BUTTERMILK ROAD RESIDENTIAL DEVELOPMENT knox county, tennessee

TRAFFIC IMPACT STUDY

BUTTERMILK ROAD KNOX COUNTY, TENNESSEE

CCI PROJECT NO. 00630-0002

REV 1



7-SC-22-C / 7-G-22-UR TIS Version 2 7/6/2022

PREPARED FOR:

Eagle Bend PO Box 11315 Knoxville, TN 37939

SUBMITTED BY:

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> REVISED JULY 6 **2022**

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REVISION I (07/06/22)

This report replaces the previous version of the traffic impact study dated 07/23/2021 prepared for this project in its entirety. The associated changes are related to comments received from the City of Knoxville and TDOT, which are located in Appendix F.

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EXECUTIVE SUMMARY

This report provides a summary of a traffic impact study that was performed for a proposed singlefamily residential development on Buttermilk Road in Knox County, Tennessee. The project site is located on the south side of Buttermilk Road between the Graybeal Road intersections with Buttermilk Road. The development plan for this project consists of 102 single-family residential units. The proposed development will create a new full-movement access intersection onto Buttermilk Road.

The purpose of this study was the evaluation of the traffic operational and safety impacts of the proposed development upon roadways in the vicinity of the project site. Discussion with Knox County and Knoxville-Knox County Planning staff resulted in the proposed site access intersection at Buttermilk Road being identified for detailed study. Appropriate intersection evaluations such as capacity analyses, intersection sight distance analyses, and turn lane warrant evaluations were conducted at the study intersection for existing and future conditions, both with and without site generated traffic, in order to determine the anticipated impacts and to establish recommended measures to mitigate these impacts.

The primary conclusion of this study is that the traffic generated from the proposed development will not have a significant impact on the studied site access intersection at Buttermilk Road. Intersection levels-of-service are expected to be "A" during peak traffic periods for the site access intersection.

The following is a listing of recommendations that were developed to address traffic concerns in the vicinity of the project site:

- 1. Buttermilk Road at Proposed Site Access Point:
 - a. Install a northbound approach leg to create the intersection of Buttermilk Road at Proposed Site Access
- 2. Maintain intersection corner sight distances on the site access locations by ensuring that site grading, landscaping, signage, and other site features do not restrict intersection sight distance lines of sight.



INTRODUCTION & PURPOSE OF STUDY

This report provides a summary of a traffic impact study that was performed for a proposed singlefamily residential development on Buttermilk Road in Knox County, Tennessee. The project site is located on the south side of Buttermilk Road between the Graybeal Road intersections with Buttermilk Road. FIGURE 1 is a location map showing the major roadways in the project site vicinity.



FIGURE 1 LOCATION MAP

The development plan for this project consists of 102 single-family residential units. The proposed development will create a new full-movement access intersection onto Buttermilk Road. FIGURE 2 is a Conceptual Site Plan detailing the proposed site.

The purpose of this study was the evaluation of the traffic operational and safety impacts of the proposed development upon roadways in the vicinity of the project site. Discussion with Knox County and Knoxville-Knox County Planning staff resulted in the proposed site access intersection at Buttermilk Road being identified for detailed study. Appropriate intersection evaluations such as capacity analyses, intersection sight distance analyses, and turn lane warrant evaluations were conducted at the study intersection for existing and future conditions, both with and without site generated traffic, in order to determine the anticipated impacts and to establish recommended measures to mitigate these impacts.



SECTION 2

INTRODUCTION & PURPOSE OF STUDY



FIGURE 2 CONCEPTUAL SITE PLAN



EXISTING CONDITIONS

EXISTING ROADWAY CONDITIONS

Roadway conditions for the study roadways are summarized as follows:

• Buttermilk Road is a two-lane roadway with one lane in each direction within the vicinity of the proposed site. It is classified as a minor collector per the Knoxville-Knox County Planning Major Road Plan. Lane widths are 9 feet with open shoulders on both sides of Buttermilk Road and the posted speed limit is 30 mph.

Traffic control for the study intersection is as follows:

• Proposed Site Access Point at Buttermilk Road is proposed to be side-street STOP controlled.

EXISTING SITE CONDITIONS

The proposed development is located on the south side of Buttermilk Road between the Graybeal Road intersections with Buttermilk Road. The site will be adjacent to existing single family residential development along Buttermilk Road. Additionally, the proposed site slopes down from Buttermilk Road to a gentle rolling landscape. FIGURE 3 provides an aerial view of the project site and the surrounding area.





FIGURE 3 EXISTING SITE CONDITIONS

EXISTING TRAFFIC DATA

Existing traffic data was gathered for this study from Knoxville-Knox County Planning. Knoxville-Knox County Planning collects annual average daily traffic (AADT) data on roadways in the study area. One count station was found near the project site that was felt to have particular relevance for this study. The most currently available data from this station is contained in Table 1.

