TOWERING OAKS VILLAGE

Traffic Impact Study Hatmaker Lane Knoxville, TN

A Traffic Impact Study for the Proposed Towering Oaks Village

Submitted to

Knoxville – Knox County Metropolitan Planning Commission

March 29, 2016 FMA Project No. 564.001

Submitted By:





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Executive Summary

New Destiny, LLC proposes a residential development with single family homes. The project is located south of N Campbell Station Road on Hatmaker Lane in Knox County. The development will consist of 75 single family homes in Phase I and 35 single family homes in Phase II for a total of 110 single family lots. Construction is proposed to take place this year and this study assumes full build out for the development will occur in 2019.

The driveway for the proposed development will tie into Hatmaker Lane east of the intersection of Hatmaker Lane and Hopper Lane. The proposed lane configuration is a single lane out of the development.

In order to maintain or provide an acceptable level-of-service for each of the intersections studied, the following recommendations are presented:

N Campbell Station Road @ Fretz Road

A westbound left turn lane is warranted during the PM Peak hour. Per AASHTO "A Policy on Geometric Design of Highways and Streets" the recommended storage length is two car lengths (approximately 50 feet) and the recommended taper length is 100 feet.

The intersection will continue to operate at a LOS B or higher after the completion of the Towering Oaks Village.

Fretz Road

The width of Fretz Road between the intersection with Hatmaker Lane and the intersection with Woodhollow Lane varies from approximately 14 feet to 22 feet. The recommended road width per AASHTO's "A Policy on Geometric Design of Highways and Streets" for a local road with a posted speed limit of 30 mph with ADT between 1,500 and 2,200 trips per day is 22 feet.

Hatmaker Lane @ Project Entrance

The proposed intersection geometry will be one lane entering and one lane exiting the subdivision. The project entrance is expected to operate at a LOS A during both the AM and PM peak hours.

1 Introduction

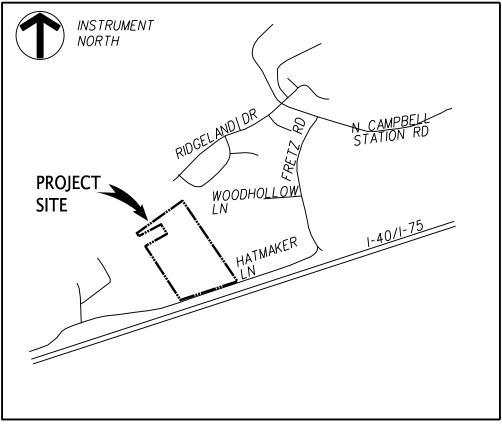
1.1 Project Description

This report provides a summary of a traffic impact study that was performed for the proposed Towering Oaks Village on Hatmaker Lane. The project site is located south of the intersection of Fretz Road and N Campbell Station Road in Knox County. The location of the site is shown in Figure 1.

The proposed Towering Oaks Village will consist of 75 single family lots in Phase I and approximately 35 single family lots in Phase II for a total proposed 110 single family lots. Full Buildout is expected to occur within three years, or by the year 2019. The proposed site layout is shown in Figure 2.

The purpose of this study is to evaluate the impacts to the traffic conditions caused by the development of the proposed subdivision.

FIGURE 1



LOCATION MAP
(NOT TO SCALE)

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FIGURE 1							
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LOCATION MAP

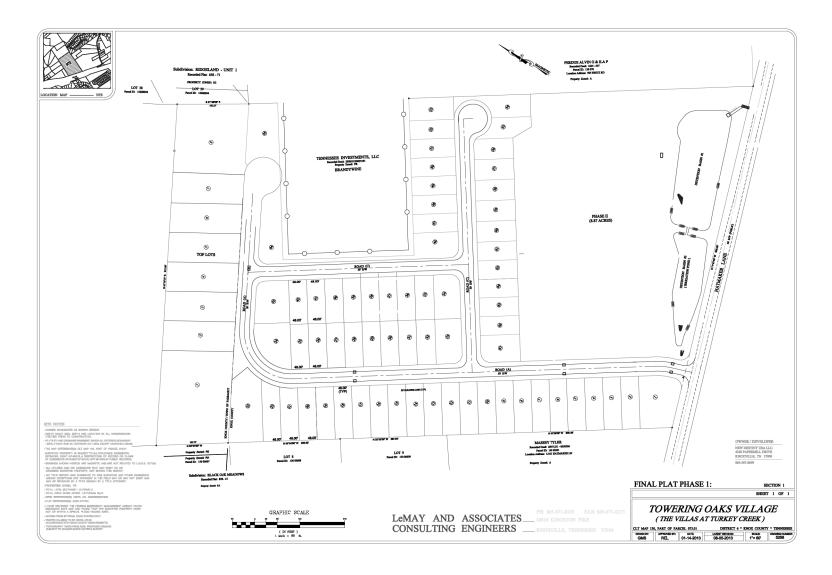
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KNOX COUNTY, TN



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FIGURE 2



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SITE PLAN

TOWERING OAKS VILLAGE
KNOX COUNTY, TN



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1.2 Existing Site Conditions

The proposed subdivision site access will tie into Hatmaker Lane approximately 1,850 feet east of the intersection of Hopper Lane and Hatmaker Lane and approximately 3,200 feet southwest of the intersection of Woodhollow Lane and Fretz Road.

During a site visit it was determined that Hatmaker Lane is a two-lane road with a width of 20-ft at the proposed project entrance. The posted speed limit on Hatmaker Lane is 30 mph. The Knoxville-Knox County Metropolitan Planning Commission does not list a classification for Hatmaker Lane per the Major Road Plan; therefore it is considered a local street. The intersection sight distance at the proposed driveway was measured to be in excess of 300-ft east and west of the intersection.

Fretz Road is a two-lane road with a posted speed limit of 30 mph. The width of Fretz Road between the intersection with Hatmaker Lane and the intersection with Woodhollow Lane varies from approximately 14 feet to 22 feet. The Knoxville-Knox County Metropolitan Planning Commission does not list a classification for Fretz Road per the Major Road Plan; therefore it is considered a local street.

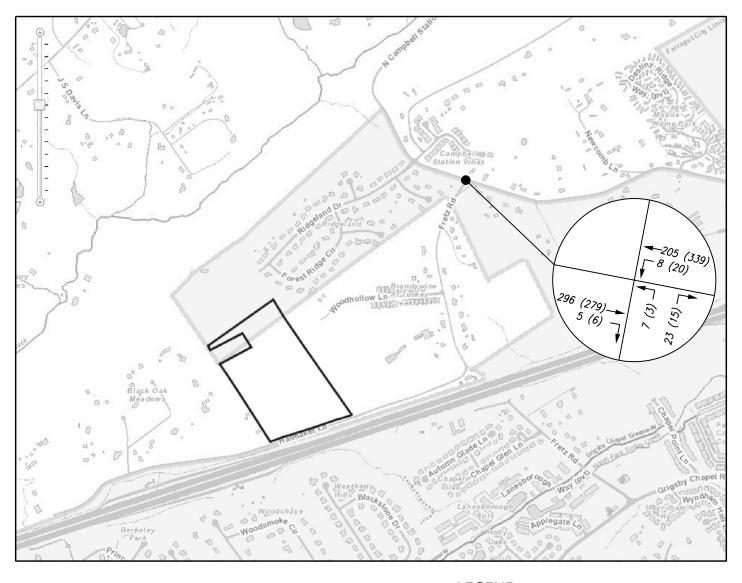
N Campbell Station Road is a two-lane road and has a posted speed limit of 35 mph. The Knoxville-Knox County Metropolitan Planning Commission classifies N Campbell Station Road as a minor arterial per the Major Road Plan.

