# TWIN OAK LANDING SUBDIVISION <br> Traffic Impact Study <br> Twin Oak Lane <br> Knoxville, TN 

## A Traffic Impact Study for the Proposed Twin Oak Landing Subdivision

Submitted to
Knoxville - Knox County Planning Commission

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Submitted By:


# Twin Oak Landing Subdivision <br> Traffic Impact Study August 10, 2020 

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## Executive Summary

The Owner, Gary Hibben, proposes a residential development (i.e. Twin Oak Landing Subdivision) located on Tazewell Pike (SR 131) at Twin Oak Lane in Corryton, TN, Knox County. The development will consist of 107 single family homes. Construction is proposed to take place this year and this study assumes full build out for the development will occur in 2023.

The main driveway connection will be located at the existing intersection of Twin Oak Lane at Tazewell Pike (SR 131). The proposed driveway connection is a single lane for exiting traffic and a proposed northbound left turn lane on Tazewell Pike (SR 131). The concept plan shows a sidewalk along the internal subdivision roads, Twin Oak Lane and on the west side of Tazewell Pike (SR 131) for the length of the property line.

The following items are recommended in order to maintain or provide an acceptable level-of-service for each of the studied intersections.

## Tazewell Pike (SR 131) @ Twin Oak Lane

After the full buildout of the Twin Oak Landing Subdivision the eastbound approach will operate at a LOS B during both the AM and PM peak hours and the northbound left turn will operate at a LOS A during both the AM and PM peak hours.

A northbound left turn lane is warranted at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane after the completion of the subdivision. FMA recommends a 75 foot storage length, a 165 foot bay taper and a minimum 11 foot lane width. The detailed design of the turn lane should be coordinated with the Tennessee Department of Transportation (TDOT) and Knox County Engineering \& Public Works.

## Twin Oak Lane

The existing width of Twin Oak Lane between the intersection with Tazewell Pike (SR 131) and the proposed project entrance varies from approximately 11.5 feet to 14 feet. The minimum pavement width for a local street is 26 feet per the "Minimum Subdivision Regulations" for Knoxville and Knox County. FMA recommends that the pavement width on Twin Oak Lane be widened to a maximum of 26 feet. The widening of Twin Oak Lane should be coordinated with Knox County Engineering \& Public Works.

## 1 Introduction

### 1.1 Project Description

This report provides a summary of a traffic impact study that was performed for the proposed Twin Oak Landing Subdivision on Twin Oak Lane. The project site is located west of Tazewell Pike (SR 131) on Twin Oak Lane in Corryton, TN in Knox County. The location of the site is shown in Figure 1.

The proposed Twin Oak Landing Subdivision will be within the Parent Responsibility Zone (PRZ) of Gibbs Elementary School, Gibbs Middle School and Gibbs High School. The PRZ is defined as those who live within one (1) mile from an elementary school or one and one-half ( $11 / 2$ ) miles from a middle school or high school by the shortest route, and are not eligible for transportation service.

The proposed Twin Oak Landing Subdivision will consist of 107 single family lots with the main driveway connection located at the existing intersection of Twin Oak Lane at Tazewell Pike (SR 131). Full buildout is expected to occur within three years, or by the year 2023.

The proposed driveway connection is a single lane for exiting traffic and a proposed northbound left turn lane on Tazewell Pike (SR 131). The concept plan shows a sidewalk along the internal subdivision roads, Twin Oak Lane and on the west side of Tazewell Pike (SR 131) for the length of the property line. The proposed site layout is shown in Figure 2.

The purpose of this study is to evaluate the impacts to the traffic conditions caused by the development of the proposed Twin Oak Landing Subdivision.


Figure 1: Location Map


Figure 2: Site Plan

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### 1.2 Existing Site Conditions

Twin Oak Lane is a two-lane road with an approximate width between 11.5 feet to 14 feet. Knoxville-Knox County Planning does not classify Twin Oak Lane; therefore it is considered a local street. There is no posted speed limit on Twin Oak Lane. Twin Oak Lane is a dead end street with an approximate length of 1,575 feet.

Tazewell Pike (SR 131) is a two-lane road at the intersection with Twin Oak Lane. Knoxville-Knox County Planning classifies Tazewell Pike (SR 131) as a Minor Arterial per the Major Road Plan with an 88 foot right-of-way. The posted speed limit on Tazewell Pike (SR 131) is 45 mph .

Along Tazewell Pike (SR 131) there are existing sidewalks on the west side of the road in front of Gibbs Elementary School that begin approximately 1,200 feet south of the intersection with Twin Oak Lane and continue south through the intersection at E Emory Road. There are no existing sidewalks on Twin Oak Lane.

An aerial photo of the existing intersection is included in Attachment 1.

## 2 Existing Traffic Volumes

Due to the altered traffic patterns from COVID-19 FMA did not collect any new turning movement counts for the Twin Oak Landing Subdivision traffic impact study. A worksheet was included in Attachment 2 outlining the step-by-step calculations and assumptions that were used in determining the traffic counts at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane.

FMA conducted a six-hour turning movement count at the intersection of Tazewell Pike (SR 131) and Twin Oak Lane on Wednesday March 22, 2017. The AM peak hour occurred between 7:00 am and 8:00 am and the PM peak hour occurred between 5:00 pm and 6:00 pm. The count data collected is included in Attachment 2.

In order to calculate existing traffic conditions entering/exiting Twin Oak Lane FMA estimated a growth rate from the 2017 count to the projected existing 2020 traffic conditions. The growth rate was determined by analyzing nearby traffic counts provided by the Tennessee Department of Transportation (TDOT) in the vicinity of the proposed development. For the purpose of this study, an annual growth rate of $2 \%$ was assumed. The ADT trend line growth charts are included in Attachment 3.

In addition to the 2017 turning movement count a more recent traffic count was conducted on February 4, 2020 at the intersection of Tazewell Pike (SR 131) and the Gibbs Elementary School driveway. FMA used the more recent count data to determine the northbound and southbound thru movement at the existing intersection of Tazewell Pike (SR 131) at Twin Oak Lane. The AM peak hour occurred between 7:00 a.m. and 8:00 a.m. with an AM peak hour volume of 135 vehicles traveling northbound and 482 vehicles traveling southbound. The PM peak hour occurred between 5:00 p.m. and 6:00 p.m. with a PM peak hour volume of 440 vehicles travelling northbound and 216 vehicles traveling southbound. The Gibbs Elementary turning movement count is included in Attachment 2.

Figure 3 shows the existing volumes for the year 2020 at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane.


LEGEND:
« 5 (16) TURNING MOVEMENT VOLUME AM (PM)

Figure 3: 2020 Existing Peak Hour Traffic

## 3 Background Growth

The Tennessee Department of Transportation (TDOT) maintains a count station in the vicinity of the proposed development. TDOT count station ID: 000006 is located on Tazewell Pike (SR 131) north of Harbison Cross Roads and south of Twin Oak Lane. The annual growth rate for this station is approximately $0.64 \%$ and the 2018 ADT was 8,153 vehicles per day.

For the purpose of this study, an annual growth rate of $2.0 \%$ was assumed for the intersection of Tazewell Pike (SR 131) at Twin Oak Lane until full occupancy is reached in 2023. Attachment 3 shows the trend line growth charts for the TDOT count station.

Figure 4 demonstrates the projected background peak hour volumes at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane after applying the background growth rate to the existing conditions.


