**Traffic Impact Study** 

Copper Trace Development Copper Ridge Road Knox County, Tennessee

01048-0000

November 8, 2012



Prepared for: Rackley Engineering 3008 Yearling Lane Maryville, TN 37803



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### **EXECUTIVE SUMMARY**

This report provides a summary of the traffic impact study that was performed for a proposed residential development to be located off Copper Ridge Road in Northwest Knox County. The project site is approximately one-half mile north of Emory Road (State Route 131) on the east side of Copper Ridge Road. The current plans for this proposed development provide for a maximum of 102 single family dwelling units at full build-out. The project entrance will be a new three-leg intersection on Copper Ridge Road located approximately one-half mile north of Emory Road.

A primary conclusion of this study is that the traffic generated by the proposed development will result in short-term impacts on traffic operational conditions in the project area, especially at the intersection of Emory Road and Copper Ridge Road. The resulting recommendation is that consideration should be given to implementing possible improvements to the intersection of Emory Road and Copper Ridge Road, with the following issues being considered in determining improvement scope and responsibility:

- 1. An eastbound left-turn lane will be justified during both A.M. and P.M. peak hour traffic conditions as a result of this development. Such a lane is currently justified during existing P.M. peak hour conditions.
- 2. The southbound capacity analyses of unimproved year 2017 conditions identified level-of-service "E" conditions for only one peak hour. With the addition of a second southbound approach lane on Copper Ridge Road at Emory Road, the resulting levels-of-service for the southbound left-turn movement is LOS "E" and LOS "C" for the southbound right turns.
- 3. Existing roadway right-of-way on both Emory Road and Copper Ridge Road at this intersection appear to be on the order of 50 feet, and are not under the control of the project developer.

Other traffic related issues evaluated for this project included corner sight distance for the proposed subdivision access roadway intersection with Copper Ridge Road, and the general width and condition of Copper Ridge Road between Emory Road and the project site. These evaluations concluded that corner sight distance requirements will be satisfied, and that although Copper Ridge Road is narrow, its width does meet or exceed the eighteen foot minimum required by Knox County.

### INTRODUCTION AND PURPOSE OF STUDY

This report provides a summary of the traffic impact study that was performed for a proposed residential development to be located off Copper Ridge Road in Northwest Knox County. The project site is approximately one-half mile north of Emory Road (State Route 131) on the east side of Copper Ridge Road. FIGURE 1 is a location map that identifies the project site in relation to the roadways in the vicinity of the proposed development.

The current plans for this proposed subdivision development provide for a maximum of 102 single family dwelling units at full build-out. FIGURE 2 is a site map showing the proposed site layout with access to Copper Ridge Road. The development entrance will be a new three-leg intersection on Copper Ridge Road located approximately one-half mile north of Emory Road.

The purpose of this study was the evaluation of the traffic operational and safety impact of the proposed development upon the adjacent portion of Copper Ridge Road. Of particular interest was the intersection of the site entrance roadway with Copper Ridge Road, as well as the intersection of Copper Ridge Road and Emory Road. This evaluation was performed assuming full build-out of all units of the subdivision, with existing and background growth conditions also evaluated for purposes of comparison.



FIGURE 1: LOCATION MAP





### **EXISTING CONDITIONS**

### **Existing Roadway Conditions**

Copper Ridge Road is a Local access roadway that is maintained by Knox County. The roadway pavement is approximately 20 feet in width, and is striped with a center double yellow line delineating two traffic lanes of approximately 8.5 to 9.0 feet. Minimal shoulders are located beyond the white pavement edgelines. The study section of Copper Ridge Road was constructed under older design standards, and thus possesses significant horizontal curvature and non-standard roadside ditches. The posted speed limit on Copper Ridge Road is 30 mph.

#### **Existing Traffic Data**

A traffic count station for collecting average daily traffic data (ADT) is located on Emory Road approximately three miles east of Copper Ridge Road. The most recent data from this station was provided by the Tennessee Department of Transportation, with resulting ADTs shown in TABLE 1.

TABLE 1											
AVERAGE	AVERAGE DAILY TRAFFIC COUNT										
SUMMARY											
Count Year	EMORY ROAD WEST OF CLINTON HIGHWAY										
	(TDOT STA. 047)										
2011	9,006										
2010	9,512										
2009	9,426										
2008	8,792										
2007	9,077										
2006	8,872										

In order to collect more refined data, and to establish a basis for trip distribution patterns, turning movement traffic counts were collected at the existing three-leg intersection of Emory Road and Copper Ridge Road. These counts were conducted during the A.M. and P.M. peak traffic hours. Raw data count summaries are contained in the APPENDIX.

In addition to helping establish trip distribution patterns, these turning movement counts were used to establish the existing traffic volumes for this study, as displayed on FIGURE 3.



FIGURE 3: 2012 EXISTING TRAFFIC DATA

#### Existing Level-of-Service Evaluation

Intersection Capacity Analyses employing the methods of the Highway Capacity Manual (HCM 2010) were used to evaluate the intersection of Emory Road and Copper Ridge Road for the existing roadway and traffic conditions. This intersection was chosen as the most critical of the two study intersections from a capacity and level-of-service perspective. The results indicate that Emory Road left-turn traffic movement is currently operating at level-of-service "A" during both the AM and PM peak hours with Copper Ridge Road turning movements operating at a level-of-service "C" during the AM peak and "B" during the PM peak. These results are summarized in detail on the "Two Way Stop Control Summary" printouts contained in the APPENDIX. Also see the APPENDIX for a discussion of Intersection Capacity and Level of Service Concepts.

### **BACKGROUND CONDITIONS**

### Background Traffic Growth

The anticipated time for full build-out of the Copper Trace Development is estimated as five years. Therefore, year 2017 was established as the appropriate design/analysis year for this study. In order to determine traffic volumes resulting solely from background traffic growth to year 2017, it was necessary to establish an annual growth rate for existing traffic. The ADT values given previously in TABLE 1, along with engineering judgment, were used to arrive at a rate of 2 percent for this development. FIGURE 4 contains the background traffic volumes that would result from this 2 percent annual growth to year 2017.

