Traffic Impact Study

Fox Road Subdivision Knox County, Tennessee



June 9, 2003

Prepared for: S&E Properties, LLC 405 Montbrook Lane Knoxville, Tennessee 37919

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This report summarizes a traffic impact study that was prepared for the proposed Fox Road Subdivision, to be located on Fox Road in West Knoxville. The study resulted in the conclusions and recommendations discussed below:

It is the primary conclusion of this study that no significant traffic volume related impacts will result from the development of the Fox Road Subdivision. In fact, capacity analyses of proposed side street (2-way) stop traffic control, indicates that very good conditions (LOS "B" or better) can be expected during all time periods. In addition, analyses of the need for auxiliary traffic lanes such as left and right turning lanes, indicates that no such lanes will be warranted under the anticipated traffic conditions.

Intersection turning sight distance is the only issue of significant concern that was identified in this study. Specifically, the sight distance looking south from the proposed subdivision access roadway intersection at Fox Road was found to be somewhat deficient. However, it was determined that the cutting down and trimming of some trees and brush would provide more than the required sight distance. Therefore, such action to provide the required sight distance is recommended prior to opening the subdivision roadways to traffic.

1

INTRODUCTION AND PURPOSE OF STUDY

This report provides a summary of the traffic impact study that was performed for the proposed Fox Road Subdivision to be located on Fox Road in the west end of the City of Knoxville. The project site is approximately 1.1 miles south of Kingston Pike, and is adjacent to and immediately west of the Pellissippi Parkway. FIGURE 1 is a location map that identifies the project site in relation to the roadways in the vicinity of the proposed subdivision.

The concept plan for this project proposes a subdivision of 146 lots at full build-out. The subdivision entrance will be at a new three-leg intersection on Fox Road, located approximately 700 feet north of the existing Tan Rara Drive, and approximately 450 feet south of the existing Pipkin Lane. FIGURE 2 provides a detailed layout of the proposed subdivision as shown on the concept plan.

The purpose of this study was the evaluation of the traffic operational and safety impact of the proposed development upon the adjacent portion of Fox Road. Of particular interest was the proposed intersection of Fox Road with the subdivision main entrance roadway.

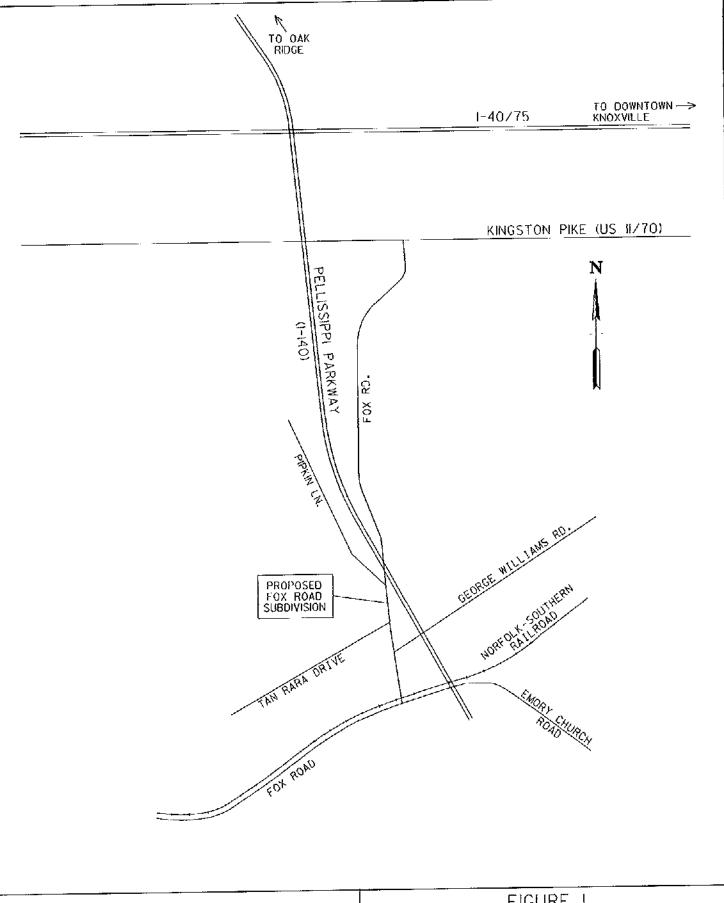
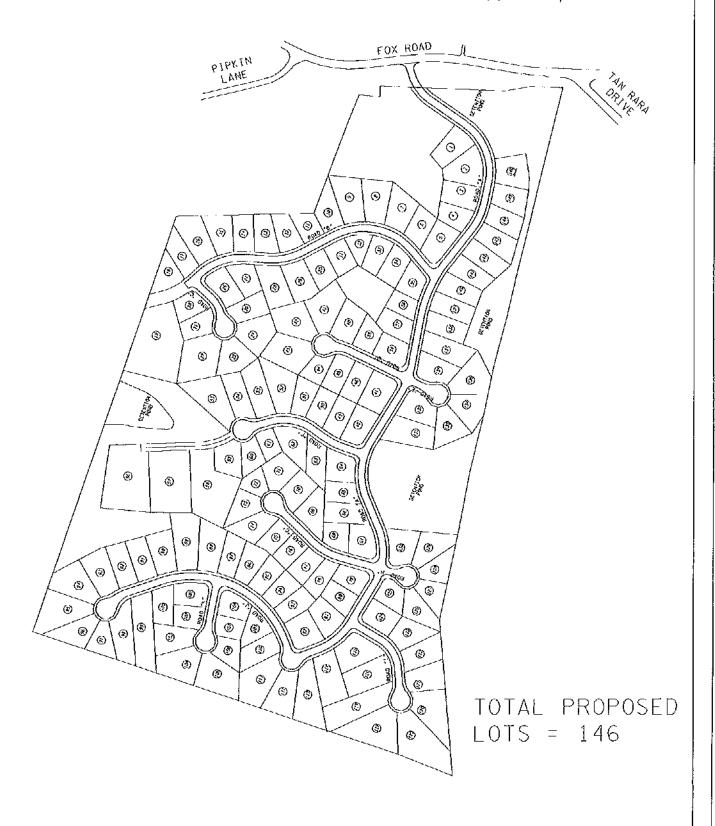