LEGEND:
« 5 (16) TURNING MOVEMENT VOLUME AM (PM)

Figure 4: 2023 Background Peak Hour Traffic

## 4 Trip Generation and Trip Distribution

The Twin Oak Landing Subdivision proposes 107 single family lots. Single- Family Detached Housing or Land Use 210 was used to calculate site trips for the subdivision using the fitted curve equations from the Trip Generation, $10^{\text {th }}$ Edition, published by the Institute of Transportation Engineers. The land use worksheets are included in Attachment 4.

The total trips generated by the full buildout of the Twin Oak Landing Subdivision was estimated to be 1,107 daily trips. The estimated trips are 81 trips during the AM peak hour and 108 trips during the PM peak hour. A trip generation summary is shown in Table 4-1.

Table 4-1
Twin Oak Landing Subdivision
Trip Generation Summary

| Land Use | Density | Daily | AM Peak Hour |  | PM Peak Hour <br> Enter |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  |  | Trips | Exit | Enter | Exit |  |
| Single-Family <br> Detached Housing <br> (Land Use 210) | 107 lots | 1107 | 20 | 61 | 68 | 40 |

The directional distribution of the traffic generated by the proposed Twin Oak Landing Subdivision was determined using the traffic data collected for the existing conditions. The typical weekday traffic pattern is for traffic to flow heavier in one direction in the morning peak period and then for the traffic to be heavier in the opposite direction during the evening peak period. Tazewell Pike (SR 131) at the intersection with Twin Oak Lane has an existing trip distribution of $20 \%$ northbound and $80 \%$ southbound during the AM peak hour and $65 \%$ northbound and $35 \%$ southbound during the PM peak hour.

Because of the low number of existing trips on Twin Oak Lane a trip distribution at the intersection of Twin Oak Lane and Tazewell Pike (SR 131) was assumed for both the AM and PM peak hour of $85 \%$ entering northbound traffic and $15 \%$ entering southbound traffic and $85 \%$ exiting Southbound traffic and $15 \%$ exiting northbound traffic. The trip distribution for the Twin Oak Landing Subdivision is shown in Figure 5.

Figure 6 shows the peak hour site trips generated by the Twin Oak Landing Subdivision and Figure 7 shows the projected full buildout peak hour traffic after the completion of the Twin Oak Landing Subdivision.


LEGEND:
$\longleftarrow 50 \%$ (50\%) TRIP DISTRIBUTION ENTERING (EXITING)

Figure 5: Peak Hour Trip Distribution


LEGEND:
« 5 (16) TURNING MOVEMENT VOLUME AM (PM)

Figure 6: Twin Oak Landing Subdivision Peak Hour Site Trips


LEGEND:
« 5 (16) TURNING MOVEMENT VOLUME AM (PM)

Figure 7: 2023 Full Buildout Peak Hour Traffic

## 5 Projected Capacity and Level of Service

The existing intersection of Tazewell Pike (SR 131) at Twin Oak Lane is a two-way stop controlled intersection.

Unsignalized intersection capacity analyses were performed for the AM and PM peak hours to evaluate the existing, background and full buildout traffic conditions at the intersection of Tazewell Pike (SR 131) and Twin Oak Lane. The existing, background and full buildout HCS7 worksheets are included in attachments 5, 6 and 7.

The results from the analyses are expressed with a term "level of service" (LOS), which is based on the amount of delay experienced at the intersection. The LOS index ranges from LOS A, indicating excellent traffic conditions with minimal delay, to LOS F indicating very congested conditions with excessive delay. LOS D generally is considered the minimum acceptable condition in urban areas. Table 51 shows the results of the capacity analyses.

Table 5-1
Intersection Analysis
Level of Service (LOS Summary)

| Delay (sec)/LOS |  |  |
| :---: | :---: | :---: |
| Tazewell Pike (SR 131) @ Twin Oak Lane (Existing 2020) |  |  |
| AM Peak | EB Approach NB Left Turn | $\begin{aligned} & 12.6 / B \\ & 8.5 / \mathrm{B} \end{aligned}$ |
| PM Peak | EB Approach NB Left Turn | $\begin{aligned} & 9.5 / A \\ & 7.7 / A \end{aligned}$ |
| Tazewell Pike (SR 131) @ Twin Oak Lane (Background 2023) |  |  |
| AM Peak | EB Approach NB Left Turn | $\begin{aligned} & 13.0 / B \\ & 8.6 / \mathrm{A} \end{aligned}$ |
| PM Peak | EB Approach NB Left Turn | $\begin{aligned} & 9.6 / \mathrm{A} \\ & 7.8 / \mathrm{A} \end{aligned}$ |
| Tazewell Pike (SR 131) @ Twin Oak Lane (Full Buildout 2023) |  |  |
| AM Peak | EB Approach NB Left Turn | $\begin{aligned} & 13.5 / B \\ & 8.6 / \mathrm{A} \end{aligned}$ |
| PM Peak | EB Approach NB Left Turn | $\begin{aligned} & 11.3 / B \\ & 7.9 / \mathrm{A} \end{aligned}$ |

## 6 Turn Lane Warrant Analysis

The intersection of Tazewell Pike (SR 131) and Twin Oak Lane was evaluated to determine if a northbound left turn lane or a southbound right turn on Tazewell Pike (SR 131) is warranted. The Knox County Department of Engineering and Public Works handbook, "Access Control and Driveway Design Policy," was used to analyze the information.

At the intersection of Tazewell Pike (SR 131) and Twin Oak Lane a southbound right turn lane on Tazewell Pike (SR 131) is not warranted and a northbound left turn lane on Tazewell Pike (SR 131) is warranted during the PM peak after the full buildout of the Twin Oak Landing Subdivision. The turn lane warrant worksheets and analysis are included in Attachment 7.

## 7 Conclusions and Recommendations

### 7.1 Tazewell Pike (SR 131) @ Twin Oak Lane

The existing, background and full buildout conditions at the two-way stop controlled intersection of Tazewell Pike (SR 131) at Twin Oak Lane were analyzed using the Highway Capacity Software (HCS7).

Both the existing and background traffic conditions for the eastbound approach operate at a LOS B during the AM peak hour and a LOS A during the PM Peak hour and the northbound left turn operates at a LOS A during both the AM and PM peak hours.

After the full buildout of the Twin Oak Landing Subdivision the eastbound approach will operate at a LOS B during both the AM and PM peak hours and the northbound left turn will operate at a LOS A during both the AM and PM peak hours.

A northbound left turn lane is warranted at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane after the completion of the subdivision. FMA recommends a 75 foot storage length, a 165 foot bay taper and a minimum 11 foot lane width. The detailed design of the turn lane should be coordinated with the Tennessee Department of Transportation (TDOT) and Knox County Engineering \& Public Works.

A southbound right turn lane is not warranted at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane after the completion of the subdivision.

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### 7.2 Twin Oak Lane

The existing width of Twin Oak Lane between the intersection with Tazewell Pike (SR 131) and the proposed project entrance varies from approximately 11.5 feet to 14 feet. The minimum pavement width for a local street is 26 feet per the "Minimum Subdivision Regulations" for Knoxville and Knox County. FMA recommends that the pavement width on Twin Oak Lane be widened to a maximum of 26 feet. The widening of Twin Oak Lane should be coordinated with Knox County Engineering \& Public Works.