TABLE 1: ANNUAL AVERAGE DAILY TRAFFIC COUNT SUMMARY										
COUNT YEAR	COUNT STATION 093M276 BUTTERMILK ROAD WEST OF PROPOSED SITE / EAST OF EVERETT ROAD									
2013	450									
2015	530									
2017	520									
2019	490									
2022	518									

DALLY TRAFFIC COUNT CUMMAADY

The adjacent intersection traffic was utilized as the 2022 existing traffic volumes for this study since the study intersections do not currently exist. The existing ADT data is summarized in FIGURE 4 and the count summary sheets are contained in APPENDIX A.



EXISTING CAPACITY ANALYSES / LEVELS-OF-SERVICE

Capacity analyses employing the methods of the *Highway Capacity Manual* are typically conducted for the existing conditions at the study intersections. Since the studied intersection is currently proposed and does not exist, existing capacity analysis could not be performed. APPENDIX C contains a section entitled "Capacity and Level of Service Concepts", which provides a description of the utilized procedures.



SECTION 3 EXISTING CONDITIONS

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FIGURE 4 2022 EXISTING TRAFFIC VOLUMES



BACKGROUND CONDITIONS

BACKGROUND TRAFFIC GROWTH

The proposed development is anticipated to be constructed in one general phase with completion anticipated by 2025. Therefore, year 2025 was established as the appropriate design / analysis year for the study. In order to determine traffic volumes resulting solely from background traffic growth to year 2025, it was necessary to establish an annual growth rate for existing traffic. In addition to the AADT values previously discussed, knowledge of the surrounding area was used to determine an approximate annual growth rate. Based on the available data and anticipated growth in the area, a background annual growth rate of five percent was assumed. FIGURE 5 contains the background traffic volumes that would result from this annual growth rate from year 2022 to year 2025.

BACKGROUND CAPACITY ANALYSES / LEVELS-OF-SERVICE

Capacity analyses employing the methods of the *Highway Capacity Manual* are typically conducted for the background conditions at the study intersection to provide a baseline for comparing build and nobuild traffic scenarios. Since the studied intersection is currently proposed and does not exist, background capacity analysis could not be performed.



SECTION 4

BACKGROUND CONDITIONS



FIGURE 5 2025 BACKGROUND TRAFFIC VOLUMES



FUTURE CONDITIONS

TRIP GENERATION

In order to estimate the expected traffic volumes to be generated by the proposed development, the procedures of *Trip Generation*, 11^{TH} *Edition* (Institute of Transportation Engineers-ITE) were utilized. The generated traffic volumes were determined based on the data for the peak hours of adjacent street traffic for Land Use Code 210 (Single Family Detached Housing). See TABLE 2 for a summary of the traffic generated for this project. More detailed information is contained in APPENDIX B.

LAND USE	ITE CODE	SIZE	WEEKDAY (TRIPS/DAY)	AM PEAK HOUR (TRIPS/HOUR)	PM PEAK HOUR (TRIPS/HOUR)
Single Family Detached Housing	ly Detached 210 102 Units		1,028	76	101
	E	ntering Trips Exiting Trips	514 (50%) 514 (50%)	20 (26%) 56 (74%)	64 (63%) 37 (37%)

TABLE 2: TRIP GENERATION SUMMARY

TRIP DISTRIBUTION AND ASSIGNMENT

The proposed trip distribution for this development was determined through a review of existing travel patterns, local knowledge of the study area, proposed site location in relation to surrounding roadway network, and engineering judgment. FIGURE 6 provides a summary of how the above site generated trips would be assigned to the study intersections. FIGURE 7 provides the proposed trip assignment volumes to the studied intersections.

FUTURE TRAFFIC VOLUMES

Future projected traffic volumes for the study intersections were developed by adding the generated and assigned trips shown in FIGURE 7 to the 2025 background traffic volumes developed in the previous section and shown in FIGURE 5. These combined 2025 volumes reflect the existing traffic, the background traffic growth, and the generated traffic from the proposed development. These future volumes are shown on FIGURE 8 and are the combined volumes used in the analyses of future conditions with the proposed development.



FUTURE CAPACITY ANALYSES / LEVELS-OF-SERVICE

Capacity analyses were conducted for future conditions utilizing the traffic volumes shown in the buildout scenario. These analyses utilized proposed intersection traffic control and proposed lane configurations to determine if any mitigation is required to accommodate traffic generated by the proposed site. Tabular summaries of the analysis results and associated discussion are also contained in the EVALUATIONS section. In addition, detailed computer printout summaries of the analyses are contained in APPENDIX C.



SECTION 5 FUTURE CONDITIONS

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FIGURE 6 TRIP DISTRIBUTION



SECTION 5 FUTURE CONDITIONS

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FIGURE 7 TRIP ASSIGNMENT



SECTION 5 FUTURE CONDITIONS

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FIGURE 8 2025 COMBINED TRAFFIC VOLUMES



EVALUATIONS

INTERSECTION CAPACITY ANALYSES

As discussed in the preceding sections of this report, capacity analyses employing the methods of the Highway Capacity Manual (HCM 6th Edition) were conducted for the study intersections. These analyses were performed for the previously discussed development scenario. A summary of the capacity analyses results is shown in TABLE 3, while the resulting conclusions and recommendations are covered in the CONCLUSIONS and RECOMMENDATIONS section of this report. The complete capacity analysis reports are contained in APPENDIX C.