### Background Level of Service Evaluation

Intersection Capacity Analyses employing the methods of the Highway Capacity Manual (HCM 2010) were used to evaluate the study intersection of Emory Road and Copper Ridge Road for the background (2017) traffic conditions, shown on FIGURE 4. The results indicate that Emory Road left-turn traffic movement would be expected to operate at level-of-service "B" during AM peak hour and "A" during the PM peak hour, and Copper Ridge Road movements would operate at a level-of-service "D" during the AM peak hour and "B" during the PM peak hour, if the proposed development is not constructed. These results are summarized in detail on the "Two-Way Stop Control Summary" printouts contained in the APPENDIX. Also see the APPENDIX for a discussion of intersection capacity and level-of-service concepts.



FIGURE 4: 2017 BACKGROUND TRAFFIC DATA

### **PROPOSED CONDITIONS**

### Trip Generation

In order to estimate the expected traffic volumes to be generated by full build-out of the proposed development, the data and procedures of *Trip Generation, Eighth Edition* (Institute of Transportation Engineers, 2008) were utilized. The generated traffic volumes were determined based on the total weekday morning, and evening peak hour of adjacent street traffic trip generation rates for single-family detached housing (Land Use Code 210, Volume 2, pages 290 to 292). As noted earlier in this report, the anticipated maximum number of units upon full build-out is 102, which was used to determine the number of new trips generated. TABLE 2 summarizes the number and directional split of entering and exiting trips for the proposed development.

		TAI	BLE 2								
	TRIP GENERATION SUMMARY										
(R	(RATES FOR SINGLE FAMILY DETACHED HOUSING – I.T.E. CODE 210)										
	SINGLE FAMILY DETACHED HOUSING – 102 UNITS										
	Total	%	%	Number	Number						
	New Trips	Entering	Exiting	Entering	Exiting						
Weekday	1058	50%	50%	529	529						
A.M. Peak	81	25%	75%	20	61						
P.M. Peak 107 63% 37% 67 40											

### Trip Distribution

FIGURE 5 provides a summary of the trip distribution patterns developed for the study intersections, which were derived from the existing traffic patterns. In addition, FIGURE 6 provides the generated traffic volumes as assigned to the local roadway network in accordance with these distribution patterns. FIGURE 7 shows the combined year 2017 volumes reflecting the existing traffic, the background traffic growth, and the newly generated traffic from the Copper Trace Development. These are the volumes used in the analysis of full build-out conditions.



FIGURE 5: TRIP DISTRIBUTION



FIGURE 6: GENERATED TRIPS



FIGURE 7: 2017 COMBINED TRAFFIC DATA

### Proposed Level-of-Service Evaluation

Intersection Capacity Analyses employing the methods of the Highway Capacity Manual were used to evaluate the intersection of Emory Road and Copper Ridge Road, for the year 2017 combined traffic volume conditions (FIGURE 7). The results indicate that with existing intersection turn lane geometry, the Emory Road left-turn traffic movement would be expected to operate at level-of-service "B" during the AM peak hour and "A" during the PM peak hour, and Copper Ridge Road movements would operate at a level-of-service "E" during the AM peak hour and "B" during the PM peak hour. These results are summarized in detail on the "Two-Way Stop Control Summary" printouts contained in the APPENDIX. The APPENDIX may also be referenced for a discussion of intersection capacity and level-of-service concepts.

### Intersection Sight Distance and Other Issues

A field review was conducted to identify any sight distance restrictions, geometric deficiencies or other issues of concern that could impact the proposed development. The results of this review are summarized below:

### 1) Intersection Corner Sight Distance:

The proposed project development entrance on Copper Ridge Road was evaluated for corner sight distance. Based on the posted 30 mph speed limit, the required minimum sight distance in accordance with Knox County regulations would be 300 feet. Field reviews indicate that this requirement will be met at this intersection, as available sight distance was measured in excess of 450 feet looking south and approximately 385 feet looking north. The sight distance to the north was measured looking through a horizontal curve and chain link fence.



Looking south from intersection along Copper Ridge Road



Looking north from intersection along Copper Ridge Road

2) Auxiliary Lanes for Proposed Development Intersection:

Turn lane warrant analyses were conducted for the intersection of Emory Road and Copper Ridge Road under proposed development conditions. These analyses employed Tables 5A and 5B from the <u>Knox County Access Control and Driveway Design Policy</u>, which are based on turn lane warrants developed by Harmelink. The results were that an eastbound left-turn lane on Emory Road at Copper Ridge Road is expected to be warranted during both peak traffic hours. As a basis of comparison, existing traffic conditions were also analyzed, with the result that the eastbound left-turn lane currently satisfies warranting conditions for the P.M. peak. A westbound right-turn lane on Emory Road was not found to be warranted. Copies of Tables 5A and 5B are located in the APPENDIX for review.

### CONCLUSIONS AND RECOMMENDATIONS

A primary conclusion of this study is that the traffic generated by the proposed development will result in short-term impacts on traffic operational conditions in the project area. Of particular concern is the existing intersection of Emory Road and Copper Ridge Road, through which the vast majority of traffic to this development will travel. The issues at this location include the fact that an eastbound left-turn lane will be clearly warranted during both A.M. and P.M. peak traffic hours, and during the A.M. peak hour the southbound traffic will experience level-of-service "E" operation. TABLE 3 summarizes all intersection capacity evaluations conducted for this study, including possible improvement alternative scenarios.

TABLE 3											
CAPACITY ANALYS	CAPACITY ANALYSES SUMMARY										
	LEVEL-OF-S	ERVICE									
EVALUATION CONDITION	LAY)										
	Southbound	Eastbound									
	(Copper Ridge Rd)	(Emory Rd)									
Existing (2012) – AM	C (22.5)	A (9.8)									
Existing (2012) – PM B (11.4) A (7.9)											
Background (2017) – AM	D (27.4)	B (10.2)									
Background (2017) – PM	B (11.8)	A (8.0)									
Combined w/ Existing Lanes – AM	E (43.3)	B (10.3)									
Combined w/ Existing Lanes - PM	B (14.9)	A (8.2)									
Combined w/ EBLT & SBRT – AM	D (26.9)*	B (10.3)									
Combined w/ EBLT & SBRT – PM	Combined w/ EBLT & SBRT – PM B (14.1)* A (8.2)										
* Southbound Breakdown by Lane:											
AM - Left-turn lane $- E(35.8)$ , Right-turn lane $- C(23.8)$											
PM – Left-turn lane – D(29.0), Right-turn lane – B(10.3)											

The above issues and evaluations indicate that consideration should be given to possible improvements to the intersection of Emory Road and Copper Ridge Road. This study indicated that some justification exists for both an eastbound left-turn lane and a southbound second approach lane. It is recommended that the following factors be considered when determining the scope of actual improvements to be required and how responsibility for the improvements is to be assigned:

- 1. Although an eastbound left-turn lane is anticipated to be justified under full project build-out traffic conditions, it is noteworthy that such a lane is <u>currently</u> justified during existing P.M. peak hour conditions.
- 2. The southbound capacity analyses of unimproved full build-out conditions identified level-ofservice "E" operation for only one peak hour.
- 3. Existing roadway right-of-way on both Emory Road and Copper Ridge Road at this intersection appear to be on the order of 50 feet, and are not under the control of the project developer.