2 Existing Traffic Volumes

FMA conducted an eight-hour turning movement count at the intersection of N Campbell Station Road and Fretz Road on Tuesday February 23, 2016. The weather conditions at the count location were intermittent light rain. The existing volume including the AM and PM peak hour traffic volumes at the count locations are shown in Figure 3 and the count data collected is included in Attachment 1.

The current AM peak hour, and PM peak hour were determined using the eighthour turning movement count that FMA conducted. The AM peak hour occurred between 7:30 am and 8:30 am and the PM peak hour occurred between 5:00 pm and 6:00 pm.





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TURNING MOVEMENT VOLUME AM (PM)

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2016 EXISTING PEAK HOUR TRAFFIC

TOWERING OAKS VILLAGE
KNOX COUNTY, TN



3 Background Growth

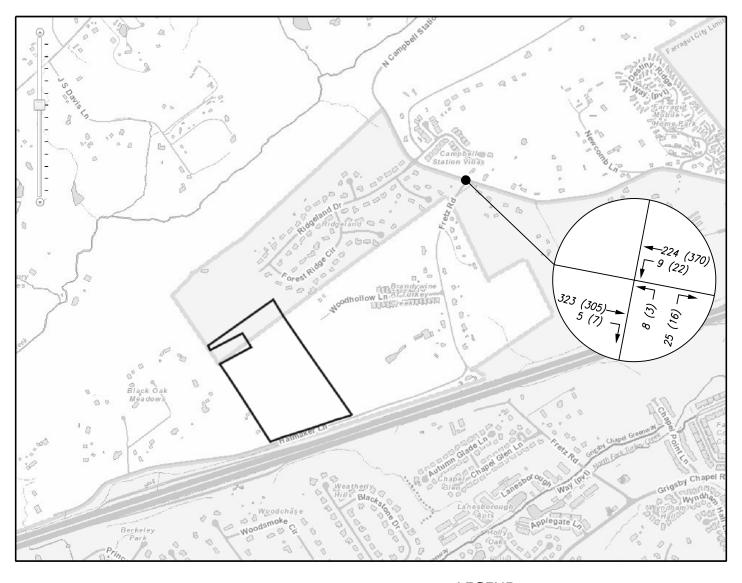
The Tennessee Department of Transportation (TDOT) maintains a count station on N Campbell Station Road south of Yarnell Road. The annual traffic growth rate for this station between 2000 and 2014 is approximately 1.88%.

The Transportation Planning Organization (TPO) maintains a count station on N Campbell Station Road north of Interstate 40. The annual traffic growth rate for this station between 2000 and 2014 is approximately 3.62%.

For the purpose of this study, an annual growth rate of 3.0% for traffic at the intersection of N Campbell Station Road and Fretz Road was assumed until full occupancy is reached in 2019.

Attachment 2 shows the trend line growth charts for the TDOT count station and the TPO count station. Figure 4 demonstrates the projected future peak hour volumes at the intersections after applying this background growth rate to the existing conditions.





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TURNING MOVEMENT VOLUME AM (PM)

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2019 BACKGROUND PEAK HOUR TRAFFIC TOWERING OAKS VILLAGE
KNOX COUNTY, TN



4 Trip Generation and Trip Distribution

Single-Family Detached Housing or Land Use 210 was used to calculate site trips for the proposed single family housing using the fitted curve equations from The *Trip Generation*, 9th *Edition*, published by the Institute of Transportation Engineers.

The total number of trips generated by the proposed Towering Oaks Village was estimated to be 1146 daily trips. During the peak hours the estimated trips are 87 trips during the AM peak hour and 114 trips during the PM peak hour.

A trip generation summary is shown in Table 4-1.

Table 4-1
Trip Generation Summary

Towering Oaks Village – Phase I & II
Single-Family Detached Housing (Land Use 210)

	Total New Trips	% Entering	%Exiting	Number Entering	Number Exiting
Weekday A.M. Peak	1146 87	50 25	50 75	573 22	573 65
P.M. Peak	114	63	37	72	42

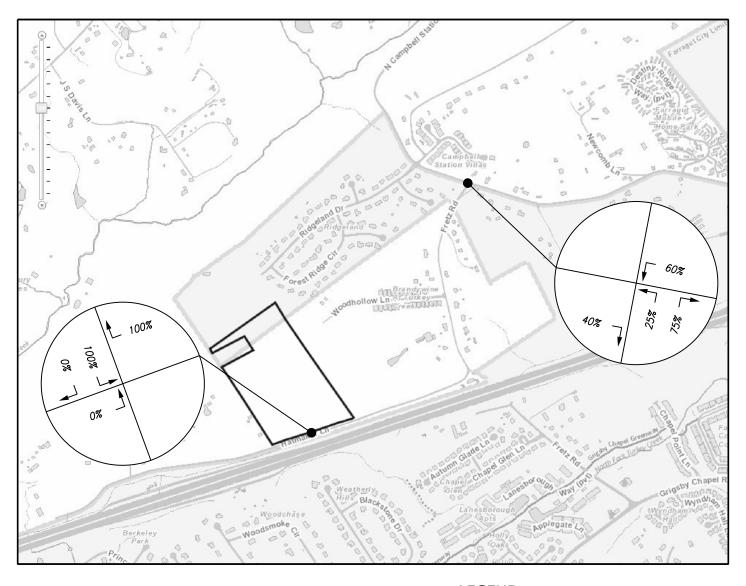
Towering Oaks Village Traffic Impact Study March 29, 2016

The directional distribution of the traffic generated by the proposed Towering Oaks Village was determined using the traffic data collected for the existing conditions. The typical weekday traffic pattern is for traffic to flow heavier in one direction in the morning peak period and then for the traffic to be heavier in the opposite direction during the evening peak period. Hatmaker Lane at the proposed Project Entrance has a trip distribution of 70% Eastbound and 30% Westbound during the AM peak hour and 40% Eastbound and 60% Westbound during the PM peak hour.

Fretz Road has a trip distribution for exiting traffic of 75% right turns and 25% left turns during the AM peak hour and 85% right turns and 15% left turns during the PM peak hour. N Campbell Station Road has a trip distribution for entering traffic onto Fretz Road of 40% Eastbound right turns and 60% Westbound left turns during the AM peak hour and 25% Eastbound right turns and 75% Westbound left turns during the PM peak hour. The trip distribution for the Towering Oaks Village is shown in Figure 5 and Figure 6.