TABLE 3: CAPACITY ANALYSES SUMMARY

INTERSECTION	TIME PERIOD	YEAR 2022 EXISTING (LOS/DELAY)	YEAR 2025 BACKGROUND (LOS/DELAY)	YEAR 2025 COMBINED (LOS/DELAY)
Buttermilk Road at Access Point ¹ SIDE STREET STOP CONTROL – NB APPROACH	A.M. P.M.	-	-	A 9.3 A 9.9

¹SIDE STREET STOP CONTROL – Data shown are Level-of-Service and Average Vehicular Delay (seconds) for the critical side street approaches and major street left turn movements utilizing HCM methodology.

TURN LANE ASSESSMENTS

Turn lane warrant evaluations were conducted at the studied intersection of Buttermilk Road at Proposed Site Access. This evaluation found that neither a left-turn nor a right-turn lane were warranted during the studied peak hours at the studied intersection. The evaluations utilized Knox County left and right-turn lane volume thresholds. The spreadsheets summarizing these evaluations are contained in APPENDIX D.

SIGHT DISTANCE ASSESSMENT

Intersection sight distance was assessed looking both directions from the proposed site driveway intersection. Based on AASHTO sight distance requirements for 30 mph roadways, 335 feet of sight distance is required to make a left turn and 290 feet of sight distance is required to make a right turn from a side street stop-controlled scenario. Additionally, Knoxville-Knox County Subdivision Regulations state, "The minimum sight distance at an intersection (in both directions along the major street) shall be ten (10) times the posted speed limit, but in no case shall it be less than 250 feet."

Sight distance field measurements were ultimately inconclusive due to the excessive sight limiting vegetation surrounding the proposed site access. Field measurements indicate that 72 feet of sight distance is currently available when looking right and 79 feet of sight distance is currently available when looking left.

However, once the vegetation is cleared, it is believed sight distance requirements looking to the right from the proposed access can be achieved as the eastern intersection of Graybeal Road at Buttermilk Road (roughly 600 feet from the proposed access) was visible when stepping out beyond the sight limiting vegetation. Additionally, sight distance requirements looking left can likely be achieved if enough



sight limiting vegetation is cleared as sight distance was measured around 435 feet when stepping out beyond the sight limiting vegetation.

Care should be taken during the site development process to ensure that site features such as landscaping and signage to do not restrict the existing sight distances. Furthermore, clear sight triangles in both directions measured 15' back from the edge of traveled way along the proposed access point shall be provided. Coordination with the developer will be needed to ensure that the sight triangles can be achieved within the development's property limits, public right-of-way, or other easements as necessary.



CONCLUSIONS & RECOMMENDATIONS

The primary conclusion of this study is that the traffic generated from the proposed development will not have a significant impact on the studied site access intersection at Buttermilk Road. Intersection levels-of-service are expected to be "A" during peak traffic periods for the site access intersection.

The following is a listing of recommendations that were developed to address traffic concerns in the vicinity of the project site:

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SECTION 8 APPENDIX

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APPENDIX

APPENDIX ORDER:

- A. TRAFFIC DATA
- **B. TRIP GENERATION INFORMATION**
- C. CAPACITY ANALYSES
- D. TURN LANE WARRANT SHEETS



APPENDIX A

TRAFFIC DATA

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APPENDIX A - TRAFFIC DATA



Prepared by National Data & Surveying Services

VOLUME - RAW

Buttermilk Rd E/O Hickory Creek Rd (35.890518°,-84.243923°)

Day: Tuesday Date: 4/5/2022

City:	Knoxv	ille	
Project #:	TN22	190004	_055

AM Period NB SB EB WB TOTAL PM Period NB SB EB WB TOTAL 0:00 0 0 2 12:30 2 5 3 8 0:01 0 0 0 12:30 2 5 7 0:43 0 0 2 2 12:30 2 5 7 0:43 0 0 2 12:45 6 17 1 1 6 28 1:45 0 0 0 1 1 1 2 13:30 3 3 3 6 2:15 0 0 0 14:15 2 1 3 7 0 7 1 7 0 7 1 3 3 3 5 8 5 2 1 3 3 3 3 3 3 3 3 3 3 3		DAILY TOTALS		_	NB		SB		EB	WB						Тс	otal
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DALLY TOTALS NB SB EB WB Total			J		-									-		-	
	SPLIT %			55.1%		44.9%		41.3%	SPLIT %				53.9%		46.1%		58.7%
					NIR-		CP.		EP	W/P							tal
		DAILY TOTALS		-			-		-		•					-	

				0	0	282	236				518
AM Peak Hour			7:45	6:45	7:45	PM Peak Hour			17:00	17:00	17:00
AM Pk Volume			30	22	46	PM Pk Volume			40	38	78
Pk Hr Factor			0.682	0.611	0.676	Pk Hr Factor			0.714	0.792	0.813
7 - 9 Volume	0	0	48	38	86	4 - 6 Volume	0	0	70	60	130
7 - 9 Peak Hour			7:45	7:00	7:45	4 - 6 Peak Hour			17:00	17:00	17:00
7 - 9 Pk Volume			30	22	46	4 - 6 Pk Volume			40	38	78
Pk Hr Factor	0.000	0.000	0.682	0.611	0.676	Pk Hr Factor	0.000	0.000	0.714	0.792	0.813