Other traffic related issues evaluated for this project included corner sight distance for the proposed subdivision access roadway intersection with Copper Ridge Road, and the general width and condition of Copper Ridge Road between Emory Road and the project site. These evaluations concluded that corner sight distance requirements will be exceeded, and that although Copper Ridge Road is narrow, its width does meet or exceed the eighteen feet minimum required by Knox County.

APPENDIX

### Intersection Capacity and Level of Service Concepts

In a general sense, a roadway is similar to a pipeline or other material-carrying conduit in that it has a certain capacity for the amount of material (vehicles) that it can efficiently carry. As the number of vehicles in a given time period gradually increases, the quality of traffic flow gradually decreases. On roadway sections this results in increasing turbulence in the traffic stream, and at intersections it results in increasing stops and delay. As the volumes begin to approach the capacity of the facility, these problems rapidly magnify, with resulting serious levels of congestions, stops, delay, excess fuel consumption, pollutant emissions, etc.

The Federal Highway Administration has published the <u>Year 2010 Highway Capacity Manual</u> (<u>HCM2010</u>), which establishes theoretical techniques to quantify the capacity conditions on all types of roadways, intersections, ramps, pedestrian facilities, etc. A basic concept that is applicable to most of these techniques is the idea of level of service (LOS). This concept establishes a rating system that quantifies the quality of traffic flow, as perceived by motorists and/or passengers. The general system is similar to a school grade scale, and is outlined as follows:

<u>Level of</u> Service(LOS)	<u>General Quality of</u> <u>Traffic Flow</u>	Description of Corresponding Conditions								
А	Excellent	Roadways – Free flow, high maneuverability Intersections – Very few stops, very low delay								
В	Very Good	Roadways – Free flow, slightly lower maneuverability Intersections – Minor stops, low delay								
С	Good	Roadways – Stable flow, restricted maneuverability Intersections – Significant stops, significant delay								
D	Fair	Roadways – Marginally stable flow, congestion seriously restricts maneuverability Intersections – High stops, long but tolerable delay								
Е	Poor	Roadways – Unstable flow*, lower operating speeds, congestion severely restricts maneuverability Intersections – All vehicles stop, very long queues and very long intolerable delay								
F	Very Poor	Roadways – Forced flow, stoppages may be lengthy, congestion severely restricts maneuverability Intersections – All vehicles stop, extensive queues and extremely long intolerable delay								

\*Unstable flow is such that minor fluctuations or disruptions can result in rapid degradation to LOS F.

### Cannon & Cannon, Inc. Consulting Engineers - Field Surveyors 8550 Kingston Pike Knoxville, TN 37919

CCI Project Name: Copper Trace TIS CCI Project Number: 1048-0000 Intersection: Emory @ Copper Ridge Counted By: CCI

### File Name : Emory\_Copper Ridge\_10-25-12 Site Code : 00000001 Start Date : 10/25/2012 Page No : 1

000000000000000000000000000000000000000								G	roups I	Printed-	1 - Uns	shifted									
		Coppe	r Rida	e Road	1		En	nory R	oad								En	nory R	oad		
		Sc	uthbo	und			W	lestbou	und			No	rthbou	Ind			E	<u>astbou</u>	ind		
Clark Time	1.04	Thr	Rig	Ped	App.	Loft	Thr	Rig	Ped	App.	Left	Thr	Rig	Ped	App.	Left	Thr	Rig	Ped	App.	Int.
Start line	Len	u	ht	S	Total	Len	u	ht	S	Total		u	ht	\$	Iotal	4.0	<u> </u>	nt	<u> </u>	Total	Totai
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.01	25	200
07:00 AM	3	0	6	0	9	0	162	0	0	162	0	0	0	0	0	2	33	0	0	30	200
07:15 AM	5	0	18	0	23	0	206	0	0	206	0	0	0	0	0	3	49	0	0	02	242
07:30 AM	7	0	9	0	16	0	245	1	0	246	0	0	0	0		1	/4	0	0	01	040 070
07:45 AM	4	0	22	0	26	0	<u>    161                               </u>	0	0	161	<u> </u>	0	<u> </u>	0		15	02	0	0	253	1102
Total	19	0	55	0	74	0	774	1	0	//5]	U	0	U	0	U	10	230	U	0	200	1102
08:00 AM	5	0	10	0	15	0	178	3	0	181	0	0	0	0	0	6	46	0	0	52	248
08:15 AM	5	0	9	0	14	0	110	1	0	111	0	0	0	0	U Q	2	61	0	0	03	100
08:30 AM	2	0	3	0	5	0	116	2	0	118	0	0 0	0	0	0	2	43	0	0	40	100
08:45 AM	3	0	6	0	9	0	107	2	0	109	0	0	0	0	0	4	39	<u> </u>	0	40	765
Total	15	0	28	0	43	0	511	8	0	519	0	U	U	U	U	14	109	U	U	203	705
*** BREAK *	**																				
04:00 PM	2	0	7	0	9	0	56	2	0	58	0	0	0	0	0	6	117	0	0	123	190
04:15 PM	0	Ó	3	0	3	0	62	1	0	63	0	0	0	0	0	8	105	0	0	113	179
04:30 PM	3	0	10	0	13	0	54	1	0	55	0	0	0	0	0	13	142	0	0	155	223
04:45 PM	1	0	1	0	2	0	56	3	0	59	0	0	0	0	0	15	152	0		167	228
Total	6	0	21	0	27	0	228	7	0	235	0	0	0	0	0	42	516	0	0	558	820
05:00 PM	1	0	5	0	6	0	59	3	0	62	0	0	0	0	0	11	156	0	0	167	235
05:15 PM	1	0	8	0	9	0	62	4	0	66	0	0	0	0	0	10	174	0	0	184	259
05:30 PM	Ó	0	4	0	4	0	76	6	0	82	0	0	0	0	0	17	162	0	0	179	265
05:45 PM	1	0	8	0	9	0	74	0	0	74	0	0	0	0	0	16	118		0	134	21/
Total	3	0	25	0	28	0	271	13	0	284	0	0	0	0	0	54	610	0	0	664	976
Grand Total	43	0	129	0	172	0	178 4	29	0	1813	0	0	0	0	0	125	155 3	0	0	1678	3663
Apprch % Total %	25.0 1.2	0.0 0.0	75.0 3.5	0.0 0.0	4.7	0.0	98.4 48.7	1.6 0.8	0.0 0.0	49.5	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0	7.4 3.4	92.6 42.4	0.0 0.0	0.0 0.0	45.8	