Attachment 1
Aerial Photo


Attachment 2
Traffic Counts

## Project: Twin Oak Landing Subdivision

Intersection: Tazewell Pike at Twin Oak Lane
Date Conducted: 03/22/2017

|  | Tazewell Pike Northbound |  |  | Tazewell Pike Southbound |  |  | Twin Oak Lane Eastbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start | Left | Thru | Total | Thru | Right | Total | Left | Right | Total | Int. Total |
| 7:00 AM |  | 26 | 26 | 118 | 0 | 118 |  | 0 | 0 | 144 |
| 7:15 AM |  | 27 | 27 | 119 | 0 | 119 |  | 1 | 1 | 147 |
| 7:30 AM |  | 62 | 62 | 151 | 0 | 151 |  | 0 | 1 | 214 |
| 7:45 AM |  | 58 | 58 | 196 | 0 | 196 |  | 0 | 0 | 254 |
| Total |  | 173 | 173 | 584 | 0 | 584 |  | 1 | 2 | 759 |


| 8:00 AM | 0 | 45 | 45 | 88 | 0 | 88 | 0 | 0 | 0 | 133 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 8:15 AM | 0 | 46 | 46 | 97 | 0 | 97 | 0 | 0 | 0 | 143 |
| 8:30 AM | 0 | 28 | 28 | 81 | 0 | 81 | 0 | 0 | 0 | 109 |
| 8:45 AM | 0 | 30 | 30 | 64 | 0 | 64 | 0 | 1 | 1 | 95 |
| Total | 0 | 149 | 149 | 330 | 0 | 330 | 0 | 1 | 1 | 480 |


| 2:00 PM | 0 | 58 | 58 | 52 | 0 | 52 | 0 | 0 | 0 | 110 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 2:15 PM | 0 | 54 | 54 | 60 | 0 | 60 | 0 | 1 | 1 | 115 |
| 2:30 PM | 1 | 50 | 51 | 65 | 0 | 65 | 0 | 0 | 0 | 116 |
| 2:45 PM | 0 | 79 | 79 | 45 | 0 | 45 | 0 | 0 | 0 | 124 |
| Total | 1 | 241 | 242 | 222 | 0 | 222 | 0 | 1 | 1 | 465 |


| 3:00 PM | 1 | 88 | 89 | 44 | 0 | 44 | 0 | 0 | 0 | 133 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| 3:15 PM | 0 | 89 | 89 | 48 | 0 | 48 | 0 | 0 | 0 | 137 |
| 3:30 PM | 0 | 165 | 165 | 50 | 0 | 50 | 0 | 0 | 0 | 215 |
| 3:45 PM | 0 | 99 | 99 | 53 | 0 | 53 | 0 | 0 | 0 | 152 |
| Total | 1 | 441 | 442 | 195 | 0 | 195 | 0 | 0 | 0 | 637 |


| 4:00 PM | 1 | 103 | 104 | 52 | 0 | 52 | 0 | 1 | 1 | 157 |
| :---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $4: 15 \mathrm{PM}$ | 0 | 85 | 85 | 50 | 0 | 50 | 0 | 0 | 0 | 135 |
| 4:30 PM | 1 | 84 | 85 | 52 | 0 | 52 | 0 | 0 | 0 | 137 |
| 4:45 PM | 1 | 116 | 117 | 56 | 0 | 56 | 0 | 1 | 1 | 174 |
| Total | 3 | 388 | 391 | 210 | 0 | 210 | 0 | 2 | 2 | 603 |


| 5:00 PM | 0 | 110 | 110 | 40 | 0 | 40 | 0 | 1 | 1 | 151 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| 5:15 PM | 2 | 91 | 93 | 54 | 0 | 54 | 0 | 0 | 0 | 147 |
| 5:30 PM | 1 | 119 | 120 | 67 | 0 | 67 | 0 | 0 | 0 | 187 |
| 5:45 PM | 3 | 114 | 117 | 65 | 0 | 65 | 0 | 0 | 0 | 182 |
| Total | 6 | 434 | 440 | 226 | 0 | 226 | 0 | 1 | 1 | 667 |


| Grand Total | 11 | 1826 | 1837 | 1767 | 0 | 1767 | 1 | 6 | 7 | 3611 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Approach \% | 0.6 | 99.4 |  | 100.0 | 0.0 |  | 14.3 | 85.7 |  |  |
| Total \% | 0.3 | 50.6 | 50.9 | 48.9 | 0.0 | 48.9 | 0.0 | 0.2 | 0.2 |  |

Project: Twin Oak Landing Subdivision
Intersection: Tazewell Pike at Twin Oak Lane
Date Conducted: 3/22/2017

| AM Peak Hour | $7: 00$ AM - 8:00 AM | 759 |
| :--- | :--- | :--- |
| PM Peak Hour | 5:00 PM - 6:00 PM | 667 |


|  | Tazewell Pike Northbound |  |  | Tazewell Pike Southbound |  |  | Twin Oak Lane Eastbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start | Left | Thru | Total | Thru | Right | Total | Left | Right | Total | Int. Total |
| Peak Hour Analysis from 7:00 AM to 9:00 AM |  |  |  |  |  |  |  |  |  |  |
| AM Peak Hour begins at 7:30 AM |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 0 | 26 | 26 | 118 | 0 | 118 | 0 | 0 | 0 | 144 |
| 7:15 AM | 0 | 27 | 27 | 119 | 0 | 119 | 0 | 1 | 1 | 147 |
| 7:30 AM | 0 | 62 | 62 | 151 | 0 | 151 | 1 | 0 | 1 | 214 |
| 7:45 AM | 0 | 58 | 58 | 196 | 0 | 196 | 0 | 0 | 0 | 254 |
| Total Volume | 0 | 173 | 173 | 584 | 0 | 584 | 1 | 1 | 2 | 759 |
| Future (2\% over 3 yrs) | 0 | 184 |  | 620 | 0 |  | 1 | 1 |  | 805 |
| Future (2\% over 6 yrs) | 0 | 195 |  | 658 | 0 |  | 1 | 1 |  | 855 |
| PHF | - | 0.70 |  | 0.74 | - |  | 0.25 | 0.25 |  | 0.75 |
| Peak Hour Analysis from 3:00 PM to 6:00 PM |  |  |  |  |  |  |  |  |  |  |
| PM Peak Hour begins at 5:00 PM |  |  |  |  |  |  |  |  |  |  |
| 5:00 PM | 0 | 110 | 110 | 40 | 0 | 40 | 0 | 1 | 1 | 151 |
| 5:15 PM | 2 | 91 | 93 | 54 | 0 | 54 | 0 | 0 | 0 | 147 |
| 5:30 PM | 1 | 119 | 120 | 67 | 0 | 67 | 0 | 0 | 0 | 187 |
| 5:45 PM | 3 | 114 | 117 | 65 | 0 | 65 | 0 | 0 | 0 | 182 |
| Total Volume | 6 | 434 | 440 | 226 | 0 | 226 | 0 | 1 | 1 | 667 |
| Future (2\% over 3 yrs ) | 6 | 461 |  | 240 | 0 |  | 0 | 1 |  | 708 |
| Future (2\% over 6 yrs) | 7 | 489 |  | 255 | 0 |  | 0 | 1 |  | 751 |
| PHF | 0.50 | 0.91 |  | 0.84 | - |  | - | 0.25 |  | 0.89 |