Prepared by National Data & Surveying Services VOLUME - SEASONAL FACTOR OF 1.30

Buttermilk Rd E/O Hickory Creek Rd (35.890518°,-84.243923°)

Day: Tuesday Date: 4/5/2022

City:	Knoxv	ille	
Project #:	TN22_	190004	_055

				NB		SB		EB	WB	3					То	tal
	DAILY TOTALS		-	0		0		368	308	_					-	75
AM Period	NB SB	EB		WB		TO	TAL	PM Period	NB	SB	EB		WB		TO	TAL
0:00		0		3		3		12:00			7		4		10	
0:15		0		0		0		12:15			3		7		9	
0:30		0		0		0		12:30			5	~~	4		9	
0:45		0		0	3	0	3	12:45			8	22	0	14	8	36
1:00 1:15		0 0		0 0		0		13:00 13:15			0 0		1 4		1 4	
1:15		1		1		0 3		13:30			4		4		4 8	
1:45		0	1	0	1	0	3	13:45			7	10	7	16	13	26
2:00		0	-	0	-	0		14:00			8	10	8	10	16	20
2:15		Õ		Õ		Õ		14:15			3		1		4	
2:30		0		Ō		0		14:30			9		Ō		9	
2:45		0		0		0		14:45			5	25	3	12	8	36
3:00		0		0		0		15:00			4		7		10	
3:15		0		1		1		15:15			4		4		8	
3:30		0		0		0		15:30			3		5		8	
3:45		0		0	1	0	1	15:45			3	13	5	21	8	34
4:00		0		1		1		16:00			5		9		14	
4:15		0		1		1		16:15			16		7		22	
4:30		0		0	2	0		16:30			13	20	7	20	20	60
4:45 5:00		<u>1</u> 0	1	0	3	1	4	16:45 17:00			<u>5</u> 9	39	7	29	12 18	68
5:15		1		1		3		17:15			9 14		9 12		26	
5:30		0		3		3		17:30			14		12		26	
5:45		0	1	4	8	4	9	17:45			10	52	13	50	31	102
6:00		1	-	4	0	5	5	18:00			5	52	8	50	13	102
6:15		5		1		7		18:15			4		5		9	
6:30		5		5		10		18:30			1		4		5	
6:45		7	18	4	14	10	33	18:45			4	14	1	18	5	33
7:00		9		5		14		19:00			4		3		7	
7:15		4		8		12		19:15			4		1		5	
7:30		5		12		17		19:30			4		1		5	
7:45		9	27	4	29	13	56	19:45			1	13	1	7	3	20
8:00		4		5		9		20:00			1		3		4	
8:15		12		4		16		20:15			3		5		8	
8:30		14	25	8	24	22	50	20:30			5	40	0	0	5	20
8:45		5	35	4	21	9	56	20:45			1	10	1	9	3	20
9:00 9:15		4 5		1 9		5 14		21:00 21:15			1 4		0		1 5	
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10:15		4		3		7		22:15			3		Ō		3	
10:30		8		3		10		22:30			1		õ		1	
10:45		1	16	3	12	4	27	22:45			1	5	Ō	1	1	7
11:00		1		7		8		23:00			1		0		1	
11:15		9		3		12		23:15			0		0		0	
11:30		13		3		16		23:30			1		1		3	
11:45		4	27	3	14	7	42	23:45			1	4	1	3	3	7
TOTALS			154		125		279	TOTALS				214		182		396
SPLIT %			55.1%		44.9%		41.3%	SPLIT %				53.9%		46.1%		58.7%
				NB		SB		EB	WB	3					To	tal
	DAILY TOTALS			0		0		368	308	3					6	75

				0	U	368	308				6/5
AM Peak Hour			7:45	6:45	7:45	PM Peak Hour			17:00	17:00	17:00
AM Pk Volume			39	29	60	PM Pk Volume			52	50	102
Pk Hr Factor			0.682	0.611	0.676	Pk Hr Factor			0.714	0.792	0.813
7 - 9 Volume	0	0	63	50	112	4 - 6 Volume	0	0	91	78	169
7 - 9 Peak Hour			7:45	7:00	7:45	4 - 6 Peak Hour			17:00	17:00	17:00
7 - 9 Pk Volume			39	29	60	4 - 6 Pk Volume			52	50	102
Pk Hr Factor	0.000	0.000	0.682	0.611	0.676	Pk Hr Factor	0.000	0.000	0.714	0.792	0.813

Prepared by National Data & Surveying Services VOLUME - ADJUSTMENT FACTOR OF 1.50

Buttermilk Rd E/O Hickory Creek Rd (35.890518°,-84.243923°)