### Cannon & Cannon, Inc. Consulting Engineers - Field Surveyors 8550 Kingston Pike Knoxville, TN 37919

CCI Project Name: Copper Trace TIS CCI Project Number: 1048-0000 Intersection: Emory @ Copper Ridge Counted By: CCI

File Name : Emory\_Copper Ridge\_10-25-12 Site Code : 00000001 Start Date : 10/25/2012 Page No : 2

		Coppe	r Ridg	e Road	ł		En	ory Re	bad			No	orthbou	Ind			Em Ea	ory Ro	oad nd		
Start Time	Left	Thr	Rig	Ped	App.	Left	Thr	Rig	Ped	App. Total	Left	Thr	Rig	Ped	App. Total	Left	Thr	Rig ht	Ped s	App. Total	int. Total
Peak Hour Fi Intersectio	rom 07 07:15	00 AN	1 to 08	:45 AN	1 - Peak	1 of 1	<u> </u>			Total	<u> </u>	<u> </u>			Total	3	<u> </u>		I		
n Volume Percent	21 26.3	0 0.0	59 73.8	0 0.0	80	0 0.0	790 99.5	4 0.5	0 0.0	794	0 0.0	0 0.0	0 0.0	0 0.0	0	19 7.0	251 93.0	0 0.0	0 0.0	270	1144
07:30 Volume Peak	7	0	9	0	16	0	245	1	0	246	0	0	0	0	0	7	74	0	0	81	343 0.834
High Int. Volume Peak Factor	07:45 4	AM 0	22	0	26 0.769	07:30 0	AM 245	1	0	246 0.807	6:45:0 0	0 AM 0	0	0	0	07:45 3	AM 82	0	0	85 0.794	
									Co Out 2:	opper Rid	ge Road To 30	tal 103]									
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			Total	1119						1 Nor	h				4	Righ					
			mory Road In	270	1 1 Thru L				10/25 10/25	/2012 7:1 /2012 8:0	5:00 AM 0:00 AM				•	4  790 It Thru F	794	Emory Road			
			o G	849	Peds				<u>1-U</u>	<u>nshifted</u>						oeds	1066				

# Cannon & Cannon, Inc. Consulting Engineers - Field Surveyors 8550 Kingston Pike Knoxville, TN 37919

File Name : Emory\_Copper Ridge\_10-25-12 Site Code : 00000001 Start Date : 10/25/2012

App. Total

S

Int.

Total

Counted By: (			•	Ū												Page N	lo :3		
		Coppe	er Ridg	e Roa	d		En	nory R /estboi	oad			No	orthbo	und			En Ei	iory F astbo	toad und
Start Time	Left	Thr	Rig ht	Ped s	App. Total	Left	Thru	Rig ht	Ped s	App. Total	Left	Thr u	Rig ht	Ped s	App. Total	Left	Thr u	Rig ht	Ped s
Peak Hour Fi Intersectio	rom 04 04:45	:00 PN PM	/ to 05	:45 PN	/l - Peak	1 of 1			•										

n	04.45	1.141																			
Volume	3	0	18	0	21	0	253	16	0	269	0	0	0	0	0	53	644	0	0	697	987
Percent	14.3	0.0	85.7	0.0		0.0	94.1	5.9	0.0		0.0	0.0	0.0	0.0		7.6	92.4	0.0	0.0		
05:30	n	0	4	0	4	0	76	6	0	82	0	0	0	0	0	17	162	0	0	179	265
Volume	v	v	-1	v	•	ľ		·	•	· · ·	-	•									
Peak																					0.931
Factor																					
High Int.	05:15	PM				05:30	) PM									05:15	PM				
Volume	1	0	8	0	9	0	76	6	0	82	0	0	0	0	0	10	174	0	0	184	
Peak					0.583					0.820										0.947	
Factor					0.000					0.020											



CCI Project Name: Copper Trace TIS CCI Project Number: 1048-0000 Intersection: Emory @ Copper Ridge

## Results for Station 47 in Knox County, TN

# Station #CountyLocationRoute #000047KnoxNEAR ANDERSON CO LINESR131

Record	Year	AADT						
1	2011	9006						
2	2010	9512						
3	2009	9426						
4	2008	8792						
5	2007	9077						
6	2006	8872						
7	2005	9140						
8	2004	8467						
9	2003	8368						
10	2002	7948						
11	2001	7419						
12	2000	7819						
13	1999	7541						
14	1998	6908						
15	1997	6865						
16	1996	6427						
17	1995	5941						
18	1994	5995						
19	1993	5993						
20	1992	5370						
21	1991	5283						
22	1990	4554						
23	1989	4910						
24	1988	4542						
25	1987	4514						
26	1986	4261						
27	1985	3919						

5- Yes- growth: 9006- 8872 (Itx) 5 x- 0.3%

# Single-Family Detached Housing (210)

## Average Vehicle Trip Ends vs: Dwelling Units On a: Weekday

Number of Studies: 351 Avg. Number of Dwelling Units: 197 Directional Distribution: 50% entering, 50% exiting

## **Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation			
9.57	4.31 - 21.85	3.69			

### **Data Plot and Equation**



Institute of Transportation Engineers

(2	:10)
Average Vehicle Trip Ends vs: On a:	Dwelling Units Weekday, Peak Hour of Adjacent Street Traffic One Hour Between 7 and 9 a.m.
Number of Studies:	286
Avg. Number of Dwelling Units:	194
Directional Distribution:	25% entering, 75% exiting