FIGURE | LOCATION MAP





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Civil Engineering - Field Surveying

FIGURE 2 SITE PLAN

Existing Roadway Conditions

Fox Road is a two-lane roadway that is classified by the Knoxville-Knox County Metropolitan Planning Commission (MPC) as a Minor Collector roadway. It is within the City limits of the City of Knoxville at the study location, and is thus maintained by the City of Knoxville. The roadway pavement consists of two 11-foot traffic lanes and two 5-foot paved shoulders. The speed limit is posted as 30 mph departing Kingston Pike, with no signing in the immediate vicinity of the proposed subdivision.

Existing Traffic Data

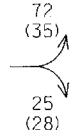
A traffic count station for collecting average daily traffic data (ADT) is located on Fox Road, a few hundred feet south of the proposed subdivision roadway. The most recent data was provided by MPC, with resulting ADTs of 2520 for year 2001 and 2686 for year 2003. A raw data summary sheet for the year 2003 count is contained in the APPENDIX.

In order to collect more refined data, and to establish a basis for trip distribution patterns, turning movement traffic counts were collected at the intersection of Fox Road and the Tan Rara Subdivision intersection, which is approximately 700 feet south of the proposed Fox Road Subdivision intersection. These counts were conducted during the A.M. and P.M. peak hours as established with the year 2003 ADT counts above. Raw data summary sheets for these counts are contained in the APPENDIX.

In addition to helping establish trip distribution patterns, these turning movement counts were used to establish the existing-background traffic volumes for this study. Specifically, the north-leg volumes from the counted intersection were used for this, as displayed on FIGURE 3. These volumes are the count data adjusted to an average weekday basis using adjustment factors developed by the University of Tennessee Transportation Research Center. In addition, another factor was applied to the A.M. data to adjust for the presence of school traffic. This was necessary since the turning movement counts were conducted after the closure for summer break of the nearby West Valley Middle School. The year 2003 ADT counts were conducted while school was in session, and they were used to develop the adjustment factor. See FIGURE 3 for additional information on these adjustments.