Day: Tuesday Date: 4/5/2022

City:	Knoxv	rille	
Project #:	TN22_	190004	_055

				NB		SB		EB	WB						То	otal
	DAILY TOTA	LS	-	0		0		551	461	-					-	013
AM Period	NB SB	EB		WB		то	TAL	PM Period	NB	SB	EB		WB		TO	TAL
0:00		0		4		4		12:00			10		6		16	
0:15		0		0		0		12:15			4		10		14	
0:30		0		0		0		12:30			8	22	6	22	14	
0:45		0		0	4	0	4	12:45 13:00			<u>12</u> 0	33	0	22	<u>12</u> 2	55
1:15		0		0		0		13:15			0		6		6	
1:30		2		2		4		13:30			6		6		12	
1:45		0	2	0	2	0	4	13:45			10	16	10	23	20	39
2:00		0	-	0	_	0		14:00			12	10	12	20	23	
2:15		0		Ō		0		14:15			4		2		6	
2:30		0		0		0		14:30			14		0		14	
2:45		0		0		0		14:45			8	37	4	18	12	55
3:00		0		0		0		15:00			6		10		16	
3:15		0		2		2		15:15			6		6		12	
3:30		0		0		0		15:30			4		8		12	
3:45		0		0	2	0	2	15:45			4	20	8	31	12	51
4:00		0		2		2		16:00			8		14		22	
4:15		0		2		2		16:15			23		10		33	
4:30		0		0		0	~	16:30			20		10		29	
4:45		2	2	0	4	2	6	16:45			8	59	10	43	18	102
5:00		0		0		0		17:00			14		14		27	
5:15		2 0		2 4		4		17:15 17:30			22		18		39	
5:30 5:45		0	2	4 6	12	4 6	14	17:30			16 27	78	23 20	74	39 47	152
6:00		2	Z	6	12	8	14	17:45			8	78	12	74	20	152
6:15		2		2		8 10		18:00			8 6		8		20 14	
6:30		8		8		16		18:30			2		6		8	
6:45		10	27	6	22	16	49	18:45			6	22	2	27	8	49
7:00		10	21	8	22	22	75	19:00			6	22	4	21	10	-+5
7:15		6		12		18		19:15			6		2		8	
7:30		8		18		25		19:30			6		2		8	
7:45		14	41	6	43	20	84	19:45			2	20	2	10	4	29
8:00		6		8	-	14		20:00			2		4		6	
8:15		18		6		23		20:15			4		8		12	
8:30		22		12		33		20:30			8		0		8	
8:45		8	53	6	31	14	84	20:45			2	16	2	14	4	29
9:00		6		2		8		21:00			2		0		2	
9:15		8		14		22		21:15			6		2		8	
9:30		12		8		20		21:30			0		4		4	
9:45		14	39	6	29	20	68	21:45			0	8	0	6	0	14
10:00		4		6		10		22:00			0		2		2	
10:15		6		4		10		22:15			4		0		4	
10:30		12 2	22	4 4	10	16	41	22:30 22:45			2	0	0	2	2	10
10:45 11:00		2	23	4	18	6 12	41	22:45			2	8	0	2	2	10
11:00		2 14		4		12		23:00			2		0		2	
11:15		20		4		23		23:15			2		2		4	
11:45		20	41	4	22	10	63	23:45			2	6	2	4	4	10
TOTALS		0	231	-	188	10	418	TOTALS			<u> </u>	321		274	-	594
SPLIT %			55.1%		44.9%		41.3%	SPLIT %				53.9%		46.1%		58.7%
		_		NID		CD				_			_		To	
	DAILY TOTA	LS	-	NB 0		SB 0		EB 551	WB 461	-						otal 013
				- 0					401							122

				0	0	551	461				1013
AM Peak Hour			7:45	6:45	7:45	PM Peak Hour			17:00	17:00	17:00
AM Peak Hour			59	43	90	PM Pk Volume			78	74	17.00
Pk Hr Factor			0.682	0.611	0.676	Pk Hr Factor			0.714	0.792	0.813
7 - 9 Volume	0	0	94	74	168	4 - 6 Volume	0	0	137	117	254
7 - 9 Peak Hour			7:45	7:00	7:45	4 - 6 Peak Hour			17:00	17:00	17:00
7 - 9 Pk Volume			59	43	90	4 - 6 Pk Volume			78	74	152
Pk Hr Factor	0.000	0.000	0.682	0.611	0.676	Pk Hr Factor	0.000	0.000	0.714	0.792	0.813

APPENDIX B TRIP GENERATION

APPENDIX B - TRIP GENERATION INFORMATION



Land Use: 210 Single-Family Detached Housing

Description

A single-family detached housing site includes any single-family detached home on an individual lot. A typical site surveyed is a suburban subdivision.