## **Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation				
0.75	0.33 - 2.27	0.90				

## **Data Plot and Equation**



Trip Generation, 8th Edition

Institute of Transportation Engineers

Single-Family Detached Housing (210)							
Average Vehicle Trip Ends vs: On a:	Dwelling Units Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.						
Number of Studies: Avg. Number of Dwelling Units:	314 208						
Directional Distribution:	63% entering, 37% exiting						

### **Trip Generation per Dwelling Unit**

Average Rate	Range of Rates	Standard Deviation
1.01	0.42 - 2.98	1.05

## **Data Plot and Equation**



Trip Generation, 8th Edition

Institute of Transportation Engineers

## **TRIP GENERATION**

COPPER TRACE DEVELOPMENT

ITE TRIP GENERATION (210) - SINGLE FAMILY DETACHED HOUSING

### 102 DWELLING UNITS

### <u>WEEKDAY</u>

LN(T) = 0.92\*LN(X) + 2.71 T = 1058 50% ENTERING = 529 50% EXITING = 529

## AM PEAK

T = 0.70\*(X) + 9.74 T = 81 25% ENTERING = 20

75% EXITING = 61

### <u>PM PEAK</u>

LN(T) = 0.90\*LN(X) + 0.51 T = 107 63% ENTERING = 67 37% EXITING = 40

	TWO	D-WAY STOP	CONTR	ol s	UM	MARY				
General Informatio	on		Site I	nforn	nati	on				
Analyst	BJH		Interse	ection			Emory @	Copp	er R	idge
Agency/Co.	Cannon	& Cannon, Inc.	Jurisdi	ction			Knox Col	unty		
Date Performed	10/31/20	12	Analys	sis Yea	ar		Existing 2	2012		
Analysis Time Period	AM Peak	(		•••						
Project Description C	Copper Trace T	'IS								
East/West Street: Emo	ory Road		North/S	South	Stree	et: Copp	er Ridge Ro	oad		
Intersection Orientation	: East-West	NUM 19 11	Study	Period	l (hrs	): 0.25				
Vehicle Volumes a	nd Adjustn	nents		• • • •						:
Major Street		Eastbound					Westbound			
Movement	1	2	3			4	5			6
	L	Т	R			L	<u> </u>			R
Volume (veh/h)	19	251					790			4
Peak-Hour Factor, PHF	0.90	0.90	1.00			1.00	0.90		0	.90
Hourly Flow Rate, HFR (veh/h)	21	278	0	0 0		877			4	
Percent Heavy Vehicles	s 0			0						
Median Type			Undivided							
RT Channelized			0							0
Lanes	0	1	0	0		0	1			0
Configuration	LT								TR	
Upstream Signal		0					0			
Minor Street		Northbound					Southbou	Ind		
Movement	7	8	9			10	11			12
	L	Т	R	R		L	Т	1		R
Volume (veh/h)						21			59	
Peak-Hour Factor, PHF	F 1.00 1.00		1.00			0.90	1.00		0	.90
Hourly Flow Rate, HFR (veh/h)	0	0	0			23	0		65	
Percent Heavy Vehicle	s 0	0	0			0	0			0
Percent Grade (%)		0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0							0
Lanes	0	0	0			0	0		. ,	0
Configuration							LR			
Delay, Queue Length.	and Level of	Service				······				
Approach	Eastbound	Westbound		lorthb	ounc	1	S	outhbo	und	
Movement	1	4	7	8		9	10	11		12
Lane Configuration	17		· · · · · · · · · · · · · · · · · · ·					IR		
Lane Comgaration	21							88		
$\nabla \left( v \in \Pi/\Pi \right)$	776		<u> </u>				1	202	,	,
	770							293	<u></u>	
V/C	0.03							0.30	<u></u>	
95% queue length	0.08			<u> </u>				1.23	5	
Control Delay (s/veh)	9.8			ļ				22.8	5	
LOS	Α							C		<u></u>
Approach Delay (s/veh)								22.5		
Approach LOS								С		

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	TWO	-WAY STOP	CONTRO	OLS	UMI	MARY			
General Informatio	n		Site Ir	nforn	nati	on			
Analyst	BJH		Interse	ection			Emory @	Copper R	idge
Agency/Co.	Cannon &	Cannon, Inc.	Jurisdi	ction			Knox Cou	inty	
Date Performed	10/31/201	12	Analys	sis Yea	ar		Existing 2	012	
Analysis Time Period	PM Peak								
Project Description C	opper Trace Tl	S							
East/West Street: Emo	ory Road		North/S	South	Stree	et: Coppe	r Ridge Ro	ad	
Intersection Orientation	East-West		Study I	Period	(hrs	): 0.25			
Vehicle Volumes a	nd Adjustm	ents							
Major Street		Eastbound					Westbour	nd	
Movement	1	2	3			4	5		6
		T	<u> </u>				050		K 16
Volume (veh/h)	53	644	1.00			1.00	203		10
Peak-Hour Factor, PHF	0.93	0.93	1.00			1.00	0.95	<u> </u>	.95
(veh/h)	56	692	0	0 0		272		17	
Percent Heavy Vehicles	s 0			0					
Median Type				Undiv	/idea	/			
RT Channelized			0	0					0
Lanes	0	1	0	0		0	1		0
Configuration	LT								TR
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	nd	
Movement	7	8	9			10	11		12
	· L	Т	R			L	Τ		R
Volume (veh/h)			1.00			3	( 00		18
Peak-Hour Factor, PHF	1.00	1.00	1.00			0.93	1.00		.93
Hourly Flow Rate, HFR (veh/h)	0	0	0		3		0		19
Percent Heavy Vehicle	s 0	0	0			0	0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			0	0		0
Configuration							LR		
Delay, Queue Length,	and Level of	Service							
Approach	Eastbound	Westbound	1	Northb	ounc	1	S	outhbound	
Movement	1	4	7	8		9	10	11	12
Lane Configuration	LT							LR	
v (veh/h)	56							22	
C (m) (veh/h)	1284							581	
v/c	0.04							0.04	
95% queue length	0.14							0.12	
Control Delay (s/veh)	7.9							11.4	
LOS	A							В	
Approach Delay (s/veh)								11.4	
Approach LOS								В	