TAN RARA SUBDIVISION



TOP NO. - A.M. PEAK HOUR (7:30 - 8:30 A.M.) - A.M. AWD FACTOR = 0.98 (WED. IN JUNE)
(BOTTOM NO.) - P.M. PEAK HOUR (5:00 P.M. - 6:00 P.M.) - P.M. AWD FACTOR = 0.99 (TUES. IN JUNE)

NOTE:
THE DATA SHOWN ARE THE RAW TRAFFIC COUNT DATA TIMES A FACTOR TO ADJUST TO AN AVERAGE WEEKDAY VOLUME FROM COUNTS TAKEN IN JUNE, SEE APPENDIX FOR RAW COUNT DATA AND FACTOR TABLE, (FACTORS DEVELOPED BY THE UNIVERSITY OF TENNESSEE TRANSPORTATION RESEARCH CENTER), IN ADDITION, RAW A.M. COUNTS WERE MULTIPLIED BY A FACTOR OF 1,235 TO ADJUST COUNTS FOR SCHOOL TRAFFIC (COUNTS TAKEN IN EARLY JUNE - SCHOOL OUT), THIS FACTOR DEVELOPED FROM MPC MACHINE COUNT DATA TAKEN WHEN SCHOOL WAS IN SESSION.



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FIGURE 3
EXISTING BACKGROUND TRAFFIC DATA

Level of Service Evaluation

Intersection Capacity Analyses employing the methods of the Highway Capacity Manual (HCM 2000) were used to evaluate the proposed study intersection of Fox Road and the Fox Road Subdivision access roadway. However, since this intersection will not exist until the subdivision is constructed, such analyses were not possible for existing conditions. It should be noted that due to the low existing traffic volumes, Fox Road almost certainly currently operates at a Level of Service "A". Please see the following section for an explanation and discussion of Level of Service concepts.

Level of Service Concepts

In a general sense, a roadway is similar to a pipeline or other material carrying conduit in that it has a certain capacity for the amount of material (vehicles) that it can efficiently carry. As the number of vehicles in a given time period gradually increases, the quality of traffic flow gradually decreases. On roadway sections this results in increasing turbulence in the traffic stream, and at intersections it results in increasing stops and delay. As the volumes begin to approach the capacity of the facility, these problems rapidly magnify, with resulting serious levels of congestion, stops, delay, excess fuel consumption, pollutant emissions, etc.

The Federal Highway Administration has published the Year 2000 Highway Capacity Manual (HCM2000), which establishes theoretical techniques to quantify the capacity conditions on all types of roadways, intersections, ramps, pedestrian facilities, etc. A basic concept that is applicable to most of these techniques is the idea of level of service (LOS). This concept establishes a rating system that quantifies the quality of traffic flow, as perceived by motorists and/or passengers. The general system is similar to a school grade scale, and is outlined as follows:

Level of Service (LOS)	General Quality of Traffic Flow	Description of Corresponding Conditions
Λ	Excellent	Roadways – Free flow, high maneuverability Intersections – Very few stops, very low delay
В	Very Good	Roadways Free flow, slightly lower maneuverability Intersections - Minor stops, low delay
С	Good	Roadways – Stable flow, restricted maneuverability Intersections – Significant stops, significant delay
D	Fair	Roadways - Marginally stable flow, congestion seriously restricts maneuverability Intersections - High stops, long but tolerable delay
Е	Poor	Roadways — Unstable flow*, lower operating speeds, congestion severely restricts maneuverability Intersections — All vehicles stop, very long queues and very long intolerable delay
F	Very Poor	Roadways – Forced flow, stoppages may be lengthy, congestion severely restricts maneuverability Intersections – All vehicles stop, extensive queues and extremely long intolerable delay

^{*}Unstable flow is such that minor fluctuations or disruptions can result in rapid degradation to LOS F.

Background Traffic Growth

The anticipated time for full build-out of the Fox Road Subdivision is 5 years. Therefore, year 2008 was established as the appropriate design/analysis year for this study. In order to determine traffic volumes resulting solely from background traffic growth to year 2008, it was necessary to establish an annual growth rate for existing traffic. The ADT values that were previously discussed represent a 3.1 percent annual growth. Projecting this growth pattern forward, and rounding up to be conservative, results in an annual growth rate of 3.5 percent. FIGURE 4 contains the background traffic volumes that would result from a 3.5 percent annual growth from year 2003 to 2008.

Trip Generation

In order to estimate the expected traffic volumes to be generated by full build-out of the proposed Fox Road Subdivision, the data and procedures of *Trip Generation*, *Sixth Edition* (Institute of Transportation Engineers, 1997) were utilized. The generated traffic volumes were determined based on the total weekday morning, and evening peak hour of adjacent street traffic regression equations for single-family detached housing development (Land Use Code 210, Volume 1, pages 263 to 265). As noted earlier in this report, the anticipated number of units upon full build-out is 146, which was used to determine the number of new trips generated. TABLE I summarizes the number and directional split of entering and exiting trips for peak periods for the proposed subdivision.