|  | SR 131/Tazewell Pike Northbound |  |  |  |  |  | SR 131/Tazewell Pike Southbound |  |  |  |  |  | Gibbs Elementary School Dwy Eastbound |  |  |  |  |  | Gibbs Elementary School Dwy Westbound |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Rgt | Uturn | Peds | App. Total | Left | Thru | Rgt | Uturn | Peds | App. Total | Left | Thru | Rgt | Uturn | Peds | App. Total | Left | Thru | Rgt | Uturn | Peds | App. Total | Int. Total |
| 7:00 AM | 75 | 7 | 0 | 0 | 0 | 82 | 0 | 93 | 24 | 0 | 0 | 117 | 6 | 0 | 84 | 0 | 0 | 90 | 0 | 0 | 0 | 0 | 0 | 0 | 289 |
| 7:15 AM | 103 | 15 | 0 | 0 | 0 | 118 | 0 | 85 | 63 | 0 | 0 | 148 | 6 | 0 | 120 | 0 | 0 | 126 | 0 | 0 | 0 | 0 | 0 | 0 | 392 |
| 7:30 AM | 106 | 23 | 0 | 0 | 0 | 129 | 0 | 70 | 70 | 0 | 0 | 140 | 29 | 0 | 124 | 1 | 0 | 154 | 0 | 0 | 0 | 0 | 0 | 0 | 423 |
| 7:45 AM | 25 | 39 | 0 | 0 | 0 | 64 | 0 | 69 | 11 | 0 | 0 | 80 | 15 | 0 | 57 | 0 | 0 | 72 | 0 | 0 | 0 | 0 | 0 | 0 | 216 |
| Total | 309 | 84 | 0 | 0 | 0 | 393 | 0 | 317 | 168 | 0 | 0 | 485 | 56 | 0 | 385 | 1 | 0 | 442 | 0 | 0 | 0 | 0 | 0 | 0 | 1320 |
| 8:00 AM | 16 | 27 | 0 | 0 | 0 | 43 | 0 | 83 | 14 | 0 | 0 | 97 | 6 | 0 | 41 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 187 |
| 8:15 AM | 9 | 37 | 0 | 0 | 0 | 46 | 0 | 84 | 8 | 0 | 0 | 92 | 6 | 0 | 10 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 154 |
| 8:30 AM | 2 | 28 | 0 | 0 | 0 | 30 | 0 | 60 | 2 | 0 | 0 | 62 | 2 | 0 | 2 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 96 |
| 8:45 AM | 4 | 14 | 0 | 0 | 0 | 18 | 0 | 61 | 0 | 0 | 0 | 61 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 80 |
| Total | 31 | 106 | 0 | 0 | 0 | 137 | 0 | 288 | 24 | 0 | 0 | 312 | 14 | 0 | 54 | 0 | 0 | 68 | 0 | 0 | 0 | 0 | 0 | 0 | 517 |
| ***BREAK*** |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:00 PM | 40 | 52 | 0 | 0 | 0 | 92 | 0 | 42 | 5 | 0 | 0 | 47 | 2 | 0 | 12 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 153 |
| 2:15 PM | 33 | 49 | 0 | 0 | 0 | 82 | 0 | 51 | 5 | 0 | 0 | 56 | 1 | 0 | 6 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 145 |
| 2:30 PM | 19 | 54 | 0 | 0 | 0 | 73 | 0 | 38 | 4 | 0 | 0 | 42 | 2 | 0 | 12 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 129 |
| 2:45 PM | 42 | 59 | 0 | 0 | 0 | 101 | 0 | 43 | 21 | 0 | 0 | 64 | 10 | 0 | 87 | 1 | 0 | 98 | 0 | 0 | 0 | 0 | 0 | 0 | 263 |
| Total | 134 | 214 | 0 | 0 | 0 | 348 | 0 | 174 | 35 | 0 | 0 | 209 | 15 | 0 | 117 | 1 | 0 | 133 | 0 | 0 | 0 | 0 | 0 | 0 | 690 |
| 3:00 PM | 27 | 66 | 0 | 0 | 0 | 93 | 0 | 39 | 13 | 0 | 0 | 52 | 16 | 0 | 84 | 0 | 0 | 100 | 0 | 0 | 0 | 0 | 0 | 0 | 245 |
| 3:15 PM | 10 | 54 | 0 | 1 | 0 | 65 | 0 | 44 | 1 | 0 | 0 | 45 | 5 | 0 | 38 | 0 | 0 | 43 | 0 | 0 | 0 | 0 | 0 | 0 | 153 |
| 3:30 PM | 52 | 142 | 0 | 0 | 0 | 194 | 0 | 49 | 7 | 0 | 0 | 56 | 12 | 0 | 47 | 0 | 1 | 59 | 0 | 0 | 0 | 0 | 0 | 0 | 309 |
| 3:45 PM | 12 | 111 | 0 | 0 | 0 | 123 | 0 | 43 | 2 | 0 | 0 | 45 | 11 | 0 | 28 | 0 | 0 | 39 | 0 | 0 | 0 | 0 | 0 | 0 | 207 |
| Total | 101 | 373 | 0 | 1 | 0 | 475 | 0 | 175 | 23 | 0 | 0 | 198 | 44 | 0 | 197 | 0 | 1 | 241 | 0 | 0 | 0 | 0 | 0 | 0 | 914 |
| 4:00 PM | 12 | 80 | 0 | 0 | 0 | 92 | 0 | 52 | 2 | 0 | 0 | 54 | 1 | 0 | 13 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 160 |
| 4:15 PM | 24 | 93 | 0 | 0 | 0 | 117 | 0 | 51 | 4 | 0 | 0 | 55 | 5 | 0 | 10 | 0 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 187 |
| 4:30 PM | 14 | 87 | 0 | 0 | 0 | 101 | 0 | 47 | 2 | 0 | 0 | 49 | 2 | 0 | 11 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 163 |
| 4:45 PM | 21 | 92 | 0 | 0 | 0 | 113 | 0 | 45 | 7 | 0 | 0 | 52 | 1 | 0 | 11 | 0 | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 177 |
| Total | 71 | 352 | 0 | 0 | 0 | 423 | 0 | 195 | 15 | 0 | 0 | 210 | 9 | 0 | 45 | 0 | 0 | 54 | 0 | 0 | 0 | 0 | 0 | 0 | 687 |
| 5:00 PM | 39 | 83 | 0 | 0 | 0 | 122 | 0 | 42 | 7 | 0 | 0 | 49 | 3 | 0 | 10 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 184 |
| 5:15 PM | 31 | 128 | 0 | 0 | 0 | 159 | 0 | 53 | 7 | 0 | 0 | 60 | 3 | 0 | 19 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 241 |
| 5:30 PM | 42 | 111 | 0 | 0 | 0 | 153 | 0 | 41 | 7 | 0 | 0 | 48 | 5 | 0 | 38 | 0 | 0 | 43 | 0 | 0 | 0 | 0 | 0 | 0 | 244 |
| 5:45 PM | 23 | 101 | 0 | 0 | 0 | 124 | 0 | 51 | 8 | 0 | 0 | 59 | 6 | 0 | 38 | 0 | 0 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 227 |
| Total | 135 | 423 | 0 | 0 | 0 | 558 | 0 | 187 | 29 | 0 | 0 | 216 | 17 | 0 | 105 | 0 | 0 | 122 | 0 | 0 | 0 | 0 | 0 | 0 | 896 |
| Grand Total | 781 | 1552 | 0 | 1 | 0 | 2334 | 0 | 1336 | 294 | 0 | 0 | 1630 | 155 | 0 | 903 | 2 | 1 | 1060 | 0 | 0 | 0 | 0 | 0 | 0 | 5024 |
| Apprch \% | 33.5 | 66.5 | 0.0 | 0.0 | 0.0 |  | 0.0 | 82.0 | 18.0 | 0.0 | 0.0 |  | 14.6 | 0.0 | 85.2 | 0.2 | 0.1 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  |
| Total \% | 15.5 | 30.9 | 0.0 | 0.0 | 0.0 | 46.5 | 0.0 | 26.6 | 5.9 | 0.0 | 0.0 | 32.4 | 3.1 | 0.0 | 18.0 | 0.0 | 0.0 | 21.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Cars, PU, Vans | 732 | 1534 | 0 | 0 |  | 2266 | 0 | 1314 | 292 | 0 |  | 1606 | 150 | 0 | 853 | 2 |  | 1005 | 0 | 0 | 0 | 0 |  | 0 | 4877 |
| \% Cars, PU, Vans | 93.7 | 98.8 | 0.0 | 0.0 |  | 97.1 | 0.0 | 98.4 | 99.3 | 0.0 |  | 98.5 | 96.8 | 0.0 | 94.5 | 100.0 |  | 94.8 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 97.1 |
| Heavy Trucks | 49 | 18 | 0 | 1 |  | 68 | 0 | 22 | 2 | 0 |  | 24 | 5 | 0 | 50 | 0 |  | 55 | 0 | 0 | 0 | 0 |  | 0 | 147 |
| \%Heavy Trucks | 6.3 | 1.2 | 0.0 | 100.0 |  | 2.9 | 0.0 | 1.6 | 0.7 | 0.0 |  | 1.5 | 3.2 | 0.0 | 5.5 | 0.0 |  | 5.2 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 2.9 |

