 PM	1
2	186	4:43:15 PM	4:43:19 PM	4
2	187	4:44:52 PM	4:45:10 PM	18
2	188	4:45:56 PM	4:46:00 PM	4
2	189	4:51:21 PM	4:51:23 PM	2
2	190	4:54:40 PM	4:54:45 PM	5
2	191	4:58:15 PM	4:58:16 PM	1
2	192	4:58:15 PM	4:58:18 PM	3
2	193	5:00:31 PM	5:00:35 PM	4
2	194	5:00:59 PM	5:02:17 PM	78
2	195	5:02:11 PM	5:02:31 PM	20
2	196	5:02:23 PM	5:02:48 PM	25
2	197	5:02:29 PM	5:02:52 PM	23
2	198	5:02:47 PM	5:02:56 PM	9
2	199	5:03:13 PM	5:03:31 PM	18
2	200	5:03:59 PM	5:04:13 PM	14
2	201	5:05:19 PM	5:05:22 PM	3
2	202	5:05:30 PM	5:05:49 PM	19
2	203	5:07:48 PM	5:07:53 PM	5
2	204	5:08:10 PM	5:08:13 PM	3
2	205	5:08:19 PM	5:08:22 PM	3
2	206	5:08:51 PM	5:08:53 PM	2
2	207	5:09:24 PM	5:10:29 PM	65
2	208	5:11:07 PM	5:11:12 PM	5
2	209	5:11:46 PM	5:12:03 PM	17
2	210	5:12:37 PM	5:13:21 PM	44
2	211	5:15:30 PM	5:15:41 PM	11
2	212	5:15:44 PM	5:15:52 PM	8
2	213	5:17:29 PM	5:17:29 PM	0
2	214	5:18:45 PM	5:18:53 PM	8
2	215	5:21:50 PM	5:23:10 PM	80
2	216	5:23:15 PM	5:23:23 PM	8
2	217	5:26:32 PM	5:26:36 PM	4
2	218	5:28:58 PM	5:29:01 PM	3
2	219	5:29:25 PM	5:29:58 PM	33
2	220	5:29:27 PM	5:30:08 PM	41
2	221	5:33:02 PM	5:33:21 PM	19
2	222	5:35:06 PM	5:35:07 PM	1
2	223	5:35:12 PM	5:35:32 PM	20
2	224	5:36:36 PM	5:36:51 PM	15
2	225	5:37:12 PM	5:37:21 PM	9
2	226	5:37:42 PM	5:38:02 PM	20
2	227	5:38:28 PM	5:38:34 PM	6
2	228	5:38:47 PM	5:39:00 PM	13
2	229	5:42:59 PM	5:43:37 PM	38
2	230	5:52:39 PM	5:53:03 PM	24
2	231	5:52:52 PM	5:54:02 PM	70
2	232	5:54:34 PM	5:56:45 PM	131

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Counted by: Allyson Foster

Lane 1- EB Garrison Drive
Lane 2- WB Garrison Drive

File Name : Garrison Stop Delay PM
Site Code : 101
Start Date : 10/28/2013
Page No : 6

Summary Information:

2:00:00 PM - 5:57:00 PM	Lane 1	Lane 2
Total Vehicle Count:	66	231
Delayed Vehicle Count:	66	231
Through Vehicle Count:	0	0
Average Stopped Time:	19.00	30.286
Maximum Stopped Time:	74	269
Min. Secs. for Delay:	0	0
Average Queue:	0.09	0.494
Queue Density:	1.12	2.222
Maximum Queue:	2	11
Delay in Vehicle Hour:	0.09	0.49
Total Delay:	1254	6996

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM
 Site Code : 101
 Start Date : 10/28/2013
 Page No : 1

