

June 27, 2019

Ms. Alicia McAuley Sr. Project Coordinator MBI Companies Inc. 299 N. Weisgarber Road Knoxville, TN 37919

RE:

Trip Generation Study Update Letter - 2019 Knoxville TVA Employees Credit Union - 7-G-19-UR Northshore Town Center - Knoxville, Tennessee

Dear Ms. McAuley:

In accordance with your request we have conducted a review and evaluation of trip generation estimates for the above referenced project, which is to be located within the Northshore Town Center development in southwest Knoxville/Knox County. The current proposal is to construct a Knoxville TVA Employees Credit Union branch facility on this site. This is consistent with the land use proposed in both the original development traffic impact study (2011) and the most recently approved update (2018), both of which assumed a drive-in bank. The only difference is that the current proposal includes a 5,500 square foot building, as opposed to the 4,500 square foot building assumed by both the 2011 study and the 2018 update.

The original traffic impact study (2011) was based on trip generation estimates taken from the Institute of Transportation Engineers (ITE) <u>Trip Generation Manual</u>, 8th Edition. The study update (2018), however, was based on the 9th Edition of this manual. The most current version of this document is the 10th Edition. For purposes of comparison, TABLE 1 below summarizes trip generation estimates for the project site looking at both 4,500 and 5,500 square foot buildings, utilizing the trip generation rates from both the most recently approved study update (2018 with 9th Edition rates) and the current edition of the Manual (10th Edition rates).

TABLE 1

Trip Generation Comparisons for Project Site (Drive-In Bank)
(ITE Trip Generation Manual 9th Edition vs. 10th Edition)

ITE Trip Generation	Weekday	AM Peak Hour	PM Peak Hour
Manual Edition	Total Trips	Total Trips	Total Trips
	By Building Size	By Building Size	By Building Size
	(4,500 sf / 5,500 sf)	(4,500 sf / 5,500 sf)	(4,500 sf / 5,500 sf)
9 th Edition	667 / 815	54 / 66	110 / 134
10 th Edition	450 / 573	43 / 52	92 / 112

As indicated in TABLE 1, the 10th Edition trip generation rates result in significantly lower trip estimates than the 9th Edition rates. This results primarily from the collection of data from additional bank locations. However, the differences in trip estimates between the two different building sizes are on the same order of magnitude between the two Editions. Therefore, for purposes of better comparison with the 2018 study update, which utilized the 9th Edition rates, these rates will also be utilized as the basis for this 2019 update.

In order to better understand the broader impact of the proposed increase in building size of the proposed credit union branch facility, TABLE 2 below summarizes the trip generation estimates for the complete Northshore Town Center development, utilizing the ITE <u>Trip Generation Manual</u> 9th Edition rates. The left half of the table contains the trip estimates from the 2018 study update, which has been approved, and the right half contains the estimates that would result from the 2019 proposed change in building size.

TABLE 2

Trip Generation Comparisons for Complete Development

(Most Recent Approved Study Update-2018 vs. Currently Proposed Study Update-2019)

Development Zone	2018 Update Weekday Trips	2018 Update AM Peak Hour Trips	2018 Update PM Peak Hour Trips	2019 Update Weekday Trips	2019 Update AM Peak Hour Trips	2019 Update PM Peak Hour Trips
Zone A Trips	1,493	124	174	1,641	136	198
Zone B Trips	2,477	227	163	2,477	227	163
Zone C Trips	7,983	294	793	7,983	294	793
Zone D Trips	6,862	206	619	6,862	206	619
Zone E Trips	3,995	252	351	3,995	252	351
Total Trips	22,810	1,104	2,101	22,958	1,116	2,125
Increase from 2018 to 2019	-	73	-	148 (+0.65%)	12 (+1.1%)	24 (+1.1%)

As shown in TABLE 2, the anticipated increase in trips due to the proposed increase in building size amounts to only a little over one percent during both the AM and PM peak traffic hours. Given the location of the site, the additional traffic will mostly be seen along Town Center Boulevard. A table is attached which is the summary of capacity and level of service table from the 2018 study update. As can be seen from this table for 2020 projected traffic, the Town Center Boulevard intersections are all expected to operate at level of service "A" or "B" under full project buildout conditions, except for the Northshore Drive intersection, where level of service "D" is anticipated during the PM peak hour. Even here, the anticipated volume to capacity ratio (V/C) is 0.68. Thus it appears that the small increase in trips anticipated from the larger credit union facility will have little or no impact on intersection operations.