roject ID: 20-10001-001
Location: SR 131/Tazewell Pike \& Gibbs Elementary School
City: Corryton

|  | SR 131/Tazewell Pike Northbound |  |  |  |  | SR 131/Tazewell Pike Southbound |  |  |  |  | Gibbs Elementary School Dwy Eastbound |  |  |  |  | Gibbs Elementary School Dwy Westbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Rgt | Uturn | App. Total | Left | Thru | Rgt | Uturn | App. Total | Left | Thru | Rgt | Uturn | App. Total | Left | Thru | Rgt | Uturn | App. Total | Int. Total |
| Peak Hour Analysis from 07:00 AM to 09:00 AM Peak Hour for Entire Intersection Begins at 07:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 75 | 7 | 0 | 0 | 82 | 0 | 93 | 24 | 0 | 117 | 6 | 0 | 84 | 0 | 90 | 0 | 0 | 0 | 0 | 0 | 289 |
| 7:15 AM | 103 | 15 | 0 | 0 | 118 | 0 | 85 | 63 | 0 | 148 | 6 | 0 | 120 | 0 | 126 | 0 | 0 | 0 | 0 | 0 | 392 |
| 7:30 AM | 106 | 23 | 0 | 0 | 129 | 0 | 70 | 70 | 0 | 140 | 29 | 0 | 124 | 1 | 154 | 0 | 0 | 0 | 0 | 0 | 423 |
| 7:45 AM | 25 | 39 | 0 | 0 | 64 | 0 | 69 | 11 | 0 | 80 | 15 | 0 | 57 | 0 | 72 | 0 | 0 | 0 | 0 | 0 | 216 |
| Total Volume | 309 | 84 | 0 | 0 | 393 | 0 | 317 | 168 | 0 | 485 | 56 | 0 | 385 | 1 | 442 | 0 | 0 | 0 | 0 | 0 | 1320 |
| \% App. Total | 78.6 | 21.4 | 0.0 | 0.0 | 100 | 0.0 | 65.4 | 34.6 | 0.0 | 100 | 12.7 | 0.0 | 87.1 | 0.2 | 100 | 0.0 | 0.0 | 0.0 | 0.0 | 0 |  |
| PHF |  |  |  |  | 0.762 |  |  |  |  | 0.819 |  |  |  |  | 0.718 |  |  |  |  |  | 0.780 |
| Cars, PU, Vans | 294 | 82 | 0 | 0 | 376 | 0 | 315 | 167 | 0 | 482 | 53 | 0 | 372 | 1 | 426 | 0 | 0 | 0 | 0 | 0 | 1284 |
| \% Cars, PU, Vans | 95.1 | 97.6 | 0.0 | 0.0 | 95.7 | 0.0 | 99.4 | 99.4 | 0.0 | 99.4 | 94.6 | 0.0 | 96.6 | 100.0 | 96.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 97.3 |
| Heavy Trucks | 15 | 2 | 0 | 0 | 17 | . | 2 | 1 | 0 | 3 | 3 | 0 | 13 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 36 |
| \%Heavy Trucks | 4.9 | 2.4 | 0.0 | 0.0 | 4.3 | 0.0 | 0.6 | 0.6 | 0.0 | 0.6 | 5.4 | 0.0 | 3.4 | 0.0 | 3.6 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 2.7 |


| PM | SR 131/Tazewell Pike Northbound |  |  |  |  | SR 131/Tazewell Pike Southbound |  |  |  |  | Gibbs Elementary School DwyEastbound |  |  |  |  | Gibbs Elementary School DwyWestbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Rgt | Uturn | App. Total | Left | Thru | Rgt | Uturn | App. Total | Left | Thru | Rgt | Uturn | App. Total | Left | Thru | Rgt | Uturn | App. Total | Int. Total |
| Peak Hour Analysis from 02:00 PM to 06:00 PM Peak Hour for Entire Intersection Begins at 02:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:45 PM | 42 | 59 | 0 | 0 | 101 | 0 | 43 | 21 | 0 | 64 | 10 | 0 | 87 | 1 | 98 | 0 | 0 | 0 | 0 | 0 | 263 |
| 3:00 PM | 27 | 66 | 0 | 0 | 93 | 0 | 39 | 13 | 0 | 52 | 16 | 0 | 84 | 0 | 100 | 0 | 0 | 0 | 0 | 0 | 245 |
| 3:15 PM | 10 | 54 | 0 | 1 | 65 | 0 | 44 | 1 | 0 | 45 | 5 | 0 | 38 | 0 | 43 | 0 | 0 | 0 | 0 | 0 | 153 |
| 3:30 PM | 52 | 142 | 0 | 0 | 194 | 0 | 49 | 7 | 0 | 56 | 12 | 0 | 47 | 0 | 59 | 0 | 0 | 0 | 0 | 0 | 309 |
| Total Volume | 131 | 321 | 0 | 1 | 453 | 0 | 175 | 42 | 0 | 217 | 43 | 0 | 256 | 1 | 300 | 0 | 0 | 0 | 0 | 0 | 970 |
| \% App. Total | 28.9 | 70.9 | 0.0 | 0.2 | 100 | 0.0 | 80.6 | 19.4 | 0.0 | 100 | 14.3 | 0.0 | 85.3 | 0.3 | 100 | 0.0 | 0.0 | 0.0 | 0.0 | 0 |  |
| PHF |  |  |  |  | 0.584 |  |  |  |  | 0.848 |  |  |  |  | 0.750 |  |  |  |  |  | 0.785 |
| Cars, PU, Vans | 113 | 316 | 0 | 0 | 429 | 0 | 167 | 42 | 0 | 209 | 43 | 0 | 242 | 1 | 286 | 0 | 0 | 0 | 0 | 0 | 924 |
| \% Cars, Pu, Vans | 86.3 | 98.4 | 0.0 | 0.0 | 94.7 | 0.0 | 95.4 | 100.0 | 0.0 | 96.3 | 100.0 | 0.0 | 94.5 | 100.0 | 95.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 95.3 |
| Heavy Trucks | 18 | 5 | 0 | 1 | 24 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 14 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 46 |
| \%Heavy Trucks | 13.7 | 1.6 | 0.0 | 100.0 | 5.3 | 0.0 | 4.6 | 0.0 | 0.0 | 3.7 | 0.0 | 0.0 | 5.5 | 0.0 | 4.7 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 4.7 |