···		TAB	LE 1		
	Tì	RIP GENERAT	TION SUMMA	RY	
FOX ROAD ST	JBDIVISION - 140	LOTS	<u> </u>		
	Total New Trips	% Entering	% Exiting	Number Entering	Number Exiting
Weekday	1468	50%	50%	734	734
A.M. Peak	112	25%	75%	28	84
P.M. Peak	151	64%	36%	97	54

ROAD $\stackrel{\times}{\circ}$ LŁ. FOX ROAD SUBDIVISION 51 (246) 255 (103)

> VOLUME LEGEND AM (PM)



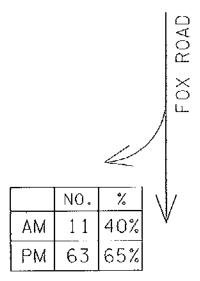
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FIGURE 4
PEAK HOUR TRAFFIC VOLUMES
BACKGROUND TRAFFIC - YEAR 2008

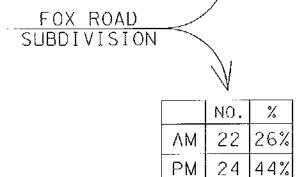
Trip Distribution

FIGURE 5 provides a summary of the trip generation patterns developed for the proposed subdivision intersection with Fox Road, which were based on the existing patterns at the nearby (700 feet south) intersection of Fox Road and the Tan Rara Subdivision. Because these intersections will be in close proximity and along the same roadway, it was assumed that their trip distribution patterns would be very similar. In addition, FIGURE 5 also provides the generated traffic volumes as assigned to the local roadway network in accordance with these patterns. FIGURE 6 shows the combined year 2005 volumes reflecting the existing traffic, the background traffic growth, and the newly generated traffic from Fox Road Subdivision at full build-out. These are the volumes used in the analysis of full build-out conditions.





	NO.	%
AM	62	74%
РМ	30	56%



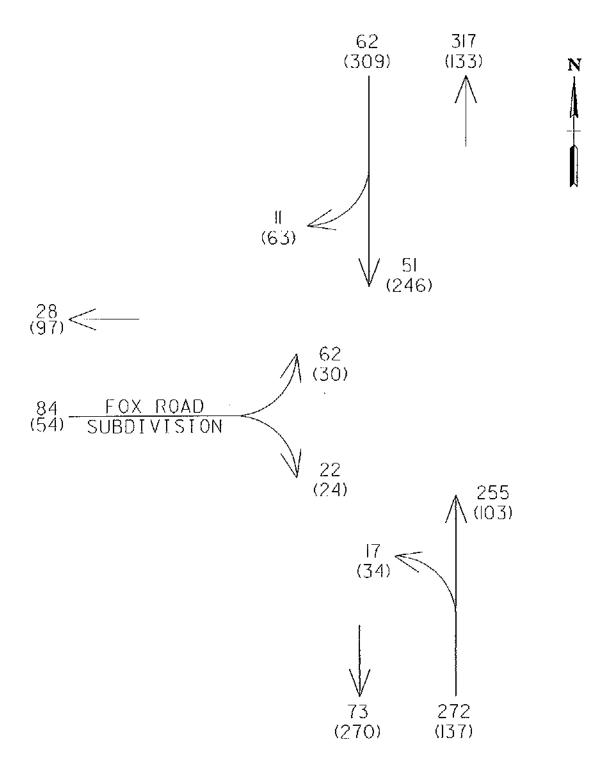
			\bigwedge
	NO.	%	
АМ	17	60%	
РM	34	35%	

	TOTAL	
GENER	ATED	TRIPS
	ENTER	EXIT
ΛM	28	84
PM	97	54

NOTE: ENTER/EXIT DISTRIBUTION PERCENTAGES ASSUMED SAME AS TAN RARA SUBDIVISION.



FIGURE 5
TRIP DISTRIBUTION PATTERNS AND ASSIGNMENT
OF GENERATED TRAFFIC



VOLUME LEGEND AM (PM)

NOTE: VOLUMES SHOWN ARE PROJECTED FULL BUILD-OUT VOLUMES FOR YEAR 2008.