As indicated previously, the 2018 traffic study update, which has been approved, utilized the trip generation rates from the ITE Trip Generation Manual, 9th Edition. TABLE 3 shows how the resulting estimated trips for a 4,500 square foot building compare to the currently proposed 5,500 square foot building, if current 10th Edition rates were applied. As can be seen, the most critical AM and PM peak hour volumes show virtually no change. Thus, the change in trip rates from the 9th to 10th editions of the ITE Trip Generation Manual appears to essentially fully compensate for the proposed increase in building size.

TABLE 3
Trip Generation Comparisons for Project Site (Drive-In Bank)
(Most Recent Approved Study Update-2018 vs. Currently Proposed Study Update-2019)

ITE Trip Generation Manual Edition	Weekday Total Trips	AM Peak Hour Total Trips	PM Peak Hour Total Trips
2018 Study Update w/9 th Edition Rates (4,500 sf building)	667	54	110
2019 Study Update w/10 th Edition Rates (5,500 sf building)	573	52	112

In summary, development of the proposed 5,500 square foot Knoxville TVA Employees Credit Union branch facility within the Northshore Town Center development is anticipated to fit within the trip generation assumptions that were used with the currently approved traffic impact study update (2018). Thus it remains reasonable to conclude that the recommendations of this traffic study update will continue to be valid, and that no additional traffic study for the project site will be necessary.

Please let us know if you have questions or require additional information.

Sincerely,

Alan Childers, P.E.

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cc: Becky Bottoms

CCI Project No.: 00590-0011

Attachments



TABLE 5 SUMMARY OF CAPACITY AND LEVEL OF SERVICE

INTERSECTION Northshore Dr.	TRAFFIC	PEAK	2018 EXI	2018 EXISTING TRAFFIC	일	2020 BAC	2020 BACKGROUND TRAFFIC	FFIC	2020 PR(2020 PROJECTED TRAFFIC	FIC
Northshore Dr.	CONTROL	PERIOD	V/C	DELAY	ros	VIC	DELAY	SOT	VIC	DELAY	TOS
at Town Center Blvd.	SIGNAL	AM PM	0.22	10.1	8 8	0.38	14.8 39.2	ВО	0.38	14.9	8 0
Town Center Blvd. at S. Shopping Ctr. Access	STOP EB/WB	AM	0.01 / 0.01 0.01 0.10 / 0.02	8.8 / 10.0	A/A B/C	0.07 / 0.15 0.39 / 0.79	9.9 / 15.6	A / C	0.07 / 0.15 0.39 / 0.82	9.9 / 16.1 16.1 16.1 16.1 / 124.9	A / C C / F
Town Center Blvd. at N. Shopping Ctr. Access	SIGNAL	AM	0.11	4.5	A B	0.19	9.8	ВЪ	0.20	9.7 15.1	ВА
Town Center Blvd. at Boardwalk Blvd.	ROUNDABOUT	AM	1 1	4.5	44	1 1	5.0	44	1 1	9.1	44
Northshore Dr. at Thunderhead Rd.	SIGNAL	AM	0.72	23.9	ပော	0.81	70.0 62.8	шш	0.82	69.7 62.7	шш
Mitigation Optimize Splits	SIGNAL	AM PM				0.83	23.4	00	0.84	23.6	υυ
Mitigation SB Double Left-Turn	SIGNAL	AM PM							0.65	20.2	O B
Mitigation 4-Lane Northshore	SIGNAL	AM PM							0.55	20.4	ВС
Mitigation SIG	SIGNAL rovement	AM PM							0.40	20.4	вС
Thunderhead Rd. at Boardwalk Blvd.	STOP WBL/WBR	AM	0.22 / 0.55 0.13 / 0.05	73.8 / 22.7 10.0 / 8.7	F/C A/A	1.21 / 0.76 0.38 / 0.21	409.8 / 38.3 18.6 / 10.7	F / E C / B	1.39 / 0.77 0.41 / 0.22	485.4 / 39.9 19.3 / 10.7	F / E C / B
Boardwalk Blvd. at Site Access	STOP NB/SB	AM				0.03 / 0.04	10.4 / 11.2	B / B B / B	0.03 / 0.06 0.09 / 0.28	10.6 / 11.6 11.6	B / B B / C