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	TWO	-WAY STOP	CONTR	OL S	UM	MARY				-
General Informatio	on		Site I	nforn	nati	on				
Analyst	BJH		Interse	ection			Emory @	Copp	er Ric	dge
Agency/Co.	Cannon &	& Cannon, Inc.	Jurisd	iction			Knox Cou	ınty		
Date Performed	10/31/20	12	Analys	sis Yea	ar		Backgrou	ind 20	17	
Analysis Time Period	AM Peak									
Project Description C	Copper Trace T	IS		·						
East/West Street: Eme	ory Road	· · · · · · · · · · · · · · · · · · ·	North/S	South	Stree	et: Coppe	er Ridge Ro	bad		
Intersection Orientation	East-West	· · · · · · · · · · · · · · · · · · ·	Study	Period	(hrs	): 0.25				
Vehicle Volumes a	<u>ınd Adjustr</u>	nents								
Major Street		Eastbound					Westbound			_
Movement	1	<u>2</u>	$\frac{3}{5}$			4	5 T			5
Volume (voh/h)	L	276	<u>к</u>				1 1			۲ ۱
Peak-Hour Factor PHF	0.90	0.90	1 00			1 00	009		0.9	90
Hourly Flow Rate, HFR	23	306	0			0	965		4	1
Percent Heavy Vehicles	s 0					0			-	-
Median Type	-			Undi	/idec	1				
RT Channelized		0 0						)		
Lanes	0	1	0	0		0	1	<u> </u>	(	)
Configuration	LT								T	R
Upstream Signal		0					0			
Minor Street		Northbound					Southbou	ind		
Movement	7	8	9	9		10	11		1	2
	L	Т	R			L	τ		R	
Volume (veh/h)						23			6	5
Peak-Hour Factor, PHF	1.00	1.00	1.00	)		0.90	1.00		0.	90
Hourly Flow Rate, HFR (veh/h)	0	0	0		25		0		72	
Percent Heavy Vehicle	s 0	0	0			0	0			)
Percent Grade (%)		0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0						(	)
Lanes	0	0	0			0	0		(	)
Configuration							LR			
Delay, Queue Length,	and Level of	Service								
Approach	Eastbound	Westbound	ł	Northb	ounc	1	S	outhbo	ound	
Movement	1	4	7	8		9	10	11	1	12
Lane Configuration	LT							LR	2	
v (veh/h)	23			1				97	·	
C (m) (veh/h)	719							256	3	··· · · · · · · · ·
v/c	0.03							0.3	8	
95% aueue lenath	0.10							1.6	9	
Control Delay (s/veh)	10.2		L					27	4	
	B			<b> </b>				 D		
Approach Delay				I		L		27.4	1	
Approach LOS								D		
r										

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	TWO	-WAY STOP	CONTR	OL SI	JMI	MARY			
General Informatio	on		Site II	nform	ati	on			
Analyst	BJH	·····	Interse	ection			Emory @	Copper	Ridge
Agency/Co.	Cannon &	& Cannon, Inc.	Jurisdi	ction			Knox County		
Date Performed	10/31/201	12	Analys	is Yea	r		Backgrou	nd 2017	
Analysis Time Period	PM Peak								
Project Description C	opper Trace T	IS							
East/West Street: Emo	ory Road		North/S	South S	tree	et: Coppe	er Ridge Ro	bad	
Intersection Orientation	: East-West		Study I	Period	(hrs	): 0.25			
Vehicle Volumes a	ind Adjustm	ients							
Major Street		Eastbound					Westbou	nd	~
Movement	1	2	3			4	5		6
	L	T	<u>R</u>				070		<u> </u>
Volume (veh/h)	58	708	1.00			1.00	270		10
Peak-Hour Factor, PHF	0.93	0.93	1.00	1.00		1.00	0.93		0.95
(veh/h)	62	761	0			0	298		19
Percent Heavy Vehicles	s 0		0						
Median Type				Undiv	idea				
RT Channelized			0	0					0
Lanes	0	1	0	0		0	1		0
Configuration	LT								TR
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	Ind	
Movement	7	8	9			10	11		12
	L	T	R			L	T		R
Volume (veh/h)						3			20
Peak-Hour Factor, PHF	r Factor, PHF 1.00 1		1.00			0.93	1.00		0.93
Hourly Flow Rate, HFR (veh/h)	0	0	0			3	0		21
Percent Heavy Vehicle	s 0	0	0			0	0		0
Percent Grade (%)		0				· · · · · · · · · · · · · · · · · · ·	0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			0	0		0
Configuration							LR		
Delay, Queue Length,	and Level of	Service							
Approach	Eastbound	Westbound	1	Vorthbo	ound		S	outhbou	nd
Movement	1	4	7	8		9	10	11	12
Lane Configuration	LT							LR	
v (veh/h)	62							24	
C (m) (veh/h)	1255							550	
v/c	0.05							0.04	
95% queue length	0.16							0.14	
Control Delay (s/veh)	8.0							11.8	
LOS	А						ļ	В	
Approach Delay (s/veh)								11.8	
Approach LOS								B	