## Detailed Calculations for Determining Count Data due to COVID-19

Project: Twin Oak Landing Subdivision
Date Conducted: 07/23/2020

Intersection: Tazewell Pike at Twin Oak Lane
Original Turning Movement Count Date: March 22, 2017

Assumed a 2\% Growth Rate for Traffic Entering / Exiting Twin Oak Lane
(reference Attachment 3 ADT trend line growth charts)
Projected Turning Movement Count for Twin Oak Lane for 2020 Existing Peak Hour Traffic

| Start | Tazewell Pike Northbound |  |  | Tazewell Pike Southbound |  | Twin Oak Lane Eastbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left |  | Thru | Thru | Right | Left |  | Right |
| AM Peak Hour |  |  |  |  |  |  |  |  |
| Existing AM Peak - March 2017 |  | 0 | - | - | 0 |  | 1 | 1 |
| Existing AM Peak - 2020 (2\% over 3 yrs) |  | 0 | - | - | 0 |  | 1 | 1 |
| PM Peak Hour |  |  |  |  |  |  |  |  |
| Existing PM Peak - March 2017 |  | 6 | - | - | 0 |  | 0 | 1 |
| Existing PM Peak - 2020 (2\% over 3 yrs) |  | 6 | - | - | 0 |  | 0 | 1 |

Figure 3: 2020 Existing Peak Hour Traffic - Includes the Twin Oak Lane Eastbound Left and Right Turns, Tazewell Pike Northbound Left Turns and Southbound Right Turns

Intersection: Tazewell Pike at Gibbs Elementary School Driveway
Original Turning Movement Count Date: February 4, 2020

|  | Tazewell Pike Northbound |  | Tazewell Pike Southbound |  | Elementary Driveway Eastbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start | Left | Thru | Thru | Right | Left | Right |
| AM Peak Hour |  |  |  |  |  |  |
| Existing AM Peak - February 2020 | - | 84 | 317 | 168 | 56 | - |
| PM Peak Hour |  |  |  |  |  |  |
| Existing PM Peak - February 2020 | - | 423 | 187 | 29 | 17 | - |

Calculated the Northbound and Southbound Thru Movements
(Count Data provided by Knox County Engineering and included in Attachment 2)

Northbound Thru Movement $=$ NB Thru + EB Left
AM Peak (7:00 a.m. to 8:00 a.m.) $=84$ veh +56 veh $=140$ veh
PM Peak (5:00 p.m. to 6:00 p.m.) $=423$ veh +17 veh $=440$ veh

Southbound Thru Movement $=$ SB Thru + SB Right
AM Peak (7:00 a.m. to 8:00 a.m.) $=317$ veh +168 veh $=485$ veh
PM Peak (5:00 p.m. to 6:00 p.m.) = 187 veh +29 veh $=216$ veh

Figure 3: 2020 Existing Peak Hour Traffic - Includes the Tazewell Pike Northbound and Southbound Thru Movements

## Attachment 3 <br> ADT Trends



Most Recent Trend Line Growth

| Year | ADT |
| :---: | :---: |
| 2001 | 7353 |
| 2018 | 8153 |

Annual Percent Growth

# Attachment 4 <br> Trip Generation 

## Project: Twin Oak Landing Subdivision

Date Conducted: 6/25/2020

Single-Family Detached Housing (LUC 210)
107 Single Family Lots
Average Daily Traffic
$\operatorname{Ln}(\mathrm{T})=0.92 \operatorname{Ln}(\mathrm{X})+2.71$
$\operatorname{Ln}(T)=0.92 \operatorname{Ln}(107)+2.71$
$\mathrm{T}=1107$

Peak Hour of Adjacent Street Traffic
One Hour Between 7 and 9 a.m.
$\mathrm{T}=0.71(\mathrm{X})+4.80$
$\mathrm{T}=0.71(107)+4.80$
$\mathrm{T}=81$
Peak Hour of Adjacent Street Traffic
One Hour Between 4 and 6 p.m.
$\operatorname{Ln}(T)=0.96 \operatorname{Ln}(X)+0.20$
$\operatorname{Ln}(T)=0.96 \operatorname{Ln}(107)+0.20$
$\mathrm{T}=108$

|  |  | Percent |  | Number |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Time Period | Total Trips | Enter | Exit | Enter | Exit |
| Weekday (24 hours) | 1107 | $50 \%$ | $50 \%$ | 554 | 554 |
| AM Peak Hour | 81 | $25 \%$ | $75 \%$ | 20 | 61 |
| PM Peak Hour | 108 | $63 \%$ | $37 \%$ | 68 | 40 |

## Single-Family Detached Housing

(210)

## Vehicle Trip Ends vs: Dwelling Units

 On a: Weekday

## Data Plot and Equation



## Single-Family Detached Housing (210)



## Data Plot and Equation



# Single-Family Detached Housing (210) 

$\left.\begin{array}{rl}\hline \text { Vehicle Trip Ends vs: } \\ \text { On a: } & \begin{array}{l}\text { Dwelling Units } \\ \text { Weekday, }\end{array} \\ & \text { Peak Hour of Adjacent Street Traffic, } \\ \text { One Hour Between 4 and 6 p.m. }\end{array}\right\}$

## Data Plot and Equation



Attachment 5
Intersection Worksheets - Existing AM/PM Peaks

## General Information

| Analyst | Addie Kirkham | Intersection | Tazewell Pike at Twin Oak |
| :--- | :--- | :--- | :--- |
| Agency/Co. | FMA | Jurisdiction | Knox County |
| Date Performed | $8 / 10 / 2020$ | East/West Street | Twin Oak Lane |
| Analysis Year | 2020 | North/South Street | Tazewell Pike |
| Time Analyzed | Existing AM Peak | Peak Hour Factor | 0.92 |
| Intersection Orientation | North-South | Analysis Time Period (hrs) | 0.25 |
| Project Description | 588.002 .1 - Twin Oak Landing |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LR |  |  |  |  |  |  | LT |  |  |  |  |  | TR |
| Volume, V (veh/h) |  | 1 |  | 1 |  |  |  |  |  | 0 | 140 |  |  |  | 485 | 0 |
| Percent Heavy Vehicles (\%) |  | 2 |  | 2 |  |  |  |  |  | 2 |  |  |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) | 7.1 | 6.2 |  |  |  |  |  | 4.1 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) | 6.42 | 6.22 |  |  |  |  |  | 4.12 |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) | 3.5 | 3.3 |  |  |  |  |  | 2.2 |  |  |  |  |  |  |
| Follow-Up Headway (sec) | 3.52 | 3.32 |  |  |  |  |  | 2.22 |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