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

L n.	No.	Joined Queue	Released From Queue	Delay
1	20	3:05:18 PM	3:05:21 PM	3
1	21	3:06:31 PM	3:06:39 PM	8
1	22	3:07:27 PM	3:07:51 PM	24
1	23	3:07:27 PM	3:08:03 PM	36
1	24	3:11:20 PM	3:12:02 PM	42
1	25	3:11:21 PM	3:12:05 PM	44
1	26	3:13:32 PM	3:13:45 PM	13
1	27	3:24:35 PM	3:24:44 PM	9
1	28	3:26:27 PM	3:26:38 PM	11
1	29	3:35:27 PM	3:36:26 PM	59
1	30	3:37:46 PM	3:37:58 PM	12
1	31	3:39:13 PM	3:39:25 PM	12
1	32	3:39:14 PM	3:39:34 PM	20
1	33	3:40:01 PM	3:40:08 PM	7
1	34	3:40:31 PM	3:40:40 PM	9
1	35	3:42:21 PM	3:42:41 PM	20
1	36	3:42:42 PM	3:42:47 PM	5
1	37	3:42:53 PM	3:43:18 PM	25
1	38	3:50:10 PM	3:50:16 PM	6
1	39	3:50:23 PM	3:50:50 PM	27
1	40	3:54:12 PM	3:54:19 PM	7
1	41	3:54:40 PM	3:55:30 PM	50
1	42	3:55:12 PM	3:55:46 PM	34
2	29	3:01:22 PM	3:01:24 PM	2
2	30	3:09:46 PM	3:09:54 PM	8
2	31	3:13:17 PM	3:13:18 PM	1
2	32	3:15:00 PM	3:15:05 PM	5
2	33	3:17:07 PM	3:17:19 PM	12
2	34	3:17:54 PM	3:17:57 PM	3
2	35	3:20:13 PM	3:20:16 PM	3
2	36	3:22:12 PM	3:22:18 PM	6
2	37	3:23:19 PM	3:23:30 PM	11
2	38	3:23:51 PM	3:23:54 PM	3
2	39	3:24:06 PM	3:24:16 PM	10
2	40	3:24:37 PM	3:24:40 PM	3
2	41	3:25:52 PM	3:25:55 PM	3
2	42	3:26:10 PM	3:26:13 PM	3
2	43	3:27:17 PM	3:27:24 PM	7
2	44	3:28:44 PM	3:28:49 PM	5
2	45	3:28:48 PM	3:28:53 PM	5
2	46	3:28:55 PM	3:28:59 PM	4
2	47	3:28:57 PM	3:29:11 PM	14
2	48	3:29:12 PM	3:29:14 PM	2
2	49	3:29:15 PM	3:29:25 PM	10
2	50	3:29:16 PM	3:29:37 PM	21
2	51	3:29:34 PM	3:29:44 PM	10
2	52	3:29:35 PM	3:29:50 PM	15
2	53	3:29:48 PM	3:29:55 PM	7
2	54	3:30:56 PM	3:31:00 PM	4
2	55	3:30:57 PM	3:31:07 PM	10
2	56	3:30:59 PM	3:31:16 PM	17
2	57	3:31:00 PM	3:31:29 PM	29
2	58	3:31:27 PM	3:31:35 PM	8
2	59	3:31:36 PM	3:31:47 PM	11
2	60	3:31:37 PM	3:31:56 PM	19
2	61	3:32:08 PM	3:32:14 PM	6
2	62	3:32:11 PM	3:32:28 PM	17
2	63	3:32:15 PM	3:32:39 PM	24
2	64	3:32:28 PM	3:32:53 PM	25
2	65	3:32:32 PM	3:33:02 PM	30
2	66	3:32:34 PM	3:33:10 PM	36
2	67	3:32:43 PM	3:33:35 PM	52
2	68	3:32:49 PM	3:33:39 PM	50

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM
 Site Code : 101
 Start Date : 10/28/2013
 Page No : 2