Note: Average vehicle delay estimated in seconds. STOP control analyses presented by minor approach.



proposed medical land use will generate additional daily trips but is not significant relative to the buildout of Northshore Town Center.

TABLE 1 NORTHSHORE TOWN CENTER TRIP GENERATION FOR BUILDOUT

ZONE	LAND USE			DAILY		AM PEAK			PM PEAK	
		L.U.C	SIZE	TRAFFIC	ENTER	EXIT	TOTAL	ENTER	EXIT	TOTAL
	DISCOUNT STORE (1)	815	135,320			EXIS	TING TRAF	FFIC		
	H.T. RESTAURANT	932	6,500	826	39	32	70	38	26	6
	DRIVE-IN BANK	912	4,500	667	31	23	54	55	55	11
Α	SUB-TOTAL		146,320	1,493	70	55	124	93	81	17
	Internal Trips	10%		149	.7	6	12	9	8	1
	Pass-By Trips	20%		299	14	11	25	19	16	3
	Primary Trips			1,045	49	39	87	65	57	12
	SUPERMARKET (1)	850	54,000			EXIS	STING TRAI	FFIC		
	SPECIAL RETAIL (1)	814	25,900			EXIS	TING TRAF	FFIC		
	FASTFOOD RESTAURANT (2)	934	4,992	2,477	116	111	227	85	78	16
	DRIVE-IN BANK	912	4,500			EXIS	TING TRA	FFIC		
В	SUB-TOTAL		89,392	2,477	116	111	227	85	78	16
	Internal Trips	10%		248	12	11	23	9	8	1
	Pass-By Trips	20%		495	23	22	45	17	16	3
	Primary Trips			1,734	81	78	159	60	55	11
	OFFICE BLDG.(1)	710	57,500			EXIS	STING TRAI	FFIC		
	SHOPPING CENTER	820	87,000	6,203	89	54	143	262	284	54
	H.T. RESTAURANT	932	14,000	1,780	83	68	151	83	55	13
	MULTIPLEX THEATER	445	. 8				-	49	60	10
C	SUB-TOTAL		158,500	7,983	172	122	294	394	399	79
	Internal Trips	10%		798	17	12	29	39	40	7
	Pass-By Trips	25%		1,996	43	31	74	99	100	19
	Primary Trips			5,189	112	79	191	256	259	51
	KNOX CO MULTI-FAMILY	225	24	265	3	11	14	14	12	2
	SHOPPING CENTER	820	79,100	5,831	84	51	135	246	266	51
	MEDICAL OFFICE (2)	720	24,000	766	45	12	57	23	58	8
D	SUB-TOTAL		103,100	6,862	132	74	206	283	336	61
	Internal Trips	10%		686	13	7	21	28	34	6
	Pass-By Trips	30%		2,058	40	22	62	85	101	18
	Primary Trips			4,117	79	44	124	170	201	37
	SINGLE FAMILY	210	120	1,242	23	70	94	78	46	12
	KNOX CO MULTI-FAMILY	225	325	2,753	35	124	159	125	102	22
	SUB-TOTAL		445	3,995	58	194	252	203	148	35
E	Internal Trips				1.5		-	(7)	12	-
	Pass-By Trips			_	4	120	2	2	2	2
	Primary Trips			3,995	58	194	252	203	148	35
	TOTAL TRIP GENERATION			22,810	549	556	1,104	1,059	1,042	2,10
OTAL F	RIMARY TRIP GENERATION			16,080	380	434	813	754	720	1,4

REFERENCE: Trip Generation, 9th Edition, published by the Institute of Transportation Engineers.