Using the existing trip distribution the trips generated from the Towering Oaks Village are shown in Figure 7. Figure 8 shows the combined peak hour traffic from the background growth and the full build out of the Towering Oaks Village.





← 50% (50%)

TRIP DISTRIBUTION AM (PM)

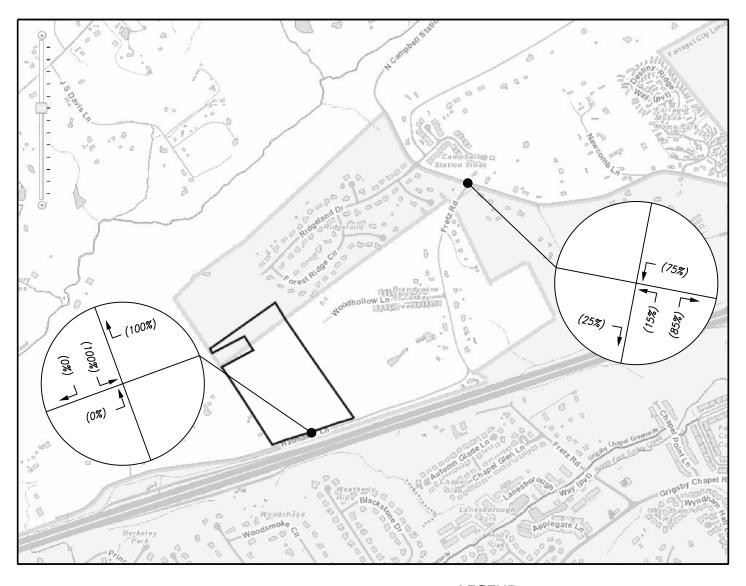
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FIGURE 5 No. Revision/Issue			Date				

AM PEAK HOUR TRIP DISTRIBUTION

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KNOX COUNTY, TN







← 50% (50%)

TRIP DISTRIBUTION AM (PM)

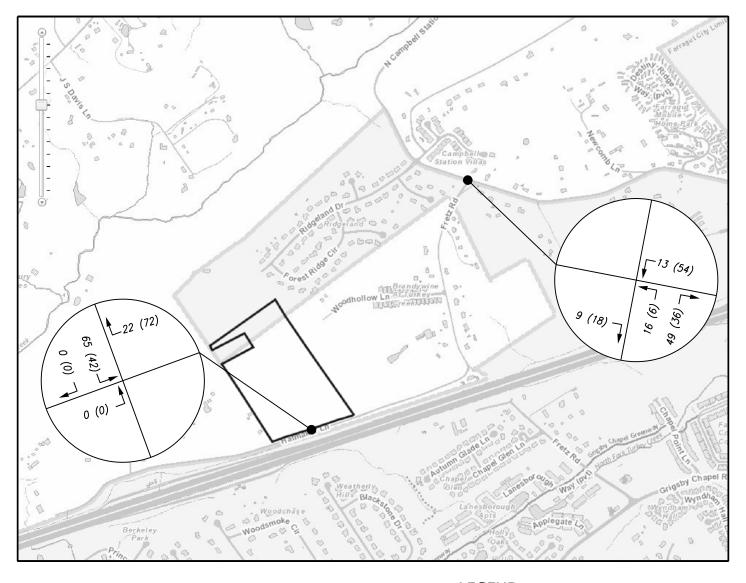
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PM PEAK HOUR TRIP DISTRIBUTION

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KNOX COUNTY, TN







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TURNING MOVEMENT VOLUME AM (PM)

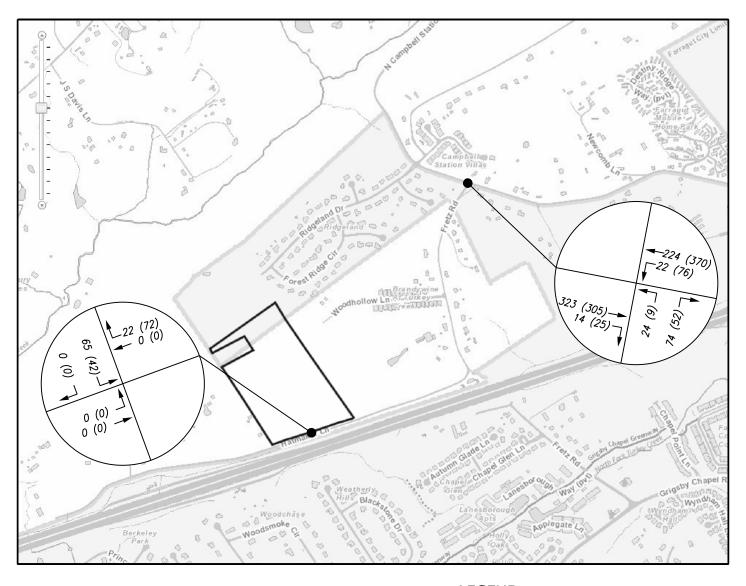
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PEAK HOUR SUBDIVISION TRAFFIC

TOWERING OAKS VILLAGE
KNOX COUNTY, TN







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TURNING MOVEMENT VOLUME AM (PM)

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FIGURE 8	No.	Revision/Is	ssue		Date

2019 PEAK HOUR TRAFFIC FULL BUILDOUT

TOWERING OAKS VILLAGE
KNOX COUNTY, TN



5 Projected Capacity and Level of Service

Unsignalized intersection capacity analyses were performed for the AM and PM peak hours to evaluate the traffic conditions at the intersections of N Campbell Station Road and Fretz Road and the intersection of Hatmaker Lane and the proposed project entrance.

The results from the analyses are measured with a term "level of service" (LOS), which is based on the amount of delay experienced at the intersection. The LOS index ranges from LOS A, indicating excellent traffic conditions with minimal delay, to LOS F indicating very congested conditions with excessive delay. LOS D generally is considered the minimum acceptable condition in urban areas. Table 5-1 shows the results of the capacity analyses.

Table 5-1

Intersection Analysis
Level of Service (LOS) Summary

		Delay (sec)/LOS	
	N Campbell Station Ro	oad @ Fretz Road (Existing 2016)	
AM Peak	WB LT	8.0 / A	
	NB LR	11.7 / B	
PM Peak	WB LT	8.1 / A	
	NB LR	11.3 / B	
N Ca	mpbell Station Road @	Fretz Road (Background Growth 2019)	
AM Peak	WB LT	8.1 / A	
	NB LR	12.1 / B	
PM Peak	WB LT	8.2 / A	
	NB LR	11.7 / B	
N Campbell	Station Road @ Fretz Ro	ad (Background Growth + Full Buildout 2	019)
AM Peak	WB LT	8.2 / A	
	NB LR	14.5 / B	
PM Peak	WB LT	8.6 / A	
	NB LR	13.5 / B	

Hatmaker	Hatmaker Lane @ Project Entrance (Background Growth + Full Buildout 2019)							
AM Peak	EB LT SB LR	7.2 / A 8.8 / A						
PM Peak	EB LT SB LR	7.4 / A 8.9 / A						

6 Turn Lane Warrant Analysis

The intersection of N Campbell Station Road and Fretz Road was evaluated to determine if an eastbound right turn lane or a westbound left turn on N Campbell Station Road was warranted. The Knox County Department of Engineering and Public Works handbook, "Access Control and Driveway Design Policy," was used to analyze the information. An eastbound right turn lane on N Campbell Station Road is not warranted during the AM or PM peak hour. A westbound left turn on N Campbell Station Road is warranted during the PM peak hour. The turn lane warrant worksheets and analysis are included in Attachment 6.