Specialized Land Use

Data have been submitted for several single-family detached housing developments with homes that are commonly referred to as patio homes. A patio home is a detached housing unit that is located on a small lot with little (or no) front or back yard. In some subdivisions, communal maintenance of outside grounds is provided for the patio homes. The three patio home sites total 299 dwelling units with overall weighted average trip generation rates of 5.35 vehicle trips per dwelling unit for weekday, 0.26 for the AM adjacent street peak hour, and 0.47 for the PM adjacent street peak hour. These patio home rates based on a small sample of sites are lower than those for single-family detached housing (Land Use 210), lower than those for single-family attached housing (Land Use 251), and higher than those for senior adult housing -- single-family (Land Use 251). Further analysis of this housing type will be conducted in a future edition of *Trip Generation Manual*.

Additional Data

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (https://www.ite.org/technical-resources/topics/trip-and-parking-generation/).

For 30 of the study sites, data on the number of residents and number of household vehicles are available. The overall averages for the 30 sites are 3.6 residents per dwelling unit and 1.5 vehicles per dwelling unit.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Arizona, California, Connecticut, Delaware, Illinois, Indiana, Kentucky, Maryland, Massachusetts, Minnesota, Montana, New Jersey, North Carolina, Ohio, Ontario (CAN), Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Vermont, Virginia, and West Virginia.

Source Numbers

100, 105, 114, 126, 157, 167, 177, 197, 207, 211, 217, 267, 275, 293, 300, 319, 320, 356, 357, 367, 384, 387, 407, 435, 522, 550, 552, 579, 598, 601, 603, 614, 637, 711, 716, 720, 728, 735, 868, 869, 903, 925, 936, 1005, 1007, 1008, 1010, 1033, 1066, 1077,1078, 1079



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units On a: Weekday

Setting/Location:	General Urban/Suburban
-------------------	------------------------

Number of Studies:	174
Avg. Num. of Dwelling Units:	246
Directional Distribution:	50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.43	4.45 - 22.61	2.13

Data Plot and Equation



Trip Gen Manual, 11th Edition

• Institute of Transportation Engineers

Single-Family Detached Housing (210)

Vehicle Trip Ends vs: On a:	Dwelling Units Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.
Setting/Location:	General Urban/Suburban
Number of Studies:	192
Avg. Num. of Dwelling Units:	226
Directional Distribution:	26% entering, 74% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.70	0.27 - 2.27	0.24

Data Plot and Equation



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Single-Family Detached Housing (210)

Vehicle Trip Ends vs: On a:	Dwelling Units Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.
Setting/Location:	General Urban/Suburban
Number of Studies:	208
Avg. Num. of Dwelling Units:	248
	63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.94	0.35 - 2.98	0.31

Data Plot and Equation



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APPENDIX C CAPACITY ANALYSES

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APPENDIX C - CAPACITY ANALYSES



CAPACITY AND LEVEL-OF-SERVICE CONCEPTS

In a general sense, a roadway is similar to a pipeline or other material carrying conduit in that it has a certain capacity for the amount of material (vehicles) that it can efficiently carry. As the number of vehicles in a given time period gradually increases, the quality of traffic flow gradually decreases. On roadway sections this results in increasing turbulence in the traffic stream, and at intersections it results in increasing stops and delay. As the volumes begin to approach the capacity of the facility, these problems rapidly magnify, with resulting serious levels of congestion, stops, delay, excess fuel consumption, pollutant emissions, etc.

The Transportation Research Board has published the <u>Year 2010 Highway Capacity Manual</u> (HCM2010), which establishes theoretical techniques to quantify the capacity conditions on all types of roadways, intersections, ramps, pedestrian facilities, etc. A basic concept that is applicable to most of these techniques is the idea of level of service (LOS). This concept establishes a rating system that quantifies the quality of traffic flow, as perceived by motorists and/or passengers. The general system is similar to a school grade scale, and is outlined as follows:

Level of Service (LOS)	General Quality of Traffic Flow	Description of Corresponding Conditions
A	Excellent	Roadways – Free flow, high maneuverability Intersections – Very few stops, very low delay
В	Very Good	Roadways – Free flow, slightly lower maneuverability Intersections – Minor stops, low delay
С	Good	Roadways – Stable flow, restricted maneuverability Intersections – Significant stops, significant delay
D	Fair	Roadways – Marginally stable flow, congestion seriously restricts maneuverability Intersections – High stops, long but tolerable delay
Е	Poor	Roadways – Unstable flow*, lower operating speeds, congestion severely restricts maneuverability Intersections – All vehicles stop, very long queues and very long intolerable delay
F	Very Poor	Roadways – Forced flow, stoppages may be lengthy, congestion severely restricts maneuverability Intersections – All vehicles stop, extensive queues and extremely long intolerable delay

*Unstable flow is such that minor fluctuations or disruptions can result in rapid degradation to LOS F.

LOS	CONTROL DELAY (S/VEH)							
203	SIGNALIZED	UNSIGNALIZED	ROUNDABOUT					
A	≤10	≤10	≤10					
В	>10-20	>10-15	>10-15					
С	>20-35	>15-25	>15-25					
D	>35-55	>25-35	>25-35					
E	>55-80	>35-50	>35-50					
F	>80	>50	>50					

LOS CRITERIA: SIGNALIZED & UNSIGNALIZED INTERSECTIONS

Another measure of intersection capacity that is often used in the evaluation of intersection operations is the volume to capacity (V/C) ratio. This ratio is defined as "the ratio of flow rate to capacity", and is a good measure of how much of an intersection's available capacity has been used up by the analysis volumes. Conversely, it also provides an indication of the reserve capacity available for future growth in traffic volumes.