5488 9

(1) Developed land use reflected in current traffic.
(2) Revised Land Use from the March 2011 Northshore Town Center traffic study.



Drive-in Bank (912)

9th Edition

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

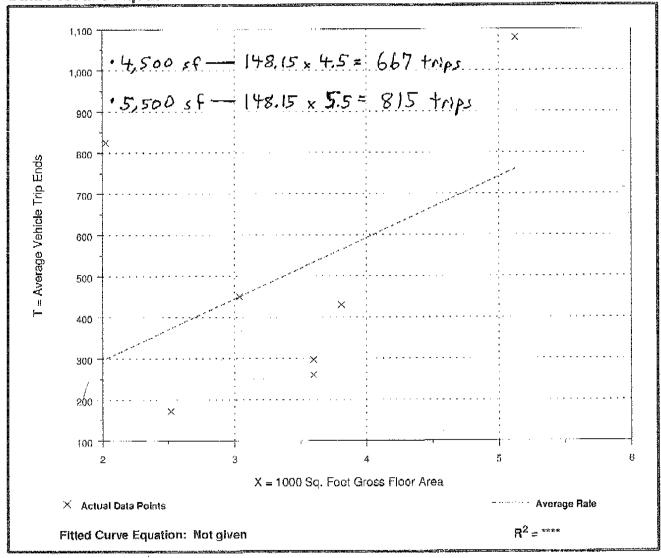
On a: Weekday

Number of Studies: 7 Average 1000 Sq. Feet GFA: 3

Directional Distribution: 50% entering, 50% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
148.15	68.23 - 407.21	97.36



Drive-in Bank (912)

9th Edition

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

On a: Weekday,

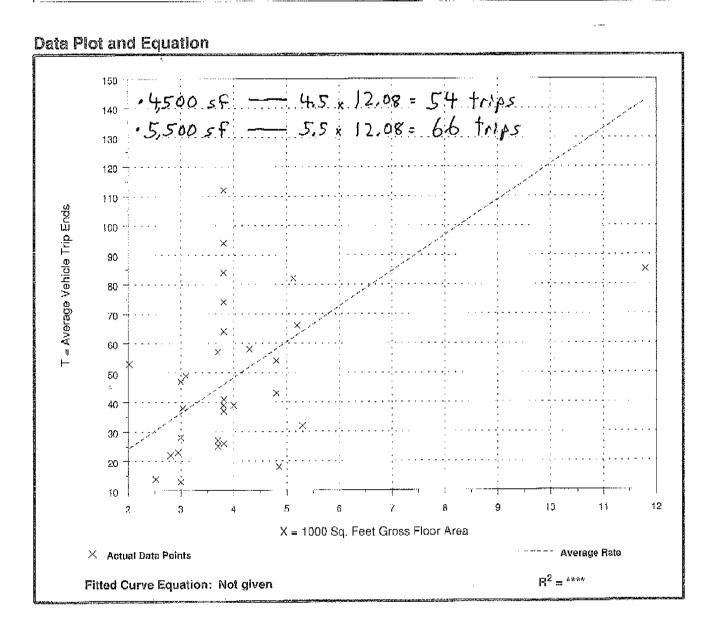
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Number of Studies: 31 Average 1000 Sq. Feet GFA: 4

Directional Distribution: 57% entering, 43% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
12.08	3.71 - 29.40	6.88



Drive-in Bank (912)