7 Conclusions and Recommendations

7.1 N Campbell Station Road @ Fretz Road

At the intersection of N Campbell Station Road and Fretz Road, all approaches currently operate at an acceptable LOS A or B during both the AM and PM peak hours. The intersection will continue to operate at a LOS A during the AM peak hour and a LOS B during the PM peak hour after the completion of the Towering Oaks Village.

An eastbound right turn lane is not warranted at the intersection of Campbell Station Road and Fretz Road.

A westbound left turn lane is warranted at the intersection of Campbell Station Road and Fretz Road after the full buildout of the Towering Oaks Village, but only during the PM peak hour. The left turn lane warrant is not met during the construction of Phase I, but will be met after 82 of the proposed 110 single family lots is completed.

The unsignalized intersection capacity analyses shows a 95% queue length at the full buildout for the westbound left turning movement of less than one car length

during both the AM and PM peak hours. Per AASHTO "A Policy on Geometric Design of Highways and Streets" the recommended storage length is two car lengths (approximately 50 feet) and the recommended taper length is 100 feet.

7.2 Fretz Road

The width of Fretz Road between the intersection with Hatmaker Lane and the intersection with Woodhollow Lane varies from approximately 14 feet to 22 feet. The recommended road width per AASHTO's "A Policy on Geometric Design of Highways and Streets" for a local road with a posted speed limit of 30 mph with ADT between 1,500 and 2,200 trips per day is 22 feet.

The length of Fretz Road is approximately 2,800 feet therefore; the minimum allowable horizontal radius of curvature at the centerline of the road right-of-way is 250 feet per the "Minimum Subdivision Regulations" for Knoxville and Knox County. The existing curvature on Fretz Road is less than 250 feet, but there does not appear to be sufficient right-of-way to make improvements to this horizontal radius.

7.3 Hatmaker Lane @ Project Entrance

The minimum intersection spacing required for a local street is 125 feet per the "Minimum Subdivision Regulations" for Knoxville and Knox County. The nearest road intersection to the project entrance is currently 1,850 feet west at the intersection of Hatmaker Lane and Hopper Drive. This intersection exceeds the typical minimum separation of 125 feet between roads on a local street; therefore, no change is necessary.

The minimum required sight distance for a road with a posted speed limit of 30 mph is 300 feet in each direction in accordance with the "Minimum Subdivision Regulations" for Knoxville and Knox County. The proposed intersection of Hatmaker Lane and the project entrance has a measured sight distance that exceeds 300-ft east and west of the intersection, which meets the requirement. FMA recommends any necessary landscaping that may be involved to maintain this sight distance and continue to comply with Knox County Engineering & Public Works.

The proposed intersection geometry will be one lane entering and one lane exiting the subdivision. The project entrance is expected to operate at a LOS A during both the AM and PM peak hours.

Attachment 1 Traffic Counts

Project: Towering Oaks Village Date Conducted: Tuesday 2/23/2016

	N Camr	bell Stati	on Rd	N Camr	bell Stati	on Rd	Fr	etz Road		
		astbound			estbound			rthbound	ł	
Start	Thru	Right	Total	Left	Thru	Total	Left	Right	Total	Int. Total
7:00 AM	47	1	48	1	22	23	0	7	7	78
7:15 AM	54	1	55	3	25	28	1	7	8	91
7:30 AM	85	1	86	4	31	35	3	8	11	132
7:45 AM	90	2	92	3	54	5 <i>7</i>	2	6	8	15 <i>7</i>
Total	276	5	281	11	132	143	6	28	34	458
8:00 AM	54	1	55	1	68	69	0	7	7	131
8:15 AM	67	1	68	0	52	52	2	2	4	124
8:30 AM	58	0	58	1	50	51	0	2	2	111
8:45 AM	52	<u>2</u> 4	54	2 4	61	63	2	3 14	3	120
Total	231	4	235	4	231	235	2	14	16	486
11:00 AM	33	0	33	2	41	43	1	7	8	84
11:15 AM	47	0	47	4	32	36	1	3	4	87
11:30 AM	35	1	36	0	36	36	0	3	3	75
11:45 AM	47	0	47	0	46	46	0	3	3	96
Total	162	1	163	6	155	161	2	16	18	342
12:00 PM	61	0	61	1	55	56	0	0	0	11 <i>7</i>
12:15 PM	56	1	57	5	47	52	0	7	7	116
12:30 PM	43	2	45	1	39	40	1	2	3	88
12:45 PM	39	0	39	3	44	47	0	2	2	88
Total	199	3	202	10	185	195	1	11	12	409
2:00 PM	47	1	48	7	52	59	1	1	2	109
2:15 PM	42	2	44	4	46	50	2	3	5	99
2:30 PM	43	0	43	5	52	57	0	3	3	103
2:45 PM	35	0	35	6	53	59	1	3	4	98
Total	167	3	1 <i>7</i> 0	22	203	225	4	10	14	409
3:00 PM	67	2	69	5	61	66	1	5	6	141
3:15 PM	48	0	48	3	47	50	0	1	1	99
3:30 PM	74	0	74	2	60	62	0	5	5	141
3:45 PM	80	1	81	15	54	69	0	7	7	157
Total	269	3	272	25	222	247	1	18	19	538
4:00 PM	45	2	47	1	56	5 <i>7</i>	1	3	4	108
4:15 PM	52	0	52	5	<i>7</i> 1	76	0	3	3	131
4:30 PM	66	2	68	9	65	74	1	5	6	148
4:45 PM	66	0	66	7	72	79	0	6	6	151
Total	229	4	233	22	264	286	2	1 <i>7</i>	19	538
5:00 PM	59	3	62	8	85	93	1	5	6	161
5:15 PM	90	1	91	6	78	84	1	3	4	179
5:30 PM	66	0	66	3	91	94	0	3	3	163
5:45 PM	64	2	66	3	85	88	1	4	5	159
Total	279	6	285	20	339	359	3	15	18	662
Crand Tatal	1010	20	1044	120	1701	1051	24	120	150	2042
Grand Total Approach %	1812 98.4	29 1.6	1841	120 6.5	1731 93.5	1851	21 14.0	129 86.0	150	3842
Total %	96.4 47.2	0.8	47.9	3.1	95.5 45.1	48.2	0.5	3.4	3.9	
. Ota: 70	7/.4	0.0	77.3	5.1	75.1	70.∠	0.5	J. T	5.9	l