## General Information

| Analyst | Addie Kirkham | Intersection | Tazewell Pike at Twin Oak |
| :--- | :--- | :--- | :--- |
| Agency/Co. | FMA | Jurisdiction | Knox County |
| Date Performed | $8 / 10 / 2020$ | East/West Street | Twin Oak Lane |
| Analysis Year | 2020 | North/South Street | Tazewell Pike |
| Time Analyzed | Existing PM Peak | Peak Hour Factor | 0.92 |
| Intersection Orientation | North-South | Analysis Time Period (hrs) | 0.25 |
| Project Description | 588.002 .1 - Twin Oak Landing |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LR |  |  |  |  |  |  | LT |  |  |  |  |  | TR |
| Volume, V (veh/h) |  | 0 |  | 1 |  |  |  |  |  | 6 | 440 |  |  |  | 216 | 0 |
| Percent Heavy Vehicles (\%) |  | 2 |  | 2 |  |  |  |  |  | 2 |  |  |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) | 7.1 | 6.2 |  |  |  |  |  | 4.1 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) | 6.42 | 6.22 |  |  |  |  |  | 4.12 |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) | 3.5 | 3.3 |  |  |  |  |  | 2.2 |  |  |  |  |  |  |
| Follow-Up Headway (sec) | 3.52 | 3.32 |  |  |  |  |  | 2.22 |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



Attachment 6 Intersection Worksheets - Background AM/PM Peaks

## General Information

| Analyst | Addie Kirkham | Intersection | Tazewell Pike at Twin Oak |
| :--- | :--- | :--- | :--- |
| Agency/Co. | FMA | Jurisdiction | Knox County |
| Date Performed | $8 / 10 / 2020$ | East/West Street | Twin Oak Lane |
| Analysis Year | 2023 | North/South Street | Tazewell Pike |
| Time Analyzed | Background AM Peak | Peak Hour Factor | 0.92 |
| Intersection Orientation | North-South | Analysis Time Period (hrs) | 0.25 |
| Project Description | 588.002 .1 - Twin Oak Landing |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LR |  |  |  |  |  |  | LT |  |  |  |  |  | TR |
| Volume, V (veh/h) |  | 1 |  | 1 |  |  |  |  |  | 0 | 149 |  |  |  | 515 | 0 |
| Percent Heavy Vehicles (\%) |  | 2 |  | 2 |  |  |  |  |  | 2 |  |  |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) | 7.1 | 6.2 |  |  |  |  |  | 4.1 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) | 6.42 | 6.22 |  |  |  |  |  | 4.12 |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) | 3.5 | 3.3 |  |  |  |  |  | 2.2 |  |  |  |  |  |  |
| Follow-Up Headway (sec) | 3.52 | 3.32 |  |  |  |  |  | 2.22 |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



## General Information

| Analyst | Addie Kirkham | Intersection | Tazewell Pike at Twin Oak |
| :--- | :--- | :--- | :--- |
| Agency/Co. | FMA | Jurisdiction | Knox County |
| Date Performed | $8 / 10 / 2020$ | East/West Street | Twin Oak Lane |
| Analysis Year | 2023 | North/South Street | Tazewell Pike |
| Time Analyzed | Background PM Peak | Peak Hour Factor | 0.92 |
| Intersection Orientation | North-South | Analysis Time Period (hrs) | 0.25 |
| Project Description | 588.002 .1 - Twin Oak Landing |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LR |  |  |  |  |  |  | LT |  |  |  |  |  | TR |
| Volume, V (veh/h) |  | 0 |  | 1 |  |  |  |  |  | 7 | 467 |  |  |  | 229 | 0 |
| Percent Heavy Vehicles (\%) |  | 2 |  | 2 |  |  |  |  |  | 2 |  |  |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) | 7.1 | 6.2 |  |  |  |  |  | 4.1 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) | 6.42 | 6.22 |  |  |  |  |  | 4.12 |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) | 3.5 | 3.3 |  |  |  |  |  | 2.2 |  |  |  |  |  |  |
| Follow-Up Headway (sec) | 3.52 | 3.32 |  |  |  |  |  | 2.22 |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



Attachment 7

## General Information

| Analyst | Addie Kirkham | Intersection | Tazewell Pike at Twin Oak |
| :--- | :--- | :--- | :--- |
| Agency/Co. | FMA | Jurisdiction | Knox County |
| Date Performed | $8 / 10 / 2020$ | East/West Street | Twin Oak Lane |
| Analysis Year | 2023 | North/South Street | Tazewell Pike |
| Time Analyzed | Full Buildout AM Peak | Peak Hour Factor | 0.92 |
| Intersection Orientation | North-South | Analysis Time Period (hrs) | 0.25 |
| Project Description | 588.002 .1 - Twin Oak Landing |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LR |  |  |  |  |  |  | L | T |  |  |  |  | TR |
| Volume, V (veh/h) |  | 10 |  | 53 |  |  |  |  |  | 17 | 149 |  |  |  | 515 | 3 |
| Percent Heavy Vehicles (\%) |  | 2 |  | 2 |  |  |  |  |  | 2 |  |  |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) | 7.1 | 6.2 |  |  |  |  |  | 4.1 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) | 6.42 | 6.22 |  |  |  |  |  | 4.12 |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) | 3.5 | 3.3 |  |  |  |  |  | 2.2 |  |  |  |  |  |  |
| Follow-Up Headway (sec) | 3.52 | 3.32 |  |  |  |  |  | 2.22 |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



## General Information

| Analyst | Addie Kirkham | Intersection | Tazewell Pike at Twin Oak |
| :--- | :--- | :--- | :--- |
| Agency/Co. | FMA | Jurisdiction | Knox County |
| Date Performed | $8 / 10 / 2020$ | East/West Street | Twin Oak Lane |
| Analysis Year | 2023 | North/South Street | Tazewell Pike |
| Time Analyzed | Full Buildout PM Peak | Peak Hour Factor | 0.92 |
| Intersection Orientation | North-South | Analysis Time Period (hrs) | 0.25 |
| Project Description | 588.002 .1 - Twin Oak Landing |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LR |  |  |  |  |  |  | L | T |  |  |  |  | TR |
| Volume, V (veh/h) |  | 6 |  | 35 |  |  |  |  |  | 65 | 467 |  |  |  | 229 | 10 |
| Percent Heavy Vehicles (\%) |  | 2 |  | 2 |  |  |  |  |  | 2 |  |  |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) | 7.1 | 6.2 |  |  |  |  |  | 4.1 |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) | 6.42 | 6.22 |  |  |  |  |  | 4.12 |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) | 3.5 | 3.3 |  |  |  |  |  | 2.2 |  |  |  |  |  |  |
| Follow-Up Headway (sec) | 3.52 | 3.32 |  |  |  |  |  | 2.22 |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



# Attachment 8 <br> Turn Lane Warrant Analysis 

Project: Twin Oak Landing Subdivision

Tazewell Pike at Twin Oak Lane LEFT TURN AM PM

Tazewell Pike at Twin Oak Lane RIGHT TURN AM PM

VOLUMES - SUBDIVISION FULL BUILDOUT

| Opposing | Thru | LT | LT MAX | Warrant Met |
| :---: | :---: | :---: | :---: | :---: |
| 518 | 149 | 17 | 70 | NO |
| 239 | 467 | 65 | 40 | YES |


| Thru | RT | RT MAX | Warrant Met |
| :---: | :---: | :---: | :---: |
| 515 | 3 | 49 | NO |
| 229 | 10 | 349 | NO |