9th Edition

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

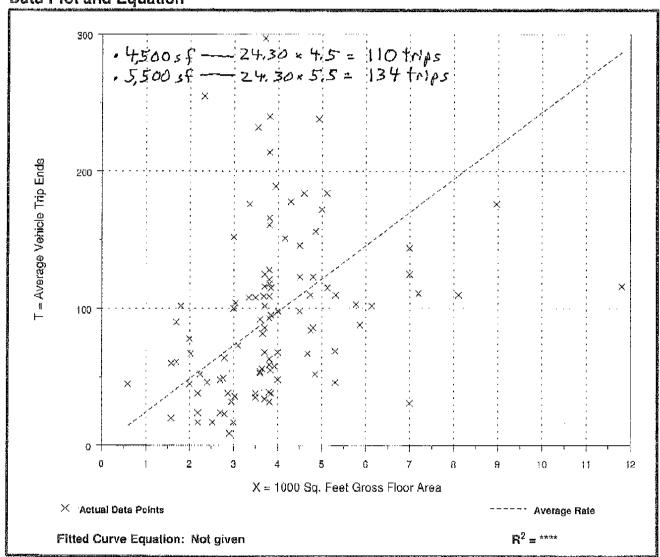
Number of Studies: 102

Average 1000 Sq. Feet GFA: 4

Directional Distribution: 50% entering, 50% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
24.30	3.09 - 109.68	16.24



(912)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday

Setting/Location: General Urban/Suburban

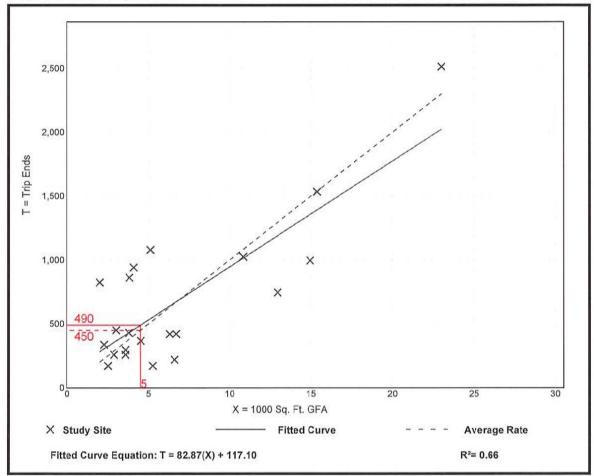
Number of Studies:

Avg. 1000 Sq. Ft. GFA:

50% entering, 50% exiting Directional Distribution:

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
100.03	32.67 - 408.42	61.61



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(912)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location:

General Urban/Suburban

Number of Studies:

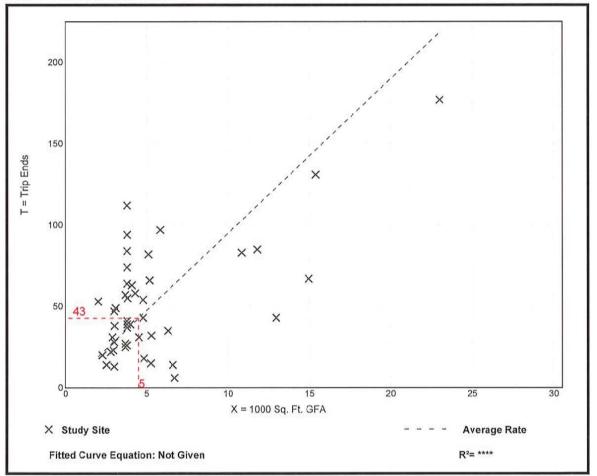
46

Avg. 1000 Sq. Ft. GFA:

Directional Distribution: 58% entering, 42% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
9.50	0.89 - 29.47	5.85



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Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

4,500 sf building

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

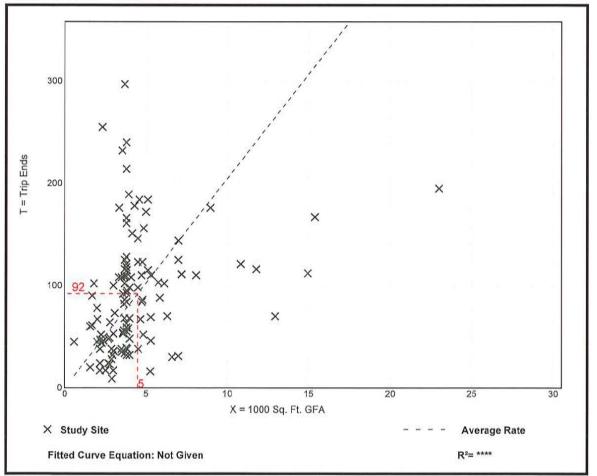
Setting/Location: General Urban/Suburban