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	TWO	-WAY STOP	CONTR	OL S	UMI	MARY					
General Informatio	n		Site I	nforn	nati	on					
Analyst	BJH		Interse	ection			Emory @	Copp	er Ric	dge	
Agency/Co.	Cannon &	& Cannon, Inc.	Jurisdi	ction			Knox County				
Date Performed	10/31/201	12	Analys	sis Yea	ar		Combined	d 2017	7		
Analysis Time Period	AM Peak										
Project Description C	opper Trace T	IS			<b>.</b> .						
East/West Street: Emo	ory Road		North/South Street: Copper Ridge Road								
Intersection Orientation	East-West	· · · · · · · · · · · · · · · · · · ·	Study I	Period	(nrs	): 0.25					
Vehicle Volumes a	nd Adjustm	ients			.,						
Major Street		Eastbound				4	Westbour				
Movement	1	2	3			4	5 T				
Volume (veh/h)	25	276	<u>к</u>				869		(	)	
Peak-Hour Factor PHF	0.90	0.90	1.00		· · · · ·	1.00	0.90		0.9	90	
Hourly Flow Rate, HFR (veh/h)	38	306	0			0	965		1	0	
Percent Heavy Vehicles	s 0					0	+-		-	~	
Median Type		•		Undiv	<i>ridea</i>	1					
RT Channelized			0						(	)	
Lanes	0	1	0	0		0	1		(	)	
Configuration	LT								TR		
Upstream Signal		0					0				
Minor Street		Northbound					Southbou	nd			
Movement	7	8	9			10	11		1	2	
	L	Т	R			L	T			R	
Volume (veh/h)						38			108		
Peak-Hour Factor, PHF	1.00	1.00	1.00			0.90	1.00		0.	90	
Hourly Flow Rate, HFR (veh/h)	0	0	0			42	0		120		
Percent Heavy Vehicles	s 0	0	0			0	0	, I	(	)	
Percent Grade (%)		0					0				
Flared Approach		N					N				
Storage		0					0		· · · · · · · · · · · · · · · · · · ·		
RT Channelized			0						(	2	
Lanes	0	0	0			0	0		(	)	
Configuration		}					<u>LR</u>				
Delay, Queue Length,	and Level of	Service						·			
Approach	Eastbound	Westbound	1	Vorthb	ound	1	S	outhbo	ound		
Movement	1	4	7	8		9	10	11		12	
Lane Configuration	LT			<u> </u>				LR			
v (veh/h)	38							162	2		
C (m) (veh/h)	716							248	3	····	
v/c	0.05							0.6	5		
95% queue length	0.17							4.09	9		
Control Delay (s/veh)	10.3							43.3	3		
LOS	В							E			
Approach Delay (s/veh)								43.3	}		
Approach LOS								Е			

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	TWC	-WAY STOP	CONTR	OLS	UMI	MARY				
General Informatio	n	·····	Site II	nform	nati	on				
Analyst	BJH		Interse	ction			Emory @	Coppe	ər Ric	dge
Agency/Co.	Cannon 8	& Cannon, Inc.	Jurisdi	ction			Κηοχ Coι	unty		
Date Performed	10/31/20	12	Analys	is Yea	ır		Combine	d 2017		
Analysis Time Period	PM Peak									
Project Description C	Copper Trace T	IS								
East/West Street: Emo	ory Road		North/S	South S	Stree	et: Coppe	er Ridge Ro	oad		
Intersection Orientation	: East-West		Study I	Period	(hrs	): 0.25				
Vehicle Volumes a	nd Adjustn	nents								
Major Street		Eastbound					Westbou	Westbound		
Movement	1	2	3			4	5		i	6
	L	T	R			L	T			R
Volume (veh/h)	105	708	( 00			4.00	278		3	5 00
Peak-Hour Factor, PHF	0.93	0.93	1.00			1.00	0.93		0.	93
Hourly Flow Rate, HFR (veh/h)	112	761	0			0	298		3	7
Percent Heavy Vehicles	s 0					0			-	-
Median Type			Undivided							
RT Channelized			0	0					(	)
Lanes	0	1	0	0 0		0	1		(	)
Configuration	LT									'R
Upstream Signal		0	"I_,							
Minor Street		Northbound	-				Southbou	ind		
Movement	7	8	9			10	11		12	
	L	Т	R			L	Т		<u>R</u>	
Volume (veh/h)						13			48	
Peak-Hour Factor, PHF	<u> </u>	1.00	1.00			0.93	3 1.00		0.93	
Hourly Flow Rate, HFR (veh/h)	0	0	0			13	0		51	
Percent Heavy Vehicle	s 0	0	0			0	0	0		2
Percent Grade (%)		0					0			
Flared Approach		N					N			
Storage		0					0			
RT Channelized			0						(	0
Lanes	0	0	0			0	0		(	0
Configuration							LR			
Delay, Queue Length,	and Level of	Service						c		
Approach	Eastbound	Westbound	N	lorthbo	ound	[	S	outhbo	und	
Movement	1	4	7	8		9	10	11		12
Lane Configuration	LT							LR		
v (veh/h)	112							64		
C (m) (veh/h)	1236							427		
v/c	0.09							0.15	;	
95% queue length	0.30			·		· · · · · · · · · · · · · · · · · · ·		0.52	2	
Control Delay (s/veh)	8.2							14.9		
LOS	A							В		
Approach Delay (s/veh)								14.9		
Approach LOS							В			

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	TWO	-WAY STOP	CONTRO	OLS	UM	MARY					
General Informatio	n		Site Ir	nforn	nati	on	<u> </u>				
Analyst	BJH		Interse	ection			Emory @	Сорр	ber Ri	idge	
Agency/Co.	Cannon &	& Cannon, Inc.	Jurisdi	ction			Knox Cou	inty			
Date Performed	10/31/201	12	Analys	is Yea	31		Combine	d 201	7		
Analysis Time Period	AM Peak										
Project Description C	opper Trace Ti	IS - Combined w	ith EB and	I SB T	urn l	lanes					
East/West Street: Emo	ory Road		North/S	South	Stree	et: Coppe	r Ridge Ro	bad		· · · · · · · · · · · · · · · · · · ·	
Intersection Orientation	: East-West		Study F	Period	(hrs	): 0.25					
Vehicle Volumes a	nd Adjustm	ents									
Major Street		Eastbound					Westbou	nd			
Movement	1	2	3			4	5			6	
		T	R				000			R Q	
Volume (veh/h)	35	2/6	1.00			1.00	009		0	<u>9</u> 00	
Hourly Flow Pote UEP	0.90	0.90	1.00			1.00	0.90		0.		
(veh/h)	38	306	0			0	965			10	
Percent Heavy Vehicles	3 0					0					
Median Type				Undiv	∕idec	1					
RT Channelized			0							0	
Lanes	1	1	0			0	1			0	
Configuration	L	T							7	ſŔ	
Upstream Signal		0					0				
Minor Street		Northbound					Southbou	Ind			
Movement	7	8	9			10	11		12		
	L	Т	R			L	T		R		
Volume (veh/h)						38			108		
Peak-Hour Factor, PHF	1.00	1.00	1.00			0.90	1.00	1.00		0.90	
Houriy Flow Rate, HFR (veh/h)	0	0	0			42	0		1	20	
Percent Heavy Vehicle	s 0	0	0			0	0		0		
Percent Grade (%)		0					0	_			
Flared Approach		N					N				
Storage		0					0				
RT Channelized			0							0	
Lanes	0	0	0			1	0			1	
Configuration						L				R	
Delay, Queue Length.	and Level of	Service									
Approach	Eastbound	Westbound	Ν	lorthb	ounc	1	S	outhb	ound		
Movement	1	4	7	8		9	10	1	1	12	
Lane Configuration	L						L			R	
v (veh/h)	38						42			120	
C (m) (veh/h)	716						158			310	
v/c	0.05						0.27			0.39	
95% queue length	0.17						1.02			1.76	
Control Delay (s/veh)	10.3						35.8			23.8	
LOS	В						E			С	
Approach Delay (s/veh)								26.9	9		
Approach LOS								D			