FIGURE 6 COMBINED VOLUMES FOR ANALYSIS

Proposed Level-of-Service

Unsignalized intersection capacity analyses were conducted utilizing the combined traffic volumes of FIGURE 6, at the proposed intersection of Fox Road and the Fox Road Subdivision access roadway. The results indicate that all traffic movements are expected to operate at level-of-service "A" or "B" during both peak hours. These results are summarized on the "Two-Way Stop Control Summary" printouts contained in the APPENDIX.

Intersection Sight Distance and Other Issues

A field review was conducted to identify any sight distance problems, geometric problems or other issues of concern that could impact the proposed subdivision. The results of this review are summarized below:

1) Sight Distance for Vehicles Exiting the Proposed Subdivision:

Looking left (north) from a STOP position at Fox Road, on the proposed subdivision roadway, the sight distance is approximately 500 feet. Looking right (south) from the same STOP position, the sight distance is approximately 350 feet.

The posted speed limit on Fox Road is 30 mph. However, when establishing the required sight distance, it is good practice to consider higher speeds where appropriate. Therefore, in consideration of observed approach speeds in excess of 30 mph, it is recommended that sight distance be provided for a minimum of 40 mph (400 feet).

Based on the above information, there is an existing sight distance problem looking south. However, the source of the restricted sight distance is trees and brush located along the frontage of the subdivision property. The cutting down and trimming of these trees and brush will provide the required sight distance, and in fact will likely provide in excess of 500 feet of total sight distance looking south.

3) Auxiliary Lanes for Proposed Subdivision Intersection:

Left and right turn lane warrant analyses were conducted for the proposed subdivision intersection. These analyses employed Tables 5A and 5B from turn lane warrants developed by Harmelink. The results were that the anticipated traffic volumes are not sufficient to satisfy the minimum warrants. Therefore, auxiliary turn lanes are not warranted. Copies of Tables 5A and 5B are located in the APPENDIX for review.

CONCLUSIONS AND RECOMMENDATIONS

It is the primary conclusion of this study that no significant traffic volume related impacts will result from the development of the Fox Road Subdivision. In fact, capacity analyses of proposed side street (2-way) stop traffic control, indicates that very good conditions (LOS "B" or better) can be expected during all time periods. In addition, analyses of the need for auxiliary traffic lanes such as left and right turning lanes, indicates that no such lanes will be warranted under the anticipated traffic conditions.

Intersection turning sight distance is the only issue of significant concern that was identified in this study. Specifically, the sight distance looking south from the proposed subdivision access roadway intersection at Fox Road was found to be somewhat deficient. However, it was determined that the cutting down and trimming of some trees and brush would provide more than the required sight distance. Therefore, such action to provide the required sight distance is recommended prior to opening the subdivision roadways to traffic.

APPENDIX

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05:00 PM	17	43	0	0	0	0	0	0	0	16	6	0	8	0	7	0	97
05:15 PM	12	40	0	0	0	0	0	0	0	13	7	0	9	0	6	0	87
05:30 PM	9	43	0	0	G	0	0	0	0	12	9	0	6	0	13	0	92
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TRAFFIC VOLUME ADJUSTMENT FACTORS TO BE USED WITH "TRAFFIC SIGNAL WARRANT ANALYSIS -- VOLUME WARRANTS" Propered and Distributed by the Tennessee Transportation Assistance Program

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Tuesday	1.00	660	0.95	0.94	0.93	0.91	0.91	0.92	880	0.94	960	760
Wednesday	101	66.0	0.95	0.92	0.92	0.90	0.91	0.92	0.93	0.94	98.0	150
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Monday	121	1.17	1.13	1.10	£0.1	1.06	1.07	1.03	1.10	1.14	3.14	02.5
Tuesday	1,17	1.16	1.1	1.10	1.09	106	1.06	1.07	1.8	1,10	1.12	1.13
Wednesday	1.18	(.16	1,11	1.07	1.07	3.6	1.06	1.07	8	1.10	111	91.0
Thursday	1.16	£.13	60,1	1.05	<u>8</u> .	8	1.04	1.05	50.1	1.07	60 37	8
Friday	90'1	1.04	1.02	0.59	76.0	56.0	0.96	0.97	760	00,1	1.07	8

Notes: 1. Traffic Signal Warrant Analysis — Volume Warrants" is a Lotus. 1—2—3th tamplate distributed by the Tunnesse Transportation Assistance Program (TTAP).
2. Factors should be applied to State highway and major street volumes only. They should be applied to volumes on diverways (shopping centers, etc.) or annor street.
3. Counts made on holidays should not be used as a basis for estimating average day, average weekday or average Friday volumes.