Number of Studies: 115 Avg. 1000 Sq. Ft. GFA: 4

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
20.45	3.04 - 109.91	15.01



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(912)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday

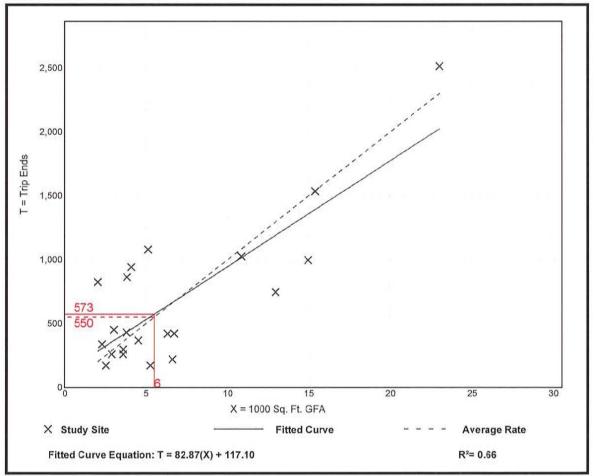
Setting/Location: General Urban/Suburban Number of Studies:

Avg. 1000 Sq. Ft. GFA:

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
100.03	32.67 - 408.42	61.61



Trip Generation Manual, 10th Edition Institute of Transportation Engineers

(912)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

> Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location:

General Urban/Suburban

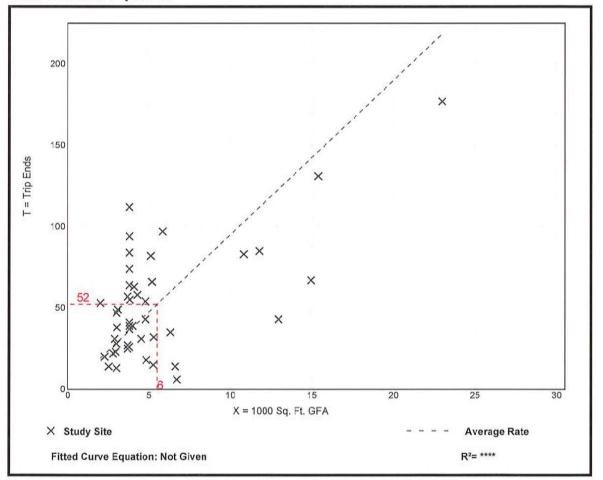
Number of Studies:

Avg. 1000 Sq. Ft. GFA: Directional Distribution:

58% entering, 42% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
9.50	0.89 - 29.47	5.85



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(912)

5,500 sf building

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Setting/Location:

General Urban/Suburban

Number of Studies:

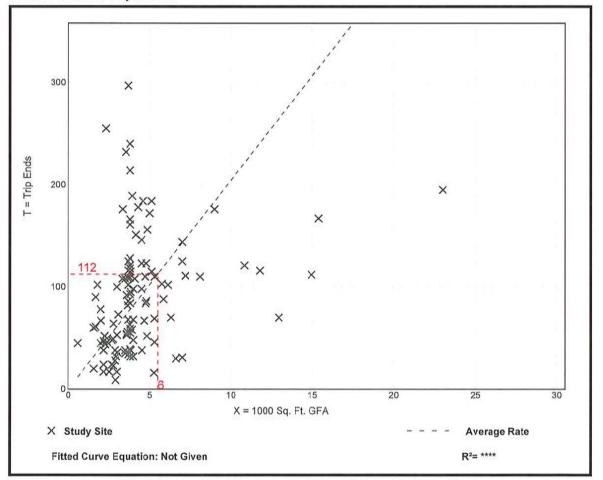
115

Avg. 1000 Sq. Ft. GFA: 4

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
20.45	3.04 - 109.91	15.01



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