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	TWO	-WAY STOP	CONTR	OL S	UMI	MARY			
General Informatio	n		Site II	nforn	nati	on			
Analyst	BJH	BJH Intersection			Emory @ Copper Ridge				
Agency/Co.	Cannon &	Cannon & Cannon, Inc.		Jurisdiction		Knox County			
Date Performed	10/31/201	2	Analysis Year			Combined 2017			
Analysis Time Period	PM Peak	PM Peak							
Project Description C	opper Trace T	S - Combined w	ith EB and	I SB T	<u>'urn L</u>	anes		-	
East/West Street: Emo	ory Road		North/South Street: Copper Ridge Road				ad		
Intersection Orientation	East-West		Study I	Period	(hrs	): 0.25			
Vehicle Volumes a	nd Adjustm	ents							
Major Street		Eastbound					Westbour	nd	
Movement	1	2	3			4	<u>5</u>		6
		1 700	К				070		<u>к</u> 25
Volume (ven/n)	105	708	1.00			1.00	270		30
Peak-Hour Factor, PHF	0.93	0.93	1.00			1.00	0.35		
(veh/h)	112	761	0			0	298		37
Percent Heavy Vehicles	3 0					0			
Median Type				Undiv	/idea				
RT Channelized			0				· · · · · · · · · · · · · · · · · · ·		0
Lanes	1	1	0			0	1		0
Configuration	L	Т							TR
Upstream Signal		0				•****	0		
Minor Street		Northbound				Southbound			
Movement	7	8	9		-	10	11		12
	L	T	R			L	Т		R
Volume (veh/h)				13		( 00		48	
Peak-Hour Factor, PHF	1.00	1.00	1.00			0.93	1.00		1.93
Hourly Flow Rate, HFR (veh/h)	0	0	0			13	0		51
Percent Heavy Vehicles	s <u> </u>	0	0			0	0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			1	0		1
Configuration						L			R
Delay, Queue Length,	and Level of	Service							
Approach	Eastbound	Westbound	1	Vorthb	ounc		Southboun		1
Movement	1	4	7	8		9	10	11	12
Lane Configuration	L						L		R
v (veh/h)	112						13		51
C (m) (veh/h)	1236						163		729
v/c	0.09			Ī			0.08		0.07
95% queue length	0.30						0.26		0.23
Control Delay (s/veh)	8.2						29.0		10.3
LOS	А						D		В
Approach Delay (s/veh)								14.1	
Approach LOS							В		

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# Emory Rood (SR 131) at Copper Ridge Rood

Eastbound Left. Turn Love Worrent Assessment

### TABLE 5A

## LEFT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

# (If the left-turn volume exceeds the table value a left -turn lane is needed)

OPPOSING	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *						
VOLUME	100'- 149	150 - 199	200 - 249	250-299	==300m349zz	\$50 - 399 \$5	
100 - 149	250	180	140	110	80.	70	
150 - 199	200	140	105	90	70	60	
2(1) - 24)	160	115	85	75	65	55	
	130	100	75	65	60	50	
300 - 300	110	90 80	70 65	60 55	55 50	45 40	
400 - 417	90	70	60	50	45	35	
	80	65	55	45	40	30	
500 - 500	70	60	45	35	35	25	
	65	55	40	35	30	25	
6001-6-19	60	45	35	30	25	25	
	55	35	35	30	25	20	
7001- 749	50	35	30	25	20	20	
750 or More	45	35	25	25 <sup>1</sup> ,3	20	20	

OPPOSING	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *						
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 599	⊭/>600	
*100 - 149 150 - 199	70 60	60 55	50 45	45 40	40 35	35 30	
200 - 249 250 - 299	55 50	50 45	40 35	35 30	30 30	30 30 <sup>2</sup> -	
300 - 349	45	40 35	35 30	30 25	25 25	25) ¥ 20	
400 - 449	35	30 25	30 25	25 20	2.0 20	20 20	
500 - 549	25	25 20	20 20	20 20	20 20	15 15	
600 - 649 600 - 600	25	20 20	20 20	20 20	20 20	15 15	
700 - 749	20 20 20	20 20	20 20	15 15	15 15	15 15	

+ Or through volume only if a right-turn lane exists 1- Existing (2012) A.M. Pedy - Volume - 19, Required - 25 - Not Warranted 2- Existing (2012) P.M. Peak- Volume : 53, Requirez: 30 - Warranted 3- Combined (2017) A.M. Pech- Volume: 35, Requires: 25- Warrantel 4- Combined (2017) P. M. Ped- Volume- 105, Required - 25- Warranted

Emory Rock (SR 131) at Copper Ridge Rock

Westbound Right-Turn Lone Worrant Assessment

### TABLE 5B

## RIGHT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *							
VOLUME	<100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399		
Fewer Than 25 25 - 49 50 - 99				*				
100 - 149 150 - 199								
200 - 249 250 - 299					Yes	Yes Yes		
30() - 349 350 - 399			Yes	Y'es Yes	Yes Yes	Yes Yes		
400 - 449 450 - 499	<u> </u>	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
500 - 549 550 - 599	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
600 or More	Yes	Yes	Yes	Yes	Yes	Yes		

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *						
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 600	+ / > 600	
Fewer Than 25 25 - 49 50 - 99				Yes	Y'es Yes	¥ Yes Yes	
100 - 149 150 - 199		Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
200 - 249 250 - 299	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
300 - 349 350 - 399	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
400 - 449 450 - 499	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
500 - 549 550 - 599	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	
600 or More	Yes	Yes	Yes	Yes	Yes	Yes	

\* Or through volume only if a left-turn lane exists.

\* The above based on Combined (2017) Traffic

Westbound Right-Turn Lone Not Warrantel