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

L n.	No.	Joined Queue	Released From Queue	Delay
2	69	3:32:52 PM	3:34:00 PM	68
2	70	3:33:04 PM	3:34:13 PM	69
2	71	3:33:19 PM	3:34:16 PM	57
2	72	3:33:22 PM	3:34:23 PM	61
2	73	3:33:26 PM	3:34:27 PM	61
2	74	3:33:40 PM	3:34:34 PM	54
2	75	3:33:50 PM	3:34:38 PM	48
2	76	3:34:20 PM	3:34:54 PM	34
2	77	3:34:26 PM	3:34:57 PM	31
2	78	3:34:28 PM	3:35:08 PM	40
2	79	3:34:31 PM	3:35:21 PM	50
2	80	3:34:42 PM	3:35:25 PM	43
2	81	3:34:46 PM	3:35:28 PM	42
2	82	3:34:49 PM	3:35:57 PM	68
2	83	3:35:05 PM	3:36:29 PM	84
2	84	3:35:12 PM	3:36:36 PM	84
2	85	3:35:33 PM	3:36:40 PM	67
2	86	3:35:54 PM	3:37:07 PM	73
2	87	3:35:56 PM	3:37:11 PM	75
2	88	3:36:41 PM	3:37:14 PM	33
2	89	3:36:53 PM	3:37:19 PM	26
2	90	3:36:55 PM	3:37:22 PM	27
2	91	3:37:03 PM	3:37:41 PM	38
2	92	3:37:09 PM	3:37:48 PM	39
2	93	3:37:16 PM	3:37:52 PM	36
2	94	3:37:37 PM	3:37:59 PM	22
2	95	3:37:38 PM	3:38:02 PM	24
2	96	3:37:39 PM	3:38:23 PM	44
2	97	3:38:00 PM	3:38:53 PM	53
2	98	3:38:07 PM	3:39:02 PM	55
2	99	3:38:11 PM	3:39:07 PM	56
2	100	3:38:14 PM	3:39:26 PM	72
2	101	3:38:18 PM	3:39:34 PM	76
2	102	3:38:23 PM	3:39:56 PM	93
2	103	3:38:25 PM	3:40:01 PM	96
2	104	3:39:06 PM	3:40:08 PM	62
2	105	3:39:08 PM	3:40:15 PM	67
2	106	3:39:08 PM	3:40:22 PM	74
2	107	3:39:16 PM	3:40:26 PM	70
2	108	3:39:23 PM	3:40:33 PM	70
2	109	3:39:23 PM	3:40:38 PM	75
2	110	3:39:23 PM	3:41:00 PM	97
2	111	3:39:33 PM	3:41:03 PM	90
2	112	3:40:09 PM	3:41:06 PM	57
2	113	3:40:13 PM	3:41:09 PM	56
2	114	3:40:28 PM	3:41:22 PM	54
2	115	3:40:31 PM	3:41:27 PM	56
2	116	3:40:59 PM	3:42:01 PM	62
2	117	3:41:00 PM	3:42:43 PM	103
2	118	3:41:19 PM	3:43:01 PM	102
2	119	3:41:33 PM	3:43:19 PM	106
2	120	3:42:20 PM	3:43:23 PM	63
2	121	3:42:26 PM	3:43:27 PM	61
2	122	3:43:05 PM	3:43:31 PM	26
2	123	3:43:24 PM	3:44:28 PM	64
2	124	3:43:26 PM	3:44:46 PM	80
2	125	3:43:29 PM	3:44:52 PM	83
2	126	3:43:50 PM	3:45:17 PM	87
2	127	3:44:02 PM	3:45:35 PM	93
2	128	3:44:14 PM	3:45:38 PM	84
2	129	3:44:39 PM	3:45:54 PM	75
2	130	3:44:53 PM	3:46:08 PM	75
2	131	3:44:55 PM	3:46:19 PM	84
2	132	3:45:35 PM	3:46:21 PM	46

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM
 Site Code : 101
 Start Date : 10/28/2013
 Page No : 3

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

L n.	No.	Joined Queue	Released From Queue	Delay
2	133	3:45:56 PM	3:46:26 PM	30
2	134	3:46:15 PM	3:46:51 PM	36
2	135	3:46:15 PM	3:47:00 PM	45
2	136	3:46:36 PM	3:47:06 PM	30
2	137	3:46:48 PM	3:47:26 PM	38
2	138	3:47:09 PM	3:47:32 PM	23
2	139	3:47:21 PM	3:47:38 PM	17
2	140	3:47:26 PM	3:47:43 PM	17
2	141	3:47:36 PM	3:47:48 PM	12
2	142	3:48:22 PM	3:48:25 PM	3
2	143	3:48:23 PM	3:48:29 PM	6
2	144	3:48:24 PM	3:48:41 PM	17
2	145	3:48:39 PM	3:48:45 PM	6
2	146	3:49:24 PM	3:49:30 PM	6
2	147	3:49:44 PM	3:50:07 PM	23
2	148	3:50:28 PM	3:50:59 PM	31
2	149	3:50:42 PM	3:51:07 PM	25
2	150	3:51:38 PM	3:51:46 PM	8
2	151	3:51:59 PM	3:52:11 PM	12
2	152	3:52:09 PM	3:52:21 PM	12
2	153	3:52:51 PM	3:52:54 PM	3
2	154	3:53:53 PM	3:54:20 PM	27
2	155	3:54:05 PM	3:54:25 PM	20
2	156	3:54:26 PM	3:54:36 PM	10
2	157	3:54:27 PM	3:55:16 PM	49
2	158	3:55:34 PM	3:55:46 PM	12
2	159	3:57:36 PM	3:57:39 PM	3
2	160	3:58:04 PM	3:58:06 PM	2
2	161	3:59:06 PM	3:59:27 PM	21

Summary Information:

3:00:00 PM - 4:00:00 PM	Lane 1	Lane 2
Total Vehicle Count:	23	133
Delayed Vehicle Count:	23	133
Through Vehicle Count:	0	0
Average Stopped Time:	21.00	38.165
Maximum Stopped Time:	59	106
Min. Secs. for Delay:	0	0
Average Queue:	0.16	1.456
Queue Density:	1.24	3.654
Maximum Queue:	2	11
Delay in Vehicle Hour:	0.16	1.456528
Total Delay:	483	5076