The Intersection Capacity Utilization (ICU) is another measure that expresses a value similar to the V/C ratio. Specifically, the ICU method "sums the amount of the time required to serve all movements at saturation for a given cycle length and divides by that reference cycle length." The ICU is considered a more accurate measure of volume to capacity conditions for a signalized intersection, primarily because it accounts for the effects of the signal timing on intersection capacity.

HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Wesley Stokes	Intersection	Buttermilk at Site Access						
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County						
Date Performed	5/26/2022	East/West Street	Buttermilk Road						
Analysis Year	2022	North/South Street	Site Access						
Time Analyzed	AM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	2025 Combined								
Lanes									



Vehicle Volumes and Adjustments

Approach		Eastb	ound			West	bound			North	bound		Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			69	12		8	37			34		22				
Percent Heavy Vehicles (%)						3				3		3				
Proportion Time Blocked																
Percent Grade (%)										()					
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						4.1				7.1		6.2				
Critical Headway (sec)						4.13				6.43		6.23				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)						2.23				3.53		3.33				
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)						9					61					
Capacity, c (veh/h)						1501					893					
v/c Ratio						0.01					0.07					
95% Queue Length, Q ₉₅ (veh)						0.0					0.2					
Control Delay (s/veh)						7.4					9.3					
Level of Service (LOS)						А					А					
Approach Delay (s/veh)					1.4			9.3								
Approach LOS										-	4					

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HCS7 Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	Wesley Stokes	Intersection	Buttermilk at Site Access						
Agency/Co.	Cannon & Cannon, Inc.	Jurisdiction	Knox County						
Date Performed	5/26/2022	East/West Street	Buttermilk Road						
Analysis Year	2022	North/South Street	Site Access						
Time Analyzed	PM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	2025 Combined								
Lanes									



Vehicle Volumes and Adjustments

Venicle volumes and Adj	ustme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0
Configuration				TR		LT					LR					
Volume (veh/h)			90	38		26	86			22		15				
Percent Heavy Vehicles (%)						3				3		3				
Proportion Time Blocked																
Percent Grade (%)										(0					
Right Turn Channelized																
Median Type Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)						4.1				7.1		6.2				
Critical Headway (sec)						4.13				6.43		6.23				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)						2.23				3.53		3.33				
Delay, Queue Length, an	d Leve	l of Se	ervice	i -												
Flow Rate, v (veh/h)						28					40					
Capacity, c (veh/h)						1438					781					
v/c Ratio						0.02					0.05					
95% Queue Length, Q ₉₅ (veh)						0.1					0.2					
Control Delay (s/veh)						7.6					9.9					
Level of Service (LOS)						A					A					
Approach Delay (s/veh)						1	.9		9.9							
Approach LOS										/	4					

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HCSTM TWSC Version 7.9 Buttermilk at Site Access - Combined PM.xtw Generated: 7/6/2022 2:26:52 PM

APPENDIX D

TURN LANE WARRANT SHEETS

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APPENDIX D - TURN LANE WARRANT SHEETS



TABLE 4A	Project No: 00630-0002
KNOX COUNTY LEFT-TURN LANE VOLUME THRESHOLDS	Project Name: Buttermilk Rd Single Family
FOR 2-LANE ROADWAYS WITH A PREVAILING SPEED OF 0 TO 35 MPH	Notes:

OPPOSING	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *									
VOLUME	100 - 149	150 - 199	200 - 249	250 - 299	300 - 349	350 - 399				
100 - 149	300	235	185	145	120	100				
150 - 199	245	200	160	130	110	90				
200 - 249	205	170	140	115	100	80				
250 - 299	175	150	125	105	90	70				
300 - 349	155	135	110	95	80	65				
350 - 399	135	120	100	85	70	60				
400 - 449	120	105	90	75	65	55				
450 - 499	105	90	80	70	60	50				
500 - 549	95	80	70	65	55	50				
550 - 599	85	70	65	60	50	45				
600 - 649	75	65	60	55	45	40				
650 - 699	70	60	55	50	40	35				
700 - 749	65	55	50	45	35	30				
750 or More	60	50	45	40	35	30				

(If the left-turn volume exceeds the table value a left-turn lane is needed)

OPPOSING	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *									
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 599	= / > 600				
100 - 149	100	80	70	60	55	50				
150 - 199	90	75	65	55	50	45				
200 - 249	80	72	60	55	50	45				
250 - 299	70	65	55	50	45	40				
300 - 349	65	60	50	50	45	40				
350 - 399	60	55	50	45	40	40				
400 - 449	55	50	45	45	40	35				
450 - 499	50	45	45	40	35	35				
500 - 549	50	45	40	40	35	35				
550 - 599	45	40	40	35	35	35				
600 - 649	40	35	35	35	35	30				
650 - 699	35	35	35	30	30	30				
700 - 749	30	30	30	30	30	30				
750 or More	30	30	30	30	30	30				