Project: Towering Oaks Village Date Conducted: Tuesday 2/23/2016

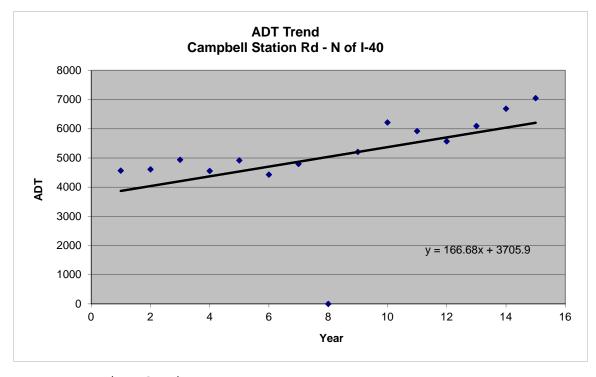
AM Peak Hour	7:30 AM - 8:30 AM	544
Lunch Peak Hour	11:45 AM - 12:45 PM	417
PM Peak Hour	5:00 PM - 6:00 PM	662

	N Ca	mpbell Stat	tion Rd	N Can	npbell Stati	ion Rd		Fretz Road		
		Eastbound	k	1	Westbound	d	١	Northboun	d	
Start	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis from 7	:00 AM to 9	:00 AM								
AM Peak Hour begins at 7:	:15 AM									
7:30 AM	85	1	86	4	31	35	3	8	11	132
7:45 AM	90	2	92	3	54	5 <i>7</i>	2	6	8	157
8:00 AM	54	1	55	1	68	69	0	7	7	131
8:15 AM	67	1	68	0	52	52	2	2	4	124
Total Volume	296	5	301	8	205	213	7	23	30	544
Future (3% over 3 yrs)	323	5		9	224		8	25		594
PHF	0.82	0.63		0.50	0.75		0.58	0.72		0.87
Peak Hour Analysis from 1	1:00 AM to	1:00 PM								
Lunch Peak Hour begins at	11:45 PM									
11:45 PM	47	0	47	0	46	46	0	3	3	96
12:00 PM	61	0	61	1	55	56	0	0	0	117
12:15 PM	56	1	5 <i>7</i>	5	47	52	0	7	7	116
12:30 PM	43	2	45	1	39	40	1	2	3	88
Total Volume	207	3	210	7	187	194	1	12	13	417
Future (3% over 3 yrs)	226	3		8	204		1	13		456
PHF	0.85	0.38		0.35	0.85		0.25	0.43		0.89
Peak Hour Analysis from 2	:00 PM to 6:	00 PM								
PM Peak Hour begins at 5:	00 PM		_							_
5:00 PM	59	3	62	8	85	93	1	5	6	161
5:15 PM	90	1	91	6	78	84	1	3	4	179
5:30 PM	66	0	66	3	91	94	0	3	3	163
5:45 PM	64	2	66	3	85	88	1	4	_	
Total Volume	279	6	285	20	339	359	3	15	18	662
Future (3% over 3 yrs)	305	7		22	370		3	16		723
PHF	0.78	0.50		0.63	0.93		0.75	0.75		0.92

Attachment 2 ADT Trends

Attachment 2 ADT Trends

		Adjusted Average
	Year	Daily Traffic
1	2000	4570
2	2001	4610
3	2002	4940
4	2003	4560
5	2004	4920
6	2005	4430
7	2006	4800
8	2007	0
9	2008	5210
10	2009	6220
11	2010	5920
12	2011	5570
13	2012	6100
14	2013	6690
15	2014	7050

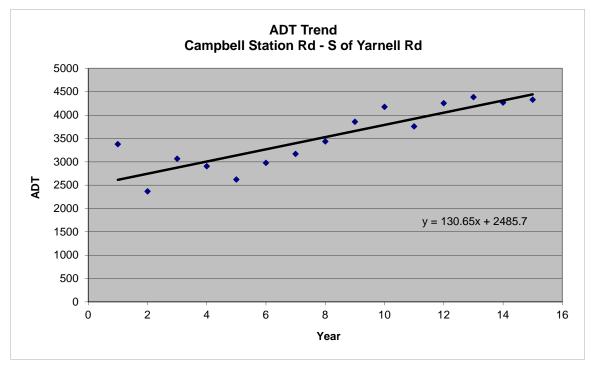


Most Recent Trend Line Growth

Year ADT 2000 4570 2014 7050

Attachment 2 ADT Trends

		Adjusted Average
	Year	Daily Traffic
1	2000	3378
2	2001	2367
3	2002	3067
4	2003	2907
5	2004	2623
6	2005	2978
7	2006	3169
8	2007	3436
9	2008	3860
10	2009	4176
11	2010	3759
12	2011	4256
13	2012	4385
14	2013	4271
15	2014	4332



Most Recent Trend Line Growth

Year ADT 2000 3378 2014 4332

Annual Percent Growth

1.88%

Attachment 3 Intersection Worksheet Existing AM/PM Peaks

		O-WAY STOP							
General Information	1		Site Ir	nformat	ion				
Analyst	Addie Kir	kham	Interse	ction		Campbel	l Station @) Fretz	
Agency/Co.	FMA		Jurisdi			Knox Co	unty		
Date Performed	2/24/2016		Analys	is Year		2016			
Analysis Time Period	AM Peak	- Existing							
Project Description 56	4.001 - Towerir	ng Oaks Village							
East/West Street: N Ca		Rd			et: Fretz	Rd			
ntersection Orientation:	East-West		Study F	Period (hr	s): 0.25				
/ehicle Volumes ar	nd Adjustme	ents							
Major Street		Eastbound				Westbou	ınd		
Movement	1	2	3		4	5	6		
	L	Т	R		L	Т		R	
/olume (veh/h)		296		5 8		205			
Peak-Hour Factor, PHF	1.00	0.82	0.63 0.50		0.75		1.00		
Hourly Flow Rate, HFR veh/h)	0	360	7		16	273		0	
Percent Heavy Vehicles	0				0				
/ledian Type				Undivide	ed				
RT Channelized			0					0	
anes	0	1	0		0	1		0	
Configuration			TR		LT				
Jpstream Signal		0				0			
Minor Street	i	Northbound	<u>'</u>	- i -		Southboo	ınd		
Movement	7	8	9		10	11	1	12	
	L	T	R		L	 		R	
/olume (veh/h)	7		23			 			
Peak-Hour Factor, PHF	0.58	1.00	0.72		1.00	1.00		1.00	
Hourly Flow Rate, HFR		1							
veh/h)	12	0	31		0	0		0	
Percent Heavy Vehicles	0	0	0		0	0		0	
Percent Grade (%)		0				0			
lared Approach		N	1			N			
Storage	1	0				0			
RT Channelized	1		0			+ -		0	
_anes	0	0	0		0	0		0	
Configuration	 	LR	 			+			
Delay, Queue Length, a	nd Lovel of Ce		<u> </u>						
	Eastbound	Westbound		Northbour		1 6	`authhaun		
Approach					_		outhboun	_	
Movement	1	4	7	8	9	10	11	12	
_ane Configuration		LT		LR				4	
/ (veh/h)		16		43					
C (m) (veh/h)		1203		582					
ı/c		0.01		0.07					
95% queue length		0.04		0.24					
Control Delay (s/veh)		8.0		11.7				1	
OS		A		B		+		+	
		 ^ 				+	<u> </u>		
Approach Delay (s/veh)				11.7					
Approach LOS			В						