Counted by: Allyson Foster

File Name : Garrison Stop Delay PM
 Site Code : 101
 Start Date : 10/28/2013
 Page No : 1

Lane 1- EB Garrison Drive
 Lane 2- WB Garrison Drive

L n.	No.	Joined Queue	Released From Queue	Delay
1	53	5:03:57 PM	5:04:21 PM	24
1	54	5:04:36 PM	5:05:15 PM	39
1	55	5:12:43 PM	5:13:53 PM	70
1	56	5:15:46 PM	5:15:55 PM	9
1	57	5:25:26 PM	5:25:31 PM	5
1	58	5:29:50 PM	5:30:01 PM	11
1	59	5:32:13 PM	5:32:53 PM	40
1	60	5:35:22 PM	5:35:35 PM	13
1	61	5:38:17 PM	5:38:19 PM	2
1	62	5:42:52 PM	5:42:58 PM	6
1	63	5:44:02 PM	5:44:39 PM	37
1	64	5:44:55 PM	5:44:58 PM	3
1	65	5:46:16 PM	5:46:26 PM	10
1	66	5:48:23 PM	5:48:26 PM	3
1	67	5:48:32 PM	5:48:33 PM	1
1	68	5:53:08 PM	5:54:06 PM	58
2	191	4:58:15 PM	4:58:16 PM	1
2	192	4:58:15 PM	4:58:18 PM	3
2	193	5:00:31 PM	5:00:35 PM	4
2	194	5:00:59 PM	5:02:17 PM	78
2	195	5:02:11 PM	5:02:31 PM	20
2	196	5:02:23 PM	5:02:48 PM	25
2	197	5:02:29 PM	5:02:52 PM	23
2	198	5:02:47 PM	5:02:56 PM	9
2	199	5:03:13 PM	5:03:31 PM	18
2	200	5:03:59 PM	5:04:13 PM	14
2	201	5:05:19 PM	5:05:22 PM	3
2	202	5:05:30 PM	5:05:49 PM	19
2	203	5:07:48 PM	5:07:53 PM	5
2	204	5:08:10 PM	5:08:13 PM	3
2	205	5:08:19 PM	5:08:22 PM	3
2	206	5:08:51 PM	5:08:53 PM	2
2	207	5:09:24 PM	5:10:29 PM	65
2	208	5:11:07 PM	5:11:12 PM	5
2	209	5:11:46 PM	5:12:03 PM	17
2	210	5:12:37 PM	5:13:21 PM	44
2	211	5:15:30 PM	5:15:41 PM	11
2	212	5:15:44 PM	5:15:52 PM	8
2	213	5:17:29 PM	5:17:29 PM	0
2	214	5:18:45 PM	5:18:53 PM	8
2	215	5:21:50 PM	5:23:10 PM	80
2	216	5:23:15 PM	5:23:23 PM	8
2	217	5:26:32 PM	5:26:36 PM	4
2	218	5:28:58 PM	5:29:01 PM	3
2	219	5:29:25 PM	5:29:58 PM	33
2	220	5:29:27 PM	5:30:08 PM	41
2	221	5:33:02 PM	5:33:21 PM	19
2	222	5:35:06 PM	5:35:07 PM	1
2	223	5:35:12 PM	5:35:32 PM	20
2	224	5:36:36 PM	5:36:51 PM	15
2	225	5:37:12 PM	5:37:21 PM	9
2	226	5:37:42 PM	5:38:02 PM	20
2	227	5:38:28 PM	5:38:34 PM	6
2	228	5:38:47 PM	5:39:00 PM	13
2	229	5:42:59 PM	5:43:37 PM	38
2	230	5:52:39 PM	5:53:03 PM	24
2	231	5:52:52 PM	5:54:02 PM	70
2	232	5:54:34 PM	5:56:45 PM	131

Counted by: Allyson Foster

Lane 1- EB Garrison Drive
Lane 2- WB Garrison Drive

File Name : Garrison Stop Delay PM
Site Code : 101
Start Date : 10/28/2013
Page No : 2

Summary Information:

4:57:00 PM - 5:57:00 PM	Lane 1	Lane 2
Total Vehicle Count:	16	42
Delayed Vehicle Count:	16	42
Through Vehicle Count:	0	0
Average Stopped Time:	20.69	21.976
Maximum Stopped Time:	70	131
Min. Secs. for Delay:	0	0
Average Queue:	0.11	0.262
Queue Density:	1.00	1.095
Maximum Queue:	1	3
Delay in Vehicle Hour:	0.11	0.262963
Total Delay:	331	923



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