* Or through volume only if a right-turn lane exists

Intersection	Time Period	Opposing Volume	Through Volume	Left-Turn Volume	Warrant Threshold	Left-Turn Lane Warranted (Yes / No)
Site @ Buttermilk	AM Peak	81	37	8	N/A	NO
Site @ Buttermilk	PM Peak	128	86	26	300	NO

TABLE 4B
KNOX COUNTY RIGHT-TURN LANE VOLUME THRESHOLDS
FOR 2-LANE ROADWAYS WITH A PREVAILING SPEED OF 0 TO 35 MPH

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *									
VOLUME	< 100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399				
Fewer Than 25										
25 - 49										
50 - 99										
100 - 149										
150 - 199										
200 - 249										
250 - 299						Yes				
300 - 349					Yes	Yes				
350 - 399				Yes	Yes	Yes				
400 - 449			Yes	Yes	Yes	Yes				
450 - 499			Yes	Yes	Yes	Yes				
500 - 549		Yes	Yes	Yes	Yes	Yes				
550 - 599		Yes	Yes	Yes	Yes	Yes				
600 or More	Yes	Yes	Yes	Yes	Yes	Yes				

RIGHT-TURN	THROUGH VOLUME PLUS LEFT-TURN VOLUME *									
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 599	= / > 600				
Fewer Than 25										
25 - 49						Yes				
50 - 99					Yes	Yes				
100 - 149				Yes	Yes	Yes				
150 - 199			Yes	Yes	Yes	Yes				
200 - 249		Yes	Yes	Yes	Yes	Yes				
250 - 299	Yes	Yes	Yes	Yes	Yes	Yes				
300 - 349	Yes	Yes	Yes	Yes	Yes	Yes				
350 - 399	Yes	Yes	Yes	Yes	Yes	Yes				
400 - 449	Yes	Yes	Yes	Yes	Yes	Yes				
450 - 499	Yes	Yes	Yes	Yes	Yes	Yes				
500 - 549	Yes	Yes	Yes	Yes	Yes	Yes				
550 - 599	Yes	Yes	Yes	Yes	Yes	Yes				
600 or More	Yes	Yes	Yes	Yes	Yes	Yes				

* Or through volume only if a left-turn lane exists

Intersection	Time Period	Through Volume	Right-Turn Volume	Right-Turn Lane Warranted (Yes / No)
Site @ Buttermilk	AM Peak	69	12	NO
Site @ Buttermilk	PM Peak	90	38	NO

Source: Knox County Department of Engineering and Public Works "Access Control and Driveway Design Policy"

APPENDIX E

TIS COMMENT RESPONSE DOCUMENT

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APPENDIX E – TIS COMMENT RESPONSE DOCUMENT





Date: July 6, 2022

Project Name: Buttermilk Road Residential Development

To: Knox County Engineering and Public Works (EPW) and Knoxville-Knox County Planning

Subject: TIS Comment Response Document for Buttermilk Road Residential Development

Dear Knox County EPW and Knoxville-Knox County Planning Staff,

The following comment response document is submitted to address comments dated June 9, 2022 that were sent to Cannon & Cannon, Inc. on July 5, 2022:

1. <u>Reviewer Comment</u>: Please review the AM peak period volume reported for the eastbound direction as it appears to be too low (we derived 59 vehicles instead of the 41 shown) based on the count data and revise all analyses as appropriate.

<u>Response</u>: Comment addressed and analysis throughout the Revised TIS has been updated.

2. <u>Reviewer Comment</u>: In the second column of Table 3 please move the 'NB' abbreviation to the Intersection column in order to avoid confusion.

Response: Comment addressed in Table 3 on page 15 of the Revised TIS.

3. <u>Reviewer Comment</u>: The sight distance assessment needs to cite the controlling standard for minimum sight distance requirements in Knox County which are documented in the Knoxville-Knox County Subdivision Regulations as being 10 times the posted speed limit.

<u>Response</u>: Comment addressed on page 15 in the "Sight Distance Assessment" section of the Revised TIS.

4. <u>Reviewer Comment</u>: Include additional detail regarding the sight distance standards and the need to provide clear sight triangles in both directions measured from 15' back from the edge of the traveled way. Due to the narrowness of the development's access right-of-way there will need to be coordination with the site engineer to ensure that sight triangles can be achieved within this development's property limits, public right-of-way or other easements as necessary that will need to be shown on the site plan.

<u>Response</u>: Comment addressed on page 16 in the "Sight Distance Assessment" section of the Revised TIS.

5. <u>Reviewer Comment</u>: Please include a header or some other method of identifying the difference between the count data shown on Pages A-2 and A-4 as it is not immediately clear whether it is



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Knoxville, TN 37919BOWLING GREENFAX 865.670.8866

just a duplication. We understand that the second count includes the seasonal factor of 1.3 but this should be clearly noted and additionally a third count sheet could be provided showing the application of the 1.5 factor that was applied.

<u>Response</u>: Comment addressed in Appendix A of the Revised TIS.

Sincerely,



Wesley Stokes, P.E.