	TW	O-WAY STOP	CONTR	OL S	UMI	MARY					
General Information	n		Site I	nforn	natio	on					
Analyst	Addie Kir	kham	Interse	ection			Campbe	ll Station	@ Fretz		
Agency/Co.	FMA		Jurisdi				Knox County				
Date Performed	2/24/2016		Analys	sis Yea	ar		2016				
Analysis Time Period	PM Peak	- Existing									
Project Description 56											
East/West Street: N Ca		Rd				t: Fretz F	Rd				
Intersection Orientation:	East-West		Study I	Period	(hrs)	: 0.25					
Vehicle Volumes ar	<u>nd Adjustme</u>										
Major Street		Eastbound					Westbou	ınd			
Movement	1	2	3			4	5		6		
\	L L	T 070	R			L	T		R		
Volume (veh/h) Peak-Hour Factor, PHF	1.00	279	6	`		20	339		1.00		
Hourly Flow Rate, HFR	1.00	0.78	0.50	'		0.63	0.93		1.00		
(veh/h)	0	357	12			31	364		0		
Percent Heavy Vehicles	0					0					
Median Type				Undivided							
RT Channelized			0						0		
Lanes	0	1	0			0	1		0		
Configuration			TR			LT	 				
Upstream Signal		0					0				
Minor Street		Northbound					Southbound				
Movement	7	8	9			10	11		12		
	L	Т	R			L	Т		R		
Volume (veh/h)	3		15				1				
Peak-Hour Factor, PHF	0.75	1.00	0.75	i		1.00	1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	4	0	20			0	0		0		
Percent Heavy Vehicles	0	0	0			0	0		0		
Percent Grade (%)		0					0				
Flared Approach		N	1				N				
Storage		0					0				
RT Channelized			0						0		
Lanes	0	0	0			0	0		0		
Configuration		LR									
Delay, Queue Length, a	and Level of Se	ervice	•				1	-			
Approach	Eastbound	Westbound		Northb	ound			Southbou	ınd		
Movement	1	4	7	8		9	10	11	12		
Lane Configuration		LT		LF	?						
v (veh/h)		31		24	!						
C (m) (veh/h)		1201		593	3						
v/c		0.03		0.04							
95% queue length		0.08		0.1			<u> </u>				
Control Delay (s/veh)		8.1		11.			 		_		
LOS				71. В			+				
		Α					 				
Approach Delay (s/veh)				11.			1				
Approach LOS				В							

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Attachment 4 Intersection Worksheet Background AM/PM Peaks

		O-WAY STOP							
General Information	1		Site Ir	nformati	ion				
Analyst	Addie Kir	kham	Interse				l Station @) Fretz	
Agency/Co.	FMA		Jurisdi			Knox Co	unty		
Date Performed	2/24/2016		Analys	is Year		2019			
Analysis Time Period	AM Peak	- Background							
		ng Oaks Village							
East/West Street: N Ca		Rd			et: Fretz	Rd			
ntersection Orientation:	East-West		Study F	Period (hrs	s): 0.25				
/ehicle Volumes ar	nd Adjustme	ents							
Major Street		Eastbound				Westbou	ınd		
Movement	1	2	3		4	5		6	
	L	Т	R		L	Т		R	
/olume (veh/h)	1	323	5		9	224			
Peak-Hour Factor, PHF	1.00	0.82	0.63		0.50	0.75		1.00	
Hourly Flow Rate, HFR veh/h)	0	393	7		18	298		0	
Percent Heavy Vehicles	0				0				
Median Type				Undivide	d				
RT Channelized			0					0	
anes	0	1	0		0	1		0	
Configuration			TR		LT				
Jpstream Signal		0				0			
Minor Street	i	Northbound	•			Southboo	und .		
Movement	7	8	9		10	11		12	
	Ĺ	T	R		L	 		R	
/olume (veh/h)	8	 	25			 	_	- • •	
Peak-Hour Factor, PHF	0.58	1.00	0.72		1.00	1.00		1.00	
Hourly Flow Rate, HFR		1							
(veh/h)	13	0	34		0	0		0	
Percent Heavy Vehicles	0	0	0		0	0		0	
Percent Grade (%)		0				0			
lared Approach		N				N			
Storage	1	0				0			
RT Channelized	1		0			+ -		0	
_anes	0	0	0		0	0		0	
Configuration	 	LR	⊢			 			
Delay, Queue Length, a	and Lovel of Ca		<u> </u>			Í			
	Eastbound	Westbound	,	Northboun	d		Southboun	d	
Approach					_			_	
Movement	1	4	7	8	9	10	11	12	
_ane Configuration		LT		LR					
/ (veh/h)		18		47					
C (m) (veh/h)		1170		551					
ı/c		0.02		0.09					
95% queue length		0.05		0.28					
Control Delay (s/veh)		8.1		12.1	1	 			
OS		A A		B	1	+	 	+	
				12.1		+	<u> </u>		
Approach Delay (s/veh)						+			
Approach LOS			В						

		O-WAY STOP							
General Information	<u>n</u>		Site Ir	nforma	tion				
Analyst	Addie Kir	kham	Interse				l Station @) Fretz	
Agency/Co.	FMA		Jurisdio			Knox Col	ınty		
Date Performed	2/24/2016		Analys	is Year		2019			
Analysis Time Period	-	- Background							
Project Description 56	4.001 - Towerir	ng Oaks Village	h		. = .	<u> </u>			
East/West Street: N Ca		Rd			eet: Fretz	Rd			
ntersection Orientation:			Study F	eriod (h	rs): 0.25				
Vehicle Volumes ar	nd Adjustme								
Major Street		Eastbound	1 0			Westbou	nd		
Movement	1	2	3		4	5		6	
(a l	L L	T	R		L	T		R	
/olume (veh/h) Peak-Hour Factor, PHF	1.00	305 0.78	7 0.50	_	22 0.63	370 0.93		1.00	
Hourly Flow Rate, HFR		1	1	- 					
veh/h)	0	391	14		34	397		0	
Percent Heavy Vehicles	0				0				
Median Type				Undivid	led				
RT Channelized			0					0	
_anes	0	1	0		0	1		0	
Configuration	1		TR		LT				
Jpstream Signal		0	1			0			
Minor Street		Northbound	•			Southbou	Southbound		
Movement	7	8	9		10	11		12	
	L	Т	R		L	Т		R	
/olume (veh/h)	3		16						
Peak-Hour Factor, PHF	0.75	1.00	0.75		1.00	1.00		1.00	
Hourly Flow Rate, HFR	4	0	21		0	0		0	
veh/h)									
Percent Heavy Vehicles	0	0	0		0	0		0	
Percent Grade (%)		0				0			
-lared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
_anes	0	0	0		0	0		0	
Configuration		LR							
Delay, Queue Length, a	nd Level of Se	ervice					-		
Approach	Eastbound	Westbound	N	Northbou	nd	S	outhboun	d	
Movement	1	4	7	8	9	10	11	12	
_ane Configuration	<u> </u>	LT	- 	LR	 	+		+	
/ (veh/h)		34		25	+	+		1	
						+		+-	
C (m) (veh/h)		1165		561		+		1	
//C		0.03		0.04				1	
95% queue length		0.09		0.14					
Control Delay (s/veh)		8.2		11.7					
_OS		Α		В					
Approach Delay (s/veh)				11.7					
			В						