Source: TABLE A — Termessee Department of Transportation (based on 1988 through 1992 data)
TABLEs B & C — Developed by T. Dozog Sulfvan, P.E. based on TABLE A data

	TWO	WAY STOP	CONTRO	OL SUI	MARY			
General Informatio	on		Site In	iforma	tion			
Analyst Agency/Co. Date Performed Analysis Time Period	ALC Cannon & 6/6/03 AM Peak	Cannon, Inc.	Intersed Jurisdic Analysis	tion		Fox Roa Rd.Subo City of K 2008	livision	
Project Description F	ox Road Subd	ivision Traffic In	pact Study	/				
East/West Street: Fox	Road Subdivis	sion	North/S	outh Stre	eet: <i>Fox</i> .	Road		
Intersection Orientation	: North-Soutf	7	Study P	eriod (h	rs): <i>0.25</i>			
Vehicle Volumes a	ınd Adjustm	ents						
Major Street		Northbound				Southbo	und	
Movement	1	2	3		4	5		6
	L.	Т	R		<u>L</u>	Т		R
Volume	17	255	0		0	51		11
Peak-Hour Factor, PHF		0.72	0.72		0.72	0.72		0.72
Hourly Flow Rate, HFR		354	0		0	70	-	15
Percent Heavy Vehicles	3 0			Undivide	-		I	
Median Type RT Channelized		1	0	Onaiviae L	a	1		
Lanes	0	1	0		0	1	<u> </u>	0
Configuration	LT	1	· ·	-	V	1		TR
Upstream Signal	E!	0	-	_		0		115
	<u> </u>		1	- 			<u> </u>	
Minor Street Movement	7	Westbound 8	9		10	Eastbou 11	12	
Movement	 '	T	R		L	'' T		R
Volume	0	0	0		62	1 0		22
volume Peak-Hour Factor, PHF		0.72	0.72		0.72	0.72	_	<u>22</u> 0.72
Hourly Flow Rate, HFR		0.72	0.72	 .	86	0.72		30
Percent Heavy Vehicles		0	0		1	0	_	1
Percent Grade (%)	' ` 	0		_	<u> </u>	0		-
Flared Approach		T N	I			l N		
	1	0				0		
Storage	1	-				 		^
RT Channelized	1 ^	ļ <u>.</u>	0					0
Lanes Configuration	0	0	0	-	0	1		0
Configuration	<u> </u>	<u> </u>			·····	LTR		
Delay, Queue Length,								
Approach	NB	SB		lestboun			Eastbound	
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT			<u></u>		<u> </u>	LTR	
v (vph)	23						116	
C (m) (vph)	1524						611	
v/c	0.02						0.19	
95% queue length	0.05						0.70	
Control Delay	7.4		·····		†		12.3	
LOS	A						B	1
Approach Delay					<u> </u>		12.3	<u>. </u>
- ' ' ' 	-							
Approach LOS	<u> </u>	vright © 2000 Univers				<u> </u>	В	Version 4

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Version 4.1c

	TWO-	WAY STOP	CONTRO	DL SUI	WMARY			
General Informatio	n		Site Ir	ıforma	tion			
Analyst Agency/Co. Date Performed Analysis Time Period	6/6/03	Cannon, Inc. Hour	Intersed Jurisdic Analysi	tion		Fox Roa Rd.Subo City of K 2008	livision	
Project Description F	ox Road Subdi	vision Traffic Im	pact Study	<i>y</i>				
East/West Street: Fox					eet: Fox	Road		
Intersection Orientation:	: North-South	}	Study F	eriod (h	rs): 0.25			
Vehicle Volumes a	nd Adjustm	ents						
Major Street		Northbound				Southbo	und	
Movement	1	2	3		4	5		6
<u></u>	L	T	R		L	T		R
Volume	34	103	0		0	246		63
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	0.91 37	0.91	0.91 0		0.91 0	0.91 270		0.91 69
Percent Heavy Vehicles	1	110		- -	0	210	-+	09
Hedian Type	·	1		Undivide	-			
RT Channelized	+	T	0	Ondivide	, u		-	0
Lanes	0	1	Ö		0	1		0
Configuration	LT	<u> </u>	·			,		ŤR
Upstream Signal	 	0		-		0	-	
Minor Street	1	Westbound		- 1		Eastbou	nd	
Movement	7	8	9		10	11	1	12
	L	Т	R		L.	T		R
Volume	0	0	0	· · · · · · · · · · · · · · · · · · ·	30	0	- -	24
Peak-Hour Factor, PHF		0.91	0.91		0.91	0.91	<u> </u>	0.91
Hourly Flow Rate, HFR	0	0	0		32	0		26
Percent Heavy Vehicles	0	0	0		1	0		1
Percent Grade (%)		0				0		
Flared Approach	1	N				N		
Storage	1	0				0		
RT Channelized	İ		0					0
Lanes	0	0	0		0	1		0
Configuration	1					LTR		
Delay, Queue Length,	and Level of S	Service						
Approach	NB	SB	V	/estboun	ıd	l e	astbound	
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	····	•		 		LTR	
v (vph)	37				 		58	
	1231	+			+	1	602	-
C (m) (vph)	0.03		<u></u>	***			0.10	
V/C				·-·	 			
95% queue length	0.09	<u></u>			1	ļi	0.32	-
Control Delay	8.0						11.6	
LOS	Α						₿	ŀ
Approach Delay						<u> </u>	11.6	
Approach LOS							В	