## TABLE 5A

LEFT-TURN LANE YOLUME TYRESHOLDS
FOR TWO-LANE ROADWAYS WITH A PREVAILING SPEED OF 36 TO 45 MPH
(If the left-turn volume exceeds the table value a left -turn lane is needed)

| OPPOSING VOLUME | THROUGH VOLUNDE EXUS RYGET-TURNVOLUME * |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 350-399 | 400-499 | 450-499 | 500-549 | 550-589 | $=1>600$ |
| $\begin{aligned} & 100-149 \\ & 150-199 \end{aligned}$ | $\begin{aligned} & 70 \\ & 60 \end{aligned}$ | $\begin{aligned} & 60 \\ & 55 \end{aligned}$ | $\begin{aligned} & 50 \\ & 45 \end{aligned}$ | $\begin{aligned} & 45 \\ & 40 \end{aligned}$ | $\begin{aligned} & 40 \\ & 35 \end{aligned}$ | $\begin{aligned} & 35 \\ & 30 \end{aligned}$ |
| $\begin{aligned} & 200-249 \\ & 2511-209 \end{aligned}$ | $\begin{aligned} & 55 \\ & 50 \end{aligned}$ | $\begin{aligned} & 50 \\ & 45 \end{aligned}$ | $40$ | Peak 65 | $\begin{aligned} & 30 \\ & 30 \end{aligned}$ | $\begin{aligned} & 30 \\ & 30 \end{aligned}$ |
| $\begin{aligned} & 300-349 \\ & 350-399 \end{aligned}$ | $\begin{aligned} & 45 \\ & \text { 41). } \end{aligned}$ | $\begin{aligned} & 40 \\ & 35 \end{aligned}$ | $\begin{aligned} & 35 \\ & 30 \end{aligned}$ | $\begin{aligned} & 30 \\ & 25 \end{aligned}$ | $\begin{aligned} & 25 \\ & 25 \end{aligned}$ | $\begin{aligned} & 25 \\ & 20 \end{aligned}$ |
| $\begin{aligned} & 400-449 \\ & 450-499 \end{aligned}$ | $\begin{aligned} & 35 \\ & 30 \end{aligned}$ | $\begin{aligned} & 30 \\ & 25 \end{aligned}$ | $\begin{aligned} & 30 \\ & 25 \end{aligned}$ | $\begin{aligned} & 25 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ |
| $\begin{aligned} & 500-549 \\ & 550-599 \end{aligned}$ | $\begin{aligned} & 25 \\ & 25 \end{aligned}$ | $\begin{aligned} & 25 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | 20 | $\begin{aligned} & 15 \\ & 15 \end{aligned}$ |
| $\begin{aligned} & 600-64 y \\ & 650-690 \end{aligned}$ | $\begin{aligned} & 25 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 15 \\ & 15 \end{aligned}$ |
| $\begin{gathered} 700-749 \\ 750 \text { or More } \end{gathered}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | $\begin{aligned} & 20 \\ & 20 \end{aligned}$ | - 20 | $\begin{aligned} & 15 \\ & 15 \end{aligned}$ | $\begin{aligned} & 15 \\ & 15 \end{aligned}$ | $\begin{aligned} & 15 \\ & 15 \end{aligned}$ |

* Or through volume only if a right-turn lane exists


## TABLE 5B

RYGHT-TURN LANE VOLUME THRESHOLDS FOR TWO-LANE ROADWAYS WITH A PREVALLING SPEED OF 36 TO 45 MPH

| RIGHT-TURN <br> VOLUME | THROUGH VOLUME PLUS LEET-TURN VOLUME * |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $<100$ | 100-199 | 200-249 | 250-299 | 300-349 | 350-399 |
| $\begin{gathered} \text { Fewer Than } 25 \\ 25-49 \\ 50-99 \end{gathered}$ |  | PM Pea | - |  |  |  |
| $\begin{aligned} & 100-149 \\ & 150-199 \end{aligned}$ |  |  |  |  |  |  |
| $\begin{aligned} & 200-249 \\ & 250-299 \end{aligned}$ |  |  |  |  | Yes | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 300-349 \\ & 350-399 \end{aligned}$ |  |  | Yes | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 400-499 \\ & 450-499 \end{aligned}$ |  | Yes | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Y'es } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 500-54 y \\ & 550-594 \end{aligned}$ | Yes | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { yes } \end{aligned}$ | $\begin{aligned} & \text { y'ts } \\ & \text { y'us } \end{aligned}$ | $\begin{aligned} & \text { yes } \\ & \text { yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| 600 or More | Yes | Yes | Yes | Yes | Yes | Yes |


| RIGITT-TURN VOLUME | THROUGH VOLUME PLUS LERT-TURN VOLUNEE * |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 350-399 | 400-449 | 450-499 | 501-549 | 550-600 | $+1>600$ |
| $\begin{gathered} \text { Fuwer Tham } 25 \\ 25-49 \\ 50-99 \end{gathered}$ |  |  | AM Pea | RT <br> Yes | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 100-149 \\ & 150-199 \end{aligned}$ |  | Yes | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 200-249 \\ & 250-299 \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Y } 5 s \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & Y 45 \end{aligned}$ |
| $\begin{aligned} & 300-349 \\ & 350-399 \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 400-449 \\ & 450-499 \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ |
| $\begin{aligned} & 500-549 \\ & 550-599 \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yes } \end{aligned}$ | $\begin{aligned} & \text { Yes } \\ & \text { Yus } \end{aligned}$ |
| . 600 or More | Yes | Yes | Yes | Yes | Yes | Yes |

* Or through volume only if a lett-turn lane exists.

Date: August 10, 2020
Project Name: Twin Oak Subdivision

## To: Knoxville-Knox County Planning

## Subject: TIS Review for Twin Oak Subdivision (8-SA-20-C / 8-B-20-UR)

Dear Knoxville-Knox County Planning staff,
The following comment response document is submitted to address comments dated July 21, 2020:

1. Reviewer Comment: Page 3, (see attached markup)
a. First line, last word - change "Lane" to "Landing".
b. Add description of the proposed widening of Twin Oak Lane to 26 feet as noted on page 19 of the study.

Response: Revised "Lane" to "Landing and added a description on the widening of Twin Oak Lane to page 3.
2. Reviewer Comment: Page 8, please provide a worksheet outlining the step-by-step calculations and assumptions that were employed for the existing traffic volumes. The Planning review team needs to be able to follow how the existing counts were configured due to the pandemic. Please add such a worksheet to the Appendix.

Response: A worksheet was included in Attachment 2 outlining the step-by-step calculations and assumptions that were used in determining the traffic counts at the intersection of Tazewell Pike (SR 131) at Twin Oak Lane.

## Additional Comments from Knoxville-Knox County Planning dated 8/6/2020.

1. Reviewer Comment: Please look at the capacity analysis sheet for the AM Peak at Tazewell Pike \& Gibbs Elementary driveway. It looks like the "Total Volume" numbers should have been used instead of the "Cars, PU, Vans" numbers. This is minor but still needs to be updated.

Response: Revised to show the total volume for the AM Peak hour and updated the figures and HCS7 reports.
2. Reviewer Comment: Where is the 2023 Background Peak Hour Traffic chart?

Response: I removed the "2023 Background Peak Hour Traffic" from the detailed calculations since it was already covered under Section 3 Background Growth.
3. Reviewer Comment: In the attachment, is it "Southbound Thru Movement $=\mathrm{SB}$ Thru + SB Left" OR "Southbound Thru Movement = SB Thru + SB Right"?

Response: Revised to "Southbound Thru Movement $=$ SB Thru + SB Right"

Sincerely,


Addie Kirkham, P.E.