Attachment 5 Intersection Worksheet Background AM/PM Peaks + Full Buildout

General Information	<u> </u>		Cita I	oformat	ion			
	formation Site Information Addie Kirkham Intersection Campbell Station							
Analyst		kham) Fretz
Agency/Co.	FMA	•	Jurisdi			Knox County		
Date Performed	3/29/2016		Analys	is Year		2019		
Analysis Time Period		- Full Buildout	[
		ng Oaks Village	N41- /C) tl Ot	-1	D./		
East/West Street: N Cantersection Orientation:		Ra			et: Fretz	Ra		
			Study F	Period (hr	s): 0.25			
Vehicle Volumes ar	<u>id Adjustme</u>			-				
Major Street		Eastbound				Westbou	ınd	
Movement	1	2	3		4	5		6
	L	T	R		L	T		R
/olume (veh/h)	1.00	323	14		22	224		4.00
Peak-Hour Factor, PHF	1.00	0.82	0.63		0.50	0.75		1.00
Hourly Flow Rate, HFR veh/h)	0	393	22		44	298		0
Percent Heavy Vehicles	0			$\overline{}$	0			
Median Type	 			Undivide				
RT Channelized	+	T	T 0	<u> </u>				0
anes	0	1	0	- -	0	1		0
Configuration	+ -	'	TR	-+	LT	+ '		
Jpstream Signal	+	0	IK	_	LI	0		
	 							
Minor Street	 	Northbound	1 0		40	Southboo	und	40
Movement	7	8 -	9		10	11		12
	L	Т	R		L	Т		R
/olume (veh/h)	24	1.00	74		4.00	1.00		4.00
Peak-Hour Factor, PHF	0.58	1.00	0.72		1.00	1.00		1.00
Hourly Flow Rate, HFR veh/h)	41	0	102		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage	1	0	 	- 		0		
RT Channelized	+	 	0	- -		+ -		0
anes	0	0	0	-+	0	0		0
Configuration	+	LR	+ -		U	1		U
	1		<u> </u>					
Delay, Queue Length, a				La sella l	1	1 -) 41- J	
Approach	Eastbound	Westbound		Northbour			Southbound	_
Movement	1	4	7	8	9	10	11	12
ane Configuration		LT		LR				
(veh/h)		44		143				
C (m) (veh/h)	<u> </u>	1155		521				
r/c		0.04		0.27				
95% queue length		0.12		1.11				1
Control Delay (s/veh)		8.2		1.11				1
					+	+	 	1
.OS		Α		В				
Approach Delay (s/veh)				14.5				
Approach LOS				В				

	TW	O-WAY STOP	CONTR	OL S	UMI	MARY					
General Information	n		Site I	nforn	natio	on					
Analyst	Addie Kir	kham	Interse	ection			Campbe	l Station	@ Fretz		
Agency/Co.	FMA		Jurisdi	ction			Knox County				
Date Performed	3/29/2016		Analys	is Yea	ar		2019				
Analysis Time Period	PM Peak	- Full Buildout									
		ng Oaks Village									
East/West Street: N Ca		Rd		n/South Street: Fretz Rd							
Intersection Orientation:			Study I	Period	(hrs)	: 0.25					
Vehicle Volumes ar	<u>nd Adjustme</u>										
Major Street		Eastbound					Westbou	ınd			
Movement	1	2	3			4	5		6		
\	L L	T	R				T		R		
Volume (veh/h) Peak-Hour Factor, PHF	1.00	305	305 25 76 0.78 0.50 0.63		76	370 0.93		1.00			
Hourly Flow Rate, HFR	1.00		0.50				0.93		1.00		
(veh/h)	0	391	50			120	397		0		
Percent Heavy Vehicles	0					0					
Median Type			Undivided								
RT Channelized			0						0		
Lanes	0	1	0			0	1		0		
Configuration			TR			LT	1				
Upstream Signal		0					0				
Minor Street		Northbound					Southbo	und			
Movement	7	8	9			10	11		12		
	L	Т	R			L	Т		R		
Volume (veh/h)	9		52								
Peak-Hour Factor, PHF	0.75	1.00	0.75	'		1.00	1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	12	0	69			0	0		0		
Percent Heavy Vehicles	0	0	0			0	0		0		
Percent Grade (%)		0					0				
Flared Approach		N					N				
Storage		0					0				
RT Channelized			0						0		
Lanes	0	0	0			0	0		0		
Configuration		LR									
Delay, Queue Length, a	and Level of Se	ervice									
Approach	Eastbound	Westbound	1	Northb	ound		9	Southbou	nd		
Movement	1	4	7	8		9	10	11	12		
Lane Configuration		LT		LF	?						
v (veh/h)		120		81	,						
C (m) (veh/h)		1130		504	4						
v/c		0.11		0.1	6						
95% queue length		0.36	0.57								
Control Delay (s/veh)		8.6		13.							
LOS		A		В							
Approach Delay (s/veh)				13.				1			
Approach LOS				13. B							
Approacti LOS											