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TABLE 5A

LEFT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

(If the left-turn volume exceeds the table value a left -turn lane is needed)

OPPOSING	THROUGH	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *								
VOLUME	100 - 149	150 - 199	200 - 249	250 - 299	300 - 349	350 - 399				
100 - 149	250	180	140	** (110)*A(¥ 80	70				
150 - 199	200	140	105	90 LT)	√1, 70	60				
200 - 249	160	115	85	75	65	55				
250 - 299	130	100	75	65	60	50				
300 - 349 350 - 399	** (10) * PM * 100 LT Vol. 2 3+	90 80	70 65	60 55	55 50	45 40				
400 - 449	90	70	60	50	45	35				
450 - 499	80	65	55	45	40	30				
500 - 549	70	60	45	35	35	25				
550 - 599	65	55	40	35	30	25				
600 - 649	60	45	35	30	25	25				
650 - 699	55	35	35	30	25	20				
700 - 749	50	35	30	25	20	20				
750 or More	45	35	25	25	20	20				

OPPOSING	THROU	THROUGH VOLUME PLUS RIGHT-TURN VOLUME *							
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 599	=/ >600			
100 - 149	70	60	50	45	40	35			
150 - 199	60	55	45	40	35	30			
200 - 249	55	50	40	35	30	30			
250 - 299	50	45	35	30	30	30			
300 - 349	45	40	35	30	25	25			
350 - 399	40	35	30	25	25	20			
400 - 449	35	30	30	25	20	20			
450 - 499	30	25	25	20	20	20			
500 - 549	25	25	20	20	20	15			
550 - 599	25	20	20	20	20	15			
600 - 649	25	20	20	20	20	15			
650 - 699	20	20	20	20	20	15			
700 - 749	20	20	20	15	15	15			
750 or More	20	20	20	15	15	15			

^{*} Or through volume only if a right-turn lane exists

TABLE 5B

RIGHT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH

RIGHT-TURN	THROU	GH VOLU	ME PLUS LEF	T-TURN	VOLUME	*
VOLUME	<100	100 - 199	200 - 249	250 - 299	300 - 349	350 - 399
Fewer Than 25 25 - 49 50 - 99	*AM Peak*		*PM Peak*			
100 - 149 150 - 199						ļ
200 - 249 250 - 299					Yes	Yes Yes
300 - 349 350 - 399			Yes	Yes Yes	Yes Yes	Yes Yes
400 - 449 450 - 499		Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
500 - 549 550 - 599	Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes
600 or More	Yes	Yes	Yes	Yes	Yes	Yes

RIGHT-TURN	THR	THROUGH VOLUME PLUS LEFT-TURN VOLUME *							
VOLUME	350 - 399	400 - 449	450 - 499	500 - 549	550 - 600	+/> 600			
Fewer Than 25 25 - 49 50 - 99				Yes	Yes Yes	Yes Yes			
100 - 149 150 - 199		Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
200 - 249 250 - 299	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
300 - 349 350 - 399	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
400 - 449 450 - 499	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
500 - 549 550 - 599	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes	Yes Yes			
600 or More	Yes	Yes	Yes	Yes	Yes	Yes			

^{*} Or through volume only if a left-turn lane exists.