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	TW	O-WAY STOP	CONTRO	DL SI	UMM	ARY				
General Information	า		Site Ir	nform	natior	1				
Analyst	Addie Kii	rkham	Interse	ction			Hatmake		oject	
Agency/Co.	FMA		Jurisdio	otion			Entrance Knox County			
Date Performed	3/29/201	6	Analysi		r		2019	unty		
Analysis Time Period	AM Peak	(Allalysi	13 1 64	U .		2019			
Project Description 56	4.001 Towering	o Oaks Village								
East/West Street: Hatm		, cancimage	North/S	outh S	Street:	Project	Entrance			
Intersection Orientation:			Study F							
Vehicle Volumes ar	nd Adiustme	ents	*							
Major Street		Eastbound					Westboo	und		
Movement	1	2	3			4	5			6
	L	Т	R			L	Т			R
Volume (veh/h)	0	0					0		2	22
Peak-Hour Factor, PHF	0.90	0.90	1.00		1	.00	0.90		0.	90
Hourly Flow Rate, HFR (veh/h)	0	0	0			0	0		2	24
Percent Heavy Vehicles	0					0				
Median Type		Undivided								
RT Channelized			0							0
Lanes	0	1	0			0	1			0
Configuration	LT								TR	
Upstream Signal		0					0			
Minor Street		Northbound					Southbound			
Movement	7	8	9			10	11		12	
	L	Т	R			L	Т			R
Volume (veh/h)						65	1 00		0.90	
Peak-Hour Factor, PHF	1.00	1.00	1.00		0	.90	1.00		0.9	
Hourly Flow Rate, HFR (veh/h)	0	0	0			72	0			0
Percent Heavy Vehicles	0	0	0			0	0			0
Percent Grade (%)		0					0			
Flared Approach		Ν					N			
Storage		0					0			
RT Channelized			0							0
Lanes	0	0	0			0	0			0
Configuration							LR			
Delay, Queue Length, a	and Level of Se	ervice								
Approach	Eastbound	Westbound	١	lorthb	ound			Southbo	ound	
Movement	1	4	7	8		9	10	1	1	12
Lane Configuration	LT							LF	₹	
v (veh/h)	0							72	2	
C (m) (veh/h)	1604							101	_	
v/c	0.00						1	0.0		
95% queue length	0.00				-		1	0.2	_	
Control Delay (s/veh)	7.2				-+			8.8		
LOS	A A				-+		 	A	-	
Approach Delay (s/veh)							 	8.8		
- ' '							 			
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	TW	O-WAY STOP	CONTR	OL SI	UMN	//ARY			
General Information	n		Site I	nform	natio	on			
Analyst	Addie Kir	kham	Interse	ection			Hatmakei	@ Project	
Agency/Co.	FMA	FMA		1			Entrance Knox County		
Date Performed	3/29/201	Jurisdiction Analysis Year			r	2019			
Analysis Time Period	PM Peak		Arialysis fear			2019			
Project Description 56	4.001 Towering	Oaks Village							
East/West Street: Hatm			North/S	South S	Stree	t: <i>Project</i>	Entrance		
Intersection Orientation:	East-West		Study F	Period	(hrs)	: 0.25			
Vehicle Volumes ar	nd Adjustme	nts							
Major Street		Eastbound					Westbou	nd	
Movement	1	2	3			4	5		6
	L	Т	R			L	Т		R
Volume (veh/h)	0	0					0		72
Peak-Hour Factor, PHF	0.90	0.90	1.00)		1.00	0.90		0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0			0	0		80
Percent Heavy Vehicles	0					0			
Median Type				Undi	videa	1			
RT Channelized			0						0
Lanes	0	1	0			0	1		0
Configuration	LT								TR
Upstream Signal		0					0		
Minor Street		Northbound					Southbou	ınd	
Movement	7	8	9			10	11		12
	L	Т	R			Т		R	
Volume (veh/h)						42			0
Peak-Hour Factor, PHF	1.00	1.00	1.00 1.00 0.		0.90	1.00		0.90	
Hourly Flow Rate, HFR (veh/h)	0	0	0			46	0		0
Percent Heavy Vehicles	0	0	0			0	0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
Lanes	0	0	0			0	0		0
Configuration							LR		
Delay, Queue Length, a	nd Level of Se	rvice							
Approach	Eastbound	Westbound	ľ	Northb	ound		S	outhbound	
Movement	1	4	7	8		9	10	11	12
Lane Configuration	LT							LR	
v (veh/h)	0							46	
C (m) (veh/h)	1531							977	
v/c	0.00							0.05	
95% queue length	0.00							0.15	
Control Delay (s/veh)	7.4							8.9	
LOS	Α							Α	
Approach Delay (s/veh)						1	†	8.9	1
Approach LOS								A	
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Attachment 6
Turn Lane Warrant Analysis

Attachment 6 Turn Lane Warrant Analysis

Project: Towering Oaks Village

N Campbell Station Road	VOLUMES				
at Fretz Road					
LEFT TURN	Opposing	Thru	LT	LT MAX	Warrant Met
AM	337	224	22	110	NO
PM	330	370	76	65	YES
N Campbell Station Road	VOLUMES				
at Fretz Road					
RIGHT TURN	_	Thru	RT	RT MAX	Warrant Met
AM	_	323	14	299	NO
PM		305	25	299	NO

TABLE 4A

LEFT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 35 MPH OR LESS

(If the left-turn volume exceeds the table value a left -turn lane is needed)

OPPOSING	THROUG	GH VOLUME	PLUS RIGH	T-TURN	AOTOWR	, *
VOLUME	100 - 149	150 - 199	200 - 249	250 - 299	300 - 349	350 - 399
100 - 149	300	235	185	145	120	100
150 - 199	245	200	160	130	110	90
200 - 249	205	170	140	115	100	80
250 - 299	175	150	125	105	90	70
300 - 349	155	135	110	95	\$0	65
350 - 399	135	120		85	70	160
400 - 419 450 - 499	120 105	105 90	96	I	PM Peak 76 LT	55 50
500 - 549	95	\$0	70	65	55	50
550 - 599	85	70	65	60	50	45
600 - 649	75	65	60	55	45	40
650 - 699	70	60	55	50	40	35
700 - 749	65	55	50	45	35	30
750 or More	60	50	45	40	35	30

OPPOSING	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *							
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 599	= 1 > 600		
160 - 149	100	80	70	60	55	50		
150 - 199	90	75	65	55	50	45		
200 - 249	80	72	460	55	50	45		
250 - 299	70	65	55	50	45	40		
300 - 349	65	60	50	50	45	40		
350 - 399	60	55	50	45	40	40		
400 - 449	55	50	45	45	40	35		
450 - 499	50	45	45	40	35	35		
500 - 549	50	45	40	40	35	35		
550 - 599	45	40	40	35	35	35		
600 - 649	40	35	35	35	35	30		
650 - 699		35	35	30	30	30		
700 - 749	30	30	30	30	30	30		
750 or More	30		30	30	30	30		

^{*} Or through volume only if a right-turn lane exists.

TABLE 4B

RIGHT-TURN LANE VOLUME THRESHOLDS

FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 35 MPH OR LESS

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *							
VOLUME	<100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399		
Fewer Than 25 25 - 49 50 - 99				ı <u>—</u>	Peak 14 Peak 25			
100 - 149 150 - 199								
200 - 249 250 - 299						Yes		
300 - 349 350 - 399			_	Yes	Yes Yes	Yes Yes		
400 - 449 450 - 499			Yes Yes	Yes Yes	Yes Yes	Yes Yes		
500 - 549 550 - 599	_	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
600 or More	Yes	Yes	Yes	Yes	Yes	Yes		

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *							
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 600	+ / > 600		
Fewer Than 25 25 - 49 50 - 99					Yes	Yes Yes		
100 - 149 150 - 199			Yes	Yes Yes	Yes Yes	Yes Yes		
200 - 249 250 - 299	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
300 - 349 350 - 399	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
400 - 449 450 - 499	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
500 - 549 550 - 599	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes		
600 or More	Yes	Yes	Yes	Yes	Yes	Yes		

^{*} Or through volume only if a left-turn